ANALYSIS OF
ADAPTIVE FINITE ELEMENT SOLUTIONS IN ELASTOPLASTICITY
WITH REFERENCE TO TRANSFER OPERATION TECHNIQUES

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DECLARATION

This work has not previously been accepted in substance for any degree and is not being concurrently submitted in candidature for any degree.

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To Aqueró
Finally, I have arrived to this point. Embarking on a Ph.D. programme abroad is a very special and privileged experience. There is always an enrichment: scientific and especially cultural.

Letting the memory go through the four last years spent in Swansea is a touching emotion. It gives joy to remind all people that have shared with me an important part of my life, bringing each of them something new. Thus, it is with great pleasure that I recall all of them.

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"No numerical method can be considered satisfactory, unless something is known about the behaviour of the error $e(p)$ of the method as a function of $p$."

Peter Henrici in "Discrete Variable Methods in Ordinary Differential Equations"
Summary

The use of adaptive strategies in the finite element solution of history-dependent problems with incremental methods is of paramount importance. An adaptive strategy can be defined as a computational procedure which delivers the finite element solution for the problem at hand to the prescribed accuracy. Key ingredients are: (i) the availability of an error estimator which accounts for the sources of error associated with the approximation, (ii) error indicators for the choice of the optimal discretization parameters, and finally (iii) a data transfer procedure in the case the current finite element mesh is different from the one of the previous time step.

When the finite element mesh is changed at time $t_n$, two finite element solutions are considered for the same load level: the one at $t_n^-$ is associated with the old mesh $T_{h_n}$, and the other at $t_n^+$ is associated with the new mesh $T_{h_{n+1}}$. The latter is computed by equilibrating the data defined by the specific transfer procedure. Consequently, a discontinuity jump will appear in the time linear interpolation of the discrete values across the time node $t_n$. The global accuracy in time of the solution, therefore, will have to depend not only on the time step and finite element mesh size but also on the value of the jump.

In this work, a new error estimate is proposed and obtained as measure of the error in the constitutive equations produced by a time discontinuous admissible solution.

The new estimate presents a term which characterizes the time discontinuity, thus it lends itself for the assessment of the effects of transfer procedures in displacement finite element solutions of rate-independent plasticity discretized in time with the backward Euler method.

The new theory is formulated in tensorial notation and its applicability is illustrated on a one dimensional model problem where a detailed study of transfer procedures is carried out with numerical results providing confirmation of theoretical developments.

With the new theory, indications on how to change the finite element space and to define the corresponding data can be given and the assessment of the several transfer operations can be finally framed in the context of the ensuing error.
Contents

I Error Estimation. General Theory 1

1 Introduction 2
  1.1 The scope of the thesis 3
  1.2 Layout 4

2 Overview on a posteriori error estimates. Literature Review 6
  2.1 Introduction 6
  2.2 Linear problems 6
    2.2.1 Residual type error estimates 10
      2.2.1.1 Explicit residual a posteriori error estimates 10
      2.2.1.2 Implicit residual a posteriori error estimates 13
    2.2.2 Recovery based error estimators 21
  2.3 Nonlinear problems 25
    2.3.1 Nonlinear incremental problem 25
    2.3.2 Analysis of the time discretization error 30
    2.3.3 Error in the Constitutive Equations 37
    2.3.4 Heuristic Error Indicators 40
  2.4 Concluding remarks 45

3 The error in the constitutive equations for dissipative nonlinear problems 46
  3.1 Introduction 46
  3.2 The reference continuum problem: The Initial Boundary Value Problem for a model with internal variables 47
    3.2.1 Preliminaries 47
    3.2.2 Equilibrium Equation 47
    3.2.3 Compatibility Equations 47
    3.2.4 Constitutive Equations 48
      3.2.4.1 Thermodynamic Admissibility 48
      3.2.4.2 State variables 49
      3.2.4.3 Equations of State 49
      3.2.4.4 Complementary Equations. Associative Plasticity 53
      3.2.4.5 Examples of standard rate independent plasticity models 58
3.3 Contractivity of the elastoplastic flow ........................................ 64
3.4 Residual versus Error in Solution ........................................... 65
   3.4.1 A simple a posteriori error estimate via discrete energy dissipation .................................................. 68
3.5 Admissible solution and measure of the error ................................. 71
   3.5.1 Error in the constitutive equations for time continuous admissible solution ............................................... 73
      3.5.1.1 Dissipation Error ........................................ 73
      3.5.1.2 Extended Dissipation Error .............................. 76
   3.5.2 Error in the constitutive equations for admissible solution with jump across time instant $t_n$ ......................... 79
      3.5.2.1 Augmented Dissipation Error ............................ 82
      3.5.2.2 Augmented Extended Dissipation Error ................. 91
   3.5.3 Definition of error in solution ........................................ 94
      3.5.3.1 Extension of the Prager-Synge theorem to the Dissipation Error .................................................. 96
3.6 Concluding Remarks ......................................................... 97

II Application to the Finite Element Solution of the IBVP in Elasto-Plasticity 99

4 The Finite Element Solution of the IBVP in Elasto-Plasticity 100
   4.1 Introduction ........................................................................ 100
   4.2 The displacement formulation of the IBVP ............................ 101
      4.2.1 Statement of the problem ....................................... 103
   4.3 The time discrete problem ................................................ 104
   4.4 The fully discrete problem: Constant finite element mesh ........ 108
      4.4.1 Change of finite element mesh .................................... 111
   4.5 Overview on the different definitions of the initial state. Transfer procedures ......................................................... 113
      4.5.1 Variationally consistent transfer .................................... 115
      4.5.1.1 Heat conduction ........................................ 116
      4.5.1.2 Weak enforcement of the constitutive equations .... 118
      4.5.2 Weak enforcement of the continuity ........................... 123
      4.5.3 Smoothing transfer ............................................. 124
   4.6 Numerical techniques. Newton-Raphson method ....................... 126
      4.6.1 Line search method ........................................... 129
   4.7 Concluding Remarks ....................................................... 130

5 The error in the constitutive equations to assess the quality of the finite element solution with mesh constant in time 131
   5.1 Introduction ....................................................................... 131
6 Numerical studies of transfer operations for adaptive finite element solutions

6.1 Introduction ........................................... 172
6.2 Numerical studies of transfer operations for adaptive finite element solutions ........................................... 173

6.2.1 Augmented Extended Dissipation Error ...... 173
6.2.1.1 Construction of the admissible solution ...... 173
6.2.1.2 Error Expressions .................................... 175
6.2.1.3 Numerical examples .................................... 178

6.3 Concluding remarks .................................... 206

7 Conclusions ........................................... 207

7.1 Suggestions for further research ......................... 208

Bibliography ........................................... 211
Part I

Error Estimation. General Theory
Chapter 1

Introduction

In the scientific approach to the solution of engineering problems mathematical models are developed that describe the essential physical aspects of the problem. Mathematical models are mostly expressed in terms of complex differential and/or integral equations, and their solution in the majority of practical situations is impossible in the closed form. Therefore various approximation techniques have been used in obtaining some form of the approximate solution of the original problem.

It was not until the mid-fifties and advances in computer technology that made the approximate methods a powerful approach to the solution of practical engineering problems. Since then the approximate methods, and the finite element method in particular, have become the principal approach to solution of a large number of industrial applications in all areas of engineering including structural, civil, mechanical, aeronautical, chemical and since recently biomedical engineering.

One of the principal difficulties associated with the use of the finite element, and other approximate methods, is related to the accuracy, i.e., closeness of the approximation to the solution of the original problem. Since the closed form is not, in general, available the so-called error estimation techniques have been proposed.

The interest is here given to the discretization errors, which are caused by the numerical discretization of the continuous mathematical model. These involve approximations with the finite elements for the space variable and with the backward Euler method for the time variable. A first question is, therefore, to quantify the distance between the approximate solution and the exact solution. This is typified by the choice of a norm which is usually indicated by the functional setting in which the variational formulation is posed. These measures have usually global character, in the sense that the values of the function and/or its derivatives all over the space-time domain of interest are involved. In general, one can define measure of the error as a non-negative scalar function depending on the approximate and exact solution and describing the extent to which the approximate solution fails to coincide with the exact solution. This definition generally involves the knowledge of the exact solution which is unknown. Thus, the question of providing an estimate of this distance comes quite naturally. In particular, our interest is in \textit{a posteriori} error estimates, that is, estimates of a given measure of the error that are constructed after
the finite element solution has been computed, and they utilize the finite element solution and the input data of the concrete case of interest. These are different from \textit{a priori} error estimates which are based on a knowledge of the characteristics of the exact solution, and provide qualitative information about the asymptotic rate of convergence of the approximation as the discretization parameters approach their corresponding limit values.

\textit{A posteriori} error estimates play an important role in two related aspects of finite element calculations. First, such estimates provide the user of a finite element code with valuable information about the overall accuracy and reliability of the calculation. Second, since most \textit{a posteriori} error estimates are computed locally, they also contain significant information about the distribution of error among individual elements, referred to as error indicators, which can form the basis of adaptive procedures. However, error indicators can also be developed on heuristics and may have no direct relation with the error.

Use of adaptive strategies in solid mechanics for the finite element solution of history-dependent non–linear problems solved by employing incremental methods is of paramount importance. An adaptive strategy can be defined as a computational procedure which delivers the finite element solution for the problem at hand to the prescribed accuracy. Key ingredients are: (i) the availability of an error estimator which accounts for the sources of error associated with the approximation, (ii) error indicators for the choice of the optimal discretization parameters, and finally (iii) a data transfer procedure when the current finite element mesh is different from the one of the previous time step.

In the finite element analysis of these problems the quality of the simulation is generally assessed by physical or heuristic arguments based on the experience and judgement of the analyst. Frequently such arguments are later proved to be flawed, they are specific for the problem under consideration and often they fail to account for all the discretizations introduced, which therefore can produce a misleading trust in the accuracy of the approximate solution produced.

1.1 The scope of the thesis

The extended dissipation error developed in Ladevèze \textit{et al.} (1999) applied to the assessment of the accuracy of the finite element solution obtained by a fully implicit displacement formulation of the elastoplastic problem is able to account for the effects of time and space discretization. The analysis, however, is carried out by assuming the finite element mesh constant throughout the loading process. A property of this error is its non–decreasing character in time due to the accumulation of the discretization errors. As a result, during the computation with incremental procedures, one may need to modify the parameters which define the fully discrete scheme, namely time step size and finite element mesh, in order to obtain the corresponding solution to the prescribed global accuracy.

When only variation of the time step is sufficient to improve the accuracy of
the solution, the extended dissipation error can be used to assess the global quality of the finite element approximation because of the time continuity of the associated admissible solution. On contrary, when the finite element mesh is changed at time \( t_n \), two finite element solutions are considered for the same load level: the one at \( t_n^- \), which is associated with the old mesh \( T_{h_n} \), and the other at \( t_n^+ \), which is associated with the new mesh \( T_{h_{n+1}} \). The solution at \( t_n^+ \) is employed to define a time linear interpolation function which we require to satisfy the following property

\[
\lim_{\Delta t \downarrow 0} f_d(t_n + \Delta t) = \lim_{\Delta t \downarrow 0} f_i(t_n + \Delta t)
\]

where \( f_i = f_i(t_n + \Delta t) \) denotes the time linear interpolation over the time interval \([t_n^-, t_{n+1}^-]\) of the discrete values \( f_n^+ \) and \( f_{n+1}^- \) whereas \( f_d = f_d(t_n + \Delta t) \) is the function which associates with any given \( \Delta t \) the solution of the discrete scheme corresponding to the given \( \Delta t \) and data \( f_n^+ \). Consequently, a discontinuity jump appears in the time linear interpolation of the discrete values across the time node \( t_n \) as a result of the change of mesh and transfer procedure. In the development of reliable \textit{a posteriori} error estimators, one needs, therefore, to account not only for the time step and finite element mesh size but also for the value of the jump.

In this work, attention is given only to the error estimation procedure itself. With this regard, an error estimator is proposed which allows the assessment of the effects of transfer procedures in displacement finite element solution of rate-independent plasticity discretized in time with the backward Euler method. The extended dissipation error developed in Ladevèze \textit{et al.} (1999) will be augmented consistently by a term which accounts for the time discontinuity in the admissible solution. The new theory is formulated in tensorial notation and its applicability is illustrated on a one dimensional model problem where a detailed study of transfer procedures (Ortiz & Quigley, 1991; Perić \textit{et al.}, 1996; Rashid, 2002) is carried out with numerical results providing confirmation of theoretical developments. With such \textit{a posteriori} error estimator at hand, indications on how to change the finite element space and define the corresponding data can be given and the assessment of the several transfer operations can be finally framed in the context of the ensuing error.

### 1.2 Layout

This thesis is divided into two parts. The first one deals with the theory of the measure of the error in the constitutive equations. The theory is general and applies to admissible solutions for the problem under consideration. In the second part applications to the assessment of accuracy of finite element solutions of the initial boundary value problem in elastoplasticity are given.

The first part of the thesis, after this introductory chapter, is arranged as follows:

**Chapter 2** gives a brief overview of some error estimators for linear and nonlinear problems. The objective is to illustrate the motivating ideas behind each
of the proposed techniques and to provide motivation for the use of the error in the constitutive equations for the assessment of the accuracy of finite element solutions on evolving meshes.

Chapter 3 presents the general theory of the error in the constitutive equations to assess the quality of the so-called admissible solutions of dissipative nonlinear problems. We employ the theory of the extended dissipation error developed by Ladevèze et al. (1999) to accommodate admissible solutions with a discontinuity jump at the time instant $t_n$. This leads to a new error estimate which we call augmented extended dissipation error.

In the second part of the thesis, the arrangement is as follows:

Chapter 4 reports on the displacement finite element method for the solution of the initial boundary value problem of an elastoplastic model with internal variables and discusses the nature of the ensuing discretization errors. In particular, there is the fundamental observation that change of data and/or finite element mesh from one time interval to the other can be both related to a discontinuity jump of the approximate solution across the time instant $t_n$. Consequently, in the development of reliable a posteriori error estimates one needs to account also for the jump. A critical review of the current techniques to transfer data from one mesh to the other concludes the chapter.

Chapter 5 focuses mainly on how to use the extended dissipation error to assess the quality of the finite element solution with constant mesh in time. The main problem is, therefore, the definition of a corresponding admissible solution, which reflects the approximations associated with the finite element solution. After giving general guidelines, actual criteria to construct an admissible solution in the case of the Prandtl–Reuss model are given. The general theory is then applied to assess the quality of the finite element solution of one dimensional elastoplastic bar under axial load. The example shows that all trends on the error in the state laws and dissipation contribution are meaningful. Notable is also the comparison with classical measures of the exact error in solution. This shows that the extended dissipation error reflects quite well the evolution of the admissible solution with respect to the exact one as described by more classical measures of the error. Comparison with the classical dissipation error introduced in Ladevèze (1989) and developed in Ladevèze & Moës (1997) concludes the chapter.

Chapter 6 presents a general methodology for the assessment of the global quality of displacement finite element solutions of elastoplastic problems discretized in time with the backward Euler method on dynamically changing meshes. The methodology employs the extended dissipation error, augmented by the term which accounts for the time discontinuity in the admissible solutions. Its applicability is shown on a one dimensional model problem where a detailed study of the transfer operators is presented. The numerical results provide confirmation of the theoretical developments.

Chapter 7 presents a short summary and the conclusions of this work. Some suggestions for future research are finally given.
Chapter 2

Overview on a posteriori error estimates. Literature Review

2.1 Introduction

In this chapter we give a brief overview on some error estimators for linear and non-linear problems. The objective is mainly to illustrate the motivating ideas behind each of the proposed techniques, rather than attempting to provide a (necessarily incomplete) list of error estimators. We start, therefore, with reviewing some a posteriori error estimators for linear elliptic problems where it is possible to provide a theoretical unifying framework, which encompasses most of the existing procedures. Such analysis has been presented in Verfurth (1996), for instance. The advances obtained in the comprehension of the mechanism of error propagation corresponds to the maturity reached in the theory of linear elliptic partial differential equations (Evans, 1999) and their finite element approximation (Ciarlet, 1978). On contrary, the remaining class of problems, and in particular the mathematical models describing rate-independent and rate-dependent plasticity, present a far less unified approach, as the various types of nonlinearity are involved in quite different ways. However, for the class of problems which can be analysed with the methods of the convex analysis it is possible to identify some underlying threads. These derive from the duality theory which is a modern branch of the calculus of variation originated from the works of Fenchel, Moreau, Rockafellar and others. The key idea of the theory – simultaneous analysis of the primal and the so-called dual variational problem – is, for instance, exploited in the works of Ainsworth & Oden (1993); Ladevèze & Pelle (2001) and Paraschivoiu et al. (1997), thus representing an important tool in the a posteriori error analysis for those classes of problems.

2.2 Linear problems

The main concepts for the global control of the discretization error in energy norm for linear elliptic partial differential equations are next presented for the displacement
formulation of the model of linear elasticity. At this end, some preliminaries are necessary.

Let $\Omega$ be a bounded open connected subset of the three dimensional Euclidean space with polyhedral boundary $\partial \Omega = \partial \Omega_d \cup \partial \Omega_t$ and $\partial \Omega_d \cap \partial \Omega_t = \emptyset$. Here, $\partial \Omega_d$ denotes the part of $\partial \Omega$ where a prescribed displacement vector $u_d$ is fixed whereas the complementary part $\partial \Omega_t$ is where the boundary traction forces vector $t$ are applied. The displacement vector field $u = u(x)$ of the linear elastic model under the body force vector field $b$ is solution of the following variational problem

$$\begin{align*}
\int_{\Omega} \mathbf{C} \nabla_s u : \nabla \eta \, d\Omega &= \int_{\Omega} b \cdot \eta \, d\Omega + \int_{\partial \Omega_t} t \cdot \eta \, ds \quad \forall \eta \in V_0
\end{align*}$$

(2.1)

where $\mathbf{C}$ is the definite positive Hooke’s fourth order tensor, $\nabla_s u$ is the symmetric part of the second order tensor $\nabla u$, gradient of $u$, and $V_0$ is the infinite-dimensional space of the test functions defined as $V_0 = \{ v = \{ v_i \} \in [H^1(\Omega)]^3 \mid v = 0 \text{ on } \partial \Omega_d \}$ with $H^1(\Omega)$ the standard Sobolev space of scalar functions of $L^2(\Omega)$ with finite norm

$$\int_{\Omega} v_i^2 \, d\Omega + \int_{\Omega} (\nabla v_i)^2 \, d\Omega < \infty.$$

The well-posedness of problem (2.1) follows from the Lax–Milgram theorem, for $\partial \Omega_d$ has positive measure (Ciarlet, 1978). Furthermore, the latter and the properties of $\mathbf{C}$ permit one to conclude that the bilinear form

$$(v, w)_{V_0} = \int_{\Omega} \mathbf{C} \nabla_s v : \nabla w \, d\Omega$$

(2.2)

defines an inner product in the space $V_0$. The associated norm is referred to as the energy norm; it is equivalent to the standard norm of $V_0$ and it is given by

$$|||v||| = \left( \int_{\Omega} \mathbf{C} \nabla_s v : \nabla v \, d\Omega \right)^{1/2} = \sup_{w \in V_0} \frac{\left| \int_{\Omega} \mathbf{C} \nabla_s v : \nabla w \, d\Omega \right|}{|||w|||},$$

(2.3)

where the second equality follows from the Cauchy-Schwartz’s inequality. Hereafter, the space $V_0$ is endowed with the energy norm.

We will consider conforming finite element approximations of the problem (2.1). With this regard, let $\mathcal{T}_h = \{ \Omega_e \}$ be a finite element partition of $\Omega$ made up of polyhedrons $\Omega_e$ with faces $\gamma$. We denote with $\mathcal{E}^{h,\Omega}$ and $\mathcal{E}^{h,\partial \Omega_t}$ the sets of the faces which are contained in $\Omega$ (i.e., the interior faces) and in $\partial \Omega_t$, respectively. For the definitions of operations on tensors we refer to Gurtin (1981).
each face \( \gamma \in \mathcal{E}^{h,\Omega} \) we denote a fixed unit normal vector \( \mathbf{n} \), chosen arbitrarily from the two possibilities. For the faces \( \gamma \in \mathcal{E}^{h,\partial \Omega} \), \( \mathbf{n} \) is the outward normal to \( \Omega \). The definition of the elements \( \Omega_{\text{e,in}} \) and \( \Omega_{\text{t,out}} \) in relation to \( \mathbf{n} \) is depicted in Figure 2.1. Let \( V_0^h \subset V \) be any conforming finite element space associated with \( T_h \) and \( \{ \mathbf{N}^h_i \} \) the basis of the finite element shape functions. The finite element solution \( u_h \in u_d + V_0^h \) of the problem (2.1) is given by

\[
\int_{\Omega} \mathbf{C} \nabla_s u_h : \nabla \eta_h \, d\Omega = \int_{\Omega} \mathbf{b} \cdot \eta_h \, d\Omega + \int_{\partial \Omega_t} \mathbf{t} \cdot \eta_h \, ds \quad \forall \eta_h \in V_0^h
\]  

(2.4)

Thus, the discretization error \( e = u - u_h \) is solution of the variational problem

\[
\int_{\Omega} \mathbf{C}(\nabla_s e) : \nabla \eta \, d\Omega = \int_{\Omega} \mathbf{b} \cdot \eta \, d\Omega + \int_{\partial \Omega_t} \mathbf{t} \cdot \eta \, ds - \int_{\Omega} \mathbf{C} \nabla_s u_h : \nabla \eta \, d\Omega, \quad \forall \eta \in V_0
\]  

(2.5)

and, one has the error representation formula

\[
|||e|||^2 = \int_{\Omega} \mathbf{C}(\nabla_s e) : \nabla \eta \, d\Omega = \int_{\Omega} \mathbf{b} \cdot \eta \, d\Omega + \int_{\partial \Omega_t} \mathbf{t} \cdot \eta \, ds - \int_{\Omega} \mathbf{C} \nabla_s u_h : \nabla \eta \, d\Omega. \quad (2.6)
\]

The functional

\[
\mathcal{R}_{u_h}(\eta) = \int_{\Omega} \mathbf{b} \cdot \eta \, d\Omega + \int_{\partial \Omega_t} \mathbf{t} \cdot \eta \, ds - \int_{\Omega} \mathbf{C} \nabla_s u_h : \nabla \eta \, d\Omega,
\]

(2.7)

which appears at the right hand side of equation (2.5), is referred to as residual functional of \( u_h \) with respect to (2.1). It can be shown that it is an element of the dual topological space\(^4\) \( V_0^* \) of \( V_0 \). From (2.4) and accounting for the definition (2.7)

---

\(^4\) Let \( V \) be a topological vector space. The dual topological space \( V^* \) of \( V \) is the vector space of the linear continuous functionals over \( V \). If \( V \) is a normed space, a linear continuous functional \( F(v) \) over \( V \) is a bounded functional, that is, \( \sup_{v \in V} \frac{|F(v)|}{\|v\|_V} < \infty \). In this case, the vector space \( V^* \) is endowed with the norm \( \|F\|_{V^*} = \sup_{v \in V} \frac{|F(v)|}{\|v\|_V} \) (Brezis, 1986).

---
it follows
\[ R_{u_h}(\eta_h) = \int_{\Omega} b \cdot \eta_h \, d\Omega + \int_{\partial \Omega_t} t \cdot \eta_h \, ds - \int_{\Omega} C \nabla_s u_h : \nabla \eta_h \, d\Omega = 0, \quad \forall \eta_h \in V_0^h \quad (2.8) \]
that is, the residual functional \( R_{u_h}(\eta_h) \) vanishes over \( V_0^h \subset V_0 \). Thus, it is also
\[ (e, \eta_h)_{V_0} = \int_{\Omega} C \nabla_s e : \nabla \eta_h \, d\Omega = \int_{\Omega} \left( \sigma_{ex} - C \nabla_s u_h \right) : \nabla \eta_h \, d\Omega = 0, \quad \forall \eta_h \in V_0^h \quad (2.9) \]
which is the orthogonality condition between the discretization error \( e \) and the finite element space \( V_0^h \) with respect to the inner product defined by (2.2). Condition (2.9) means that the error \( e \in V_0 \) solution of (2.5) presents zero component in the space \( V_0^h \).

The localization of the integrals in (2.7) over each finite element \( \Omega_e \) and use of integration by parts gives
\[
R_{u_h}(\eta) = \sum_{\Omega_e \in T^h} \int_{\Omega_e} r_{u_h} \cdot \eta \, d\Omega + \sum_{\gamma \in E^{h,\Omega} \cup E^{h,\Omega_t}} \int_{\gamma} J_{u_h}^\gamma \cdot \eta \, ds = \]
\[
= \sum_{\Omega_e \in T^h} \left\{ \int_{\Omega_e} r_{u_h} \cdot \eta \, d\Omega + \sum_{\gamma \in \partial \Omega_e} \int_{\gamma} J_{u_h}^\gamma \cdot \eta \, ds \right\}. \quad (2.10)
\]
In equation (2.10), \( r_{u_h} = \text{div} \, C \nabla_s u_h + b \) is the regular part of the global residual associated with the lack of equilibrium of the finite element solution within the interior of the elements \( \Omega_e \), whereas \( J_{u_h}^\gamma \) has the following definition
\[
J_{u_h}^\gamma = \left\{ \begin{array}{ll}
[C \nabla_s u_h : n]_\gamma & \text{on } \gamma \in E^{h,\Omega} \\
t - C \nabla_s u_h : n & \text{on } \gamma \in E^{h,\Omega_t} \end{array} \right.
\]
where \( [C \nabla_s u_h : n]_\gamma \) denotes the jump of \( C \nabla_s u_h : n \) across the edge \( \gamma \in E^{h,\Omega} \), this value is independent on the choice of \( n \). Thus, \( J_{u_h}^\gamma \) represents the singular part of the global residual due to the lack of equilibrium in the normal tractions across the interelement boundaries and on the boundary \( \partial \Omega_t \), that is, on \( \gamma \in E^{h,\Omega} \cup E^{h,\Omega_t} \).

Evaluating the discretization error means to solve the same linear elastic model as (2.1) but with different boundary conditions and external loads. Now, the boundary conditions are given by \( e = 0 \) on \( \partial \Omega_d \) and \( C \nabla_s e : n = t - C \nabla_s u_h : n \) on \( \partial \Omega_t \), whereas the body forces are \( -\text{div} \, C \nabla_s u_h - b \) over each element \( \Omega_e \) and \( [C \nabla_s u_h : n]_\gamma \) are the surface loads applied on the faces \( \gamma \in E^{h,\Omega} \).

Nevertheless, problem (2.5) presents the same difficulty as the original problem (2.1), for it is posed in the infinite dimensional space \( V_0 \). One could, therefore, think of computing a finite element approximation \( e_h \) of \( e \). The adoption of the

---

\[ \text{II} \text{Given } \gamma \in E^{h,\Omega} \text{ with } n \text{ unit normal to } \gamma \text{ and } v \text{ a vector field defined in } \Omega, \text{ we denote by } [v]_\gamma \text{ the jump of } v \text{ across } \gamma \text{ in the direction } n: [v]_\gamma (x) = \lim_{a \to 0} v(x + an) - \lim_{a \to 0} v(x - an), \quad \forall x \in \gamma. \]
same finite element space $V^h_0$ would, however, deliver $e_h = 0$, because of the orthogonality of $e$ with respect to $V^h_0$. If a more accurate approximation for the error $e$ is sought, this would be equivalent to solve the original problem. Furthermore, this would also involve a computational effort that could be directed toward the evaluation of a better approximation for the solution of (2.1). In such a case, however, the error of the new more accurate finite element solution should presumably be estimated in any case, so that the same dilemma re–appears (Ainsworth & Oden, 2000). Keeping at the minimum the computation cost for the assessment of the accuracy of an approximate solution is, indeed, a fundamental feature of any error estimation technique.

The current schemes for accurate and quantitative estimates for the discretization error are usually classified according to how estimates of a given norm (or linear functional) of $e$ are obtained. Our attention is here mainly directed to the control of the accuracy in energy norm. In particular, we will consider the residual type and the averaging type error estimates.

2.2.1 Residual type error estimates

The residual functional $R_{u_h}(\eta)$ is the forcing of the problem (2.5) that defines the finite element error $e$. As a result, the solution of (2.5) will depend on $R_{u_h}(\eta)$. The starting point for this class of estimators can be assumed to be the equality between the energy norm of the error and the norm of the residual functional in $V^*_0$. This follows easily from the definition of norm of residual (see note ‡), equation (2.5) and (2.3),

$$\|R_{u_h}\|_{V^*_0} = \sup_{\eta \in V_0} \frac{|R_{u_h}(\eta)|}{\|\eta\|} = \frac{\int_{\Omega} C(\nabla e) : \nabla \eta \, d\Omega}{\|\eta\|} = |||e|||.$$  

Estimates for $|||e|||$ are, therefore, obtained by providing estimates of $\|R_{u_h}\|_{V^*_0}$. In turn, these can be obtained either through a direct computation using the finite element solution and the available data, or by solving local auxiliary problems, which give a representation of the functional $R_{u_h} = R_{u_h}(\eta)$. The first class of residual error estimates are referred to as explicit whereas the second one is called implicit.

2.2.1.1 Explicit residual a posteriori error estimates

These estimators were first introduced in Babuska & Rheinboldt (1978b) for the assessment of the accuracy of finite element approximations with higher order elements of 1D elliptic problems and then extended to 2D in Babuska & Rheinboldt (1979b). The bound can be expressed, in general, as follows

$$|||e||| \leq \sum_{\Omega \in T_h} \left\{ C_{i,1}^e h e \|r_{u_h}\|_{L^2(\Omega)} + \sum_{\gamma \in \partial \Omega} C_{i,2}^e h e^\frac{1}{2} \|J_{u_h}\|_{L^2(\partial e)} \right\}$$  

(2.11)

where $C_{i,i}^e$, $i = 1, 2$ are interpolation constants (Ciarlet, 1978) which depend on the shape of the element and the local order of the polynomial approximation, whereas
$h_e$ is the diameter of the element $\Omega_e$.

Apart from the constant $C^e_{I,j}$, all of the quantities on the right-hand side can be computed directly from the finite element approximation and the data for the problem of interest.

The relative importance of the two terms which appear in (2.11), the one associated with the interior residual $r_{\mathbf{u}_h}$ and the one associated with the jump $\mathbf{J}^e_{\gamma \mathbf{u}_h}$, was analysed for two-dimensional problems in Babuska & Miller (1987) and in Babuska & Yu (1987) for the case of irregular grids of bilinear quadrilaterals and biquadratic approximations, respectively. In the first case, the dominant term of the estimate was the residual jump whereas in the second case the error could be expressed only in terms of the residual in the interior of each element (see also the work of Carstensen & Verfurth (1999), where it is proven that for general meshes of linear triangles, the energy norm of the error may be estimated by employing only the jump $\mathbf{J}^e_{\gamma \mathbf{u}_h}$).

Error estimates in norms other than energy norm were analysed in Babuska & Rheinboldt (1981), though for one dimensional problems. However, the theoretical analysis appears quite cumbersome and not providing for an immediate extension. A streamlining of the estimation technique was contributed noteworthy by Johnson and coworkers in Eriksson & Johnson (1991); Johnson & Hansbo (1992); Eriksson et al. (1995). Their works involve a number of basic ideas which represent also the basis of the estimates for the quantity of interest. The gist of the procedure is the duality argument used by Aubin and Nitsche for the derivation of a priori error estimates in norms other than the energy norm (Ciarlet, 1978). The duality argument is also used for the purpose of deriving a posteriori error estimates through the following points (Johnson, 1994):

1. Error representation formula by means of a dual problem
2. Orthogonality of the Galerkin approximation
3. Interpolation error estimates
4. Strong stability of the dual problem

As an illustration of this procedure, we next sketch the control of the error in $L^2$ norm. Also, for simplicity, we will assume that $\partial \Omega_d = \partial \Omega$ so that $\mathbf{e} \in \mathcal{V}_0$. In this case, the error representation formula is given by

$$\|\mathbf{e}\|^2_{[L^2(\Omega)]^3} = \int_{\Omega} \mathbf{e} \cdot \mathbf{e} \, d\Omega = \int_{\Omega} \mathbf{C}^* \nabla_s \mathbf{\varphi} : \nabla \mathbf{e} \, d\Omega$$

where $\mathbf{\varphi} \in \mathcal{V}_0$ is solution of the dual problem

*Find: $\mathbf{\varphi} \in \mathcal{V}_0$

$$\int_{\Omega} \mathbf{C}^* \nabla_s \mathbf{\varphi} : \nabla \mathbf{\eta} \, d\Omega = \int_{\Omega} \mathbf{e} \cdot \mathbf{\eta} \, d\Omega \quad \forall \mathbf{\eta} \in \mathcal{V}_0$$

(2.12)
and $C^*$ is the adjoint of $C^\dagger$. As a result of the definition of adjoint and the Galerkin orthogonality (2.9), it is also

$$\|e\|_{L^2(\Omega)}^2 = \int_\Omega C^* \nabla_s \varphi : \nabla e \, d\Omega = \int_\Omega C \nabla_s e : (\nabla \varphi - \nabla I_{\Omega h} \varphi) \, d\Omega$$  (2.13)

where $I_{\Omega h} : \mathcal{V}_0 \to \mathcal{V}_h^*\mathcal{V}$ is a suitable $\mathcal{V}_h^\omega$-interpolation operator (Clément, 1975; Bernardi & Girault, 1998).

From equation (2.5), the localization of the integral in (2.13) and the use of Cauchy-Schwartz’s inequality, it follows

$$\|e\|_{L^2(\Omega)}^2 \leq \sum_{\Omega_e \in \mathcal{T}_h} \left\{ \|r_{u_h}\|_{L^2(\Omega_e^\omega)}^3 \|\varphi - \varphi_h\|_{L^2(\Omega_e^\omega)}^3 + \right.$$

$$\quad + \sum_{\gamma \in \partial \Omega_e} \|J_{u_h}^\gamma\|_{L^2(\gamma)}^3 \|\varphi - \varphi_h\|_{L^2(\gamma)}^3 \left\} \right.$$ \n
The terms $\|\varphi - \varphi_h\|_{L^2(\Omega_e^\omega)}^3$ and $\|\varphi - \varphi_h\|_{L^2(\partial \Omega_e)}^3$ describe the weight of the terms $\|r_{u_h}\|_{L^2(\Omega_e^\omega)}^3$ and $\|J_{u_h}^\gamma\|_{L^2(\partial \Omega_e)}^3$ in the local contribution to the error in $L^2$–norm, respectively.

The use of the interpolation error estimates (Ainsworth & Oden, 2000)

$$\|\varphi - \varphi_h\|_{L^2(\Omega_e^\omega)}^3 \leq C_{r, 1} \|\nabla \varphi\|_{L^2(\Omega_e^\omega)}^3 \|\Omega_e^\omega\|_{L^2(\partial \Omega_e)}^2 \|\varphi - \varphi_h\|_{L^2(\Omega_e^\omega)}^3 \leq C_{r, 1} \|\nabla \varphi\|_{L^2(\Omega_e^\omega)}^3,$$

where $\tilde{\Omega_e}$ is the patch of elements associated with $\Omega_e^\omega$, along with the stability of the global dual problem (2.12),

$$\|\nabla \varphi\|_{L^2(\Omega)}^2 \|\Omega_e^\omega\|_{L^2(\partial \Omega_e)}^2 \leq C_s \|e\|_{L^2(\Omega)}^2$$

where $C_s$ is the stability constant, it finally, gives

$$\|e\|_{L^2(\Omega)}^2 \leq C_s \sum_{\Omega_e \in \mathcal{T}_h} \left\{ C_{r, 1} h_e \|r_{u_h}\|_{L^2(\Omega_e^\omega)}^3 + \sum_{\gamma \in \partial \Omega_e} C_{r, 2} h^2 \|J_{u_h}^\gamma\|_{L^2(\gamma)}^3 \right\}. \quad (2.13)$$

The above error estimate does not admit cancellation between different elements $\Omega_e$ and on element level, as well. As a result, it is not very sharp. Furthermore, the

\[\text{Let } \mathcal{U} \text{ and } \mathcal{V} \text{ be Hilbert spaces with inner product } (\bullet, \bullet)_\mathcal{U} \text{ and } (\bullet, \bullet)_\mathcal{V}, \text{ respectively. Given a linear bounded operator } A : \mathcal{U} \to \mathcal{V}, \text{ there exists only one linear bounded operator } A^* : \mathcal{V} \to \mathcal{U} \text{ such that } (Au, v)_\mathcal{V} = (u, A^*v)_\mathcal{U}, \forall u \in \mathcal{U}, \forall v \in \mathcal{V}. A^* \text{ is called the adjoint of } A \text{ (Brezis, 1986). Fourth order tensors can be defined as linear mappings between the finite–dimensional vector spaces of second order tensors (Bonet & Wood, 1997). As a result, given } C, \text{ its adjoint } C^* \text{ is the fourth order tensor such that } C u : v = u : C^* v, \forall u, v \text{ second order tensors.}

\[\text{Given an element } \Omega_e \text{ of the mesh } \mathcal{T}_h, \text{ the patch } \tilde{\Omega_e} \text{ of elements associated with } \Omega_e \text{ is the set of elements } \Omega_i \text{ which share an edge with } \Omega_e. \text{ Given a vertex } i \text{ of the mesh } \mathcal{T}_h, \text{ the patch } \omega_i \text{ of elements associated with } i \text{ is the set of elements } \Omega_i \text{ which have } i \text{ as one of its vertices (Verfurth, 1996).} \]
mechanism of error propagation is accounted for only through the global stability constant $C_s$. In order to reflect better the local contribution of the local residual, in the context of control of quantity of interest in linear elasticity, Rannacher & Suttmeier (1997) implement only the steps 1, 2 and 3 of the above procedure. The term $\|\nabla \varphi\|_{L^2(\tilde{\Omega}_e)}$ is computed numerically by simply taking the first order difference quotient of an approximate solution $\varphi_h \in \mathcal{V}_h$ of the dual problem. This procedure was further improved by Suli & Houston (2001) in the case of error control of output of hyperbolic problems. Only the steps 1 and 2 were implemented and an approximation of the dual solution was retained in the bound as a local weight–function.

2.2.1.2 Implicit residual a posteriori error estimates

Estimates of the norm of the residual can be also obtained by solving local problems which define approximations to the local representation of the residual as opposite to the explicit estimators described in the previous section, which are computed directly in terms of the norm of the residuals $r_{uh}$ and $J_{uh}$. Despite the simplicity of implementation, the main disadvantages of the explicit estimators are the presence of generally unknowns constants $C_{l,i}$ and the lack of sharpness of the bound. The latter is consequence of various applications of the Cauchy-Schwarz inequality which provokes the loss of cancellation between the various types of residual (Ainsworth & Oden, 2000).

With the implicit approach, on contrary, in the definition of the estimate, one tries to retain the structure of the equation (2.5), which defines the error, as far as possible.

These estimators have, generally, the following format (Babuska & Strouboulis, 2001)

$$\eta = \sqrt{\sum_{\omega_i \in \mathcal{R}} \| e_{\omega_i} \|^2}$$

where $\mathcal{R} = \{\omega_i\}$ is a covering of $\Omega$ and $e_{\omega_i}$ is solution of a boundary value problem of the form

$$\begin{align*}
\text{Find: } e_{\omega_i} \in \mathcal{S}(\omega_i) \\
\int_\Omega C \nabla e_{\omega_i} \cdot \nabla \eta \, d\Omega = \mathcal{F}_{\omega_i}(\eta), \quad \forall \eta \in \mathcal{S}(\omega_i)
\end{align*} \quad (2.14)$$

with $\mathcal{S}(\omega_i)$ a suitable solution space and $\mathcal{F}_{\omega_i}(\eta)$ is defined in terms of the residuals $r_{uh}$ and $J_{uh}$.

According to the formulation of the auxiliary problems to solve, we distinguish

- subdomain residual error estimators;
- element residual error estimators;
- equilibrated element residual error estimators.
The subdomain residual error estimator was first introduced by Babuska & Reineboldt (1978a). The main argument is a localization via a partition of unity of $\Omega$. This leads to problems posed on the patch of elements $\omega_i$ associated with each node $i$. The solution space $S(\omega_i)$ is a finite element space with its elements vanishing on $\partial \omega_i - \partial \Omega$, continuous and piecewise polynomials of a sufficiently high degree whereas $F_{\omega_i}(\eta)$ is given by

$$ F_{\omega_i}(\eta) = \int_{\omega_i} b \cdot \eta \, d\Omega + \int_{\partial \omega_i \cap \partial \Omega_t} t \cdot \eta \, ds - \int_{\omega_i} C \nabla_s u_h : \nabla \eta \, d\Omega. $$

The function $e_{\omega_i}$ is, therefore, solution of a Dirichlet problem with homogeneous essential boundary conditions on $\partial \omega_i$. Existence and uniqueness of this solution is guaranteed by the Lax–Milgram’s theorem. The local patches used in this technique are, however, rather expensive to approximate accurately. In effect, each element is treated several times according to the number of patches with which it is associated. Also, the error indicators $\|e_{\omega_i}\|$ are in this way associated with the patches $\omega_i$ and not with the single element $\Omega_e$. This makes more difficult the definition of an optimal adaptive procedure based on $\|e_{\omega_i}\|$. The use of patches of elements $\omega_i$ is essentially a consequence of imposing Dirichlet boundary conditions on the auxiliary problems and of certain conditions required for the reliability of the error estimator (Verfürth, 1996).

On contrary, if only Neumann boundary conditions are imposed on the auxiliary problems, one can choose $\omega_i = \Omega_e$ (Verfürth, 1996). The resulting error estimators are referred to as elemental types and an immediate outcome of this approach is to have error indicators defined element by element. However, some care must be taken to insure the local Neumann problems are well posed.

The different estimation techniques differ in the way the well–posedness is achieved. With this regard, we distinguish the equilibrated elemental residual error estimators obtained by choosing the boundary data so that the underlying local problem is well posed and the elemental residual error estimators obtained by choosing the solution space $S(\Omega_e)$ so that the bilinear form $\int_{\Omega_e} C \nabla_s v : \nabla \eta \, d\Omega$ is coercive.

For instance, the second and third version of the error estimates introduced by Bank & Weiser (1985) are of the latter type, whereas the first version is of the former type, which will be described later on.

In the elemental error estimates, the functional $F_{\Omega_e}(\eta)$ is given by

$$ F_{\Omega_e}(\eta) = \int_{\Omega_e} r_{u_h} \cdot \eta \, d\Omega + \int_{\partial \Omega_e} J'_{u_h,av} \cdot \eta \, ds = $$

$$ = \int_{\Omega_e} b \cdot \eta \, d\Omega + \int_{\partial \Omega_e} t \cdot \eta \, ds - \int_{\Omega_e} C \nabla_s u_h : \nabla \eta \, d\Omega + \int_{\partial \Omega_e} J'_{u_h,av} \cdot \eta \, ds, $$

where $J'_{u_h,av}$ is obtained by averaging the jump $J^\gamma_{u_h}$ between the elements sharing the edge $\gamma \in E_{h,\Omega}$ as follows

$$ J_{u_h,av} = \frac{1}{2} \left[ C \nabla_s u_h^{\Omega_e,in} \cdot n + C \nabla_s u_h^{\Omega_l,out} \cdot n \right]_\gamma. $$
In this case, special consideration must be given to the choice of the finite element space \( S(\Omega_e) \) in order to guarantee the solvability of the local problems (2.14) and to produce useful error estimators.

In the case of linear finite element approximations of scalar elliptic equations, the space \( S(\Omega_e) \) used by Bank & Weiser (1985) to define the second and third version of their error estimates, is the space of the so-called bubble functions, that is, quadratic functions defined over \( \Omega_e \) and vanishing at its vertices. This space has been then augmented by cubic bubble functions by Verfurth (1989) in the definition of an error estimator for linear finite element approximations of Stokes equations.

Guidelines for choosing the space \( S(\Omega_e) \) are well established in the case of first order finite element approximations and are discussed in Oden et al. (1989). In the case of higher order finite element approximations, the selection of \( S(\Omega_e) \) is not an easy matter. In general, the criterion is the same as the one underlying the error estimates based on hierarchical bases (Bank & Smith, 1993): to increase the order of the space used to construct the original finite element approximation and then form the quotient space by subtracting the original finite element space. The influence of the choice of the different spaces on the solution of the local problem has been, however, investigated by Ainsworth (1996). A quite unsatisfactory state of affair has been shown, due to the sensitivity of the estimate to the choice of \( S(\Omega_e) \). In some cases the estimator is a gross overestimate, yet in others the estimated error is zero, despite the true error being nonzero. An alternative possibility is given, therefore, by the equilibrated element residual error estimates.

Likewise the estimators described previously, the equilibrated element residual error estimates are obtained by solving local Neumann problems. In this case, the well–posedness of the local problems is achieved by imposing the consistency of the boundary data. The idea is to consider an equilibrated splitting of the interelement flux \( J_{\mathbf{u}_h}^\gamma \) such that

\[
\begin{aligned}
\int_{\Omega_e} \mathbf{r}_{\mathbf{u}_h} \, d\Omega + \oint_{\partial \Omega_e} J_{\mathbf{u}_h}^\gamma \, d\mathbf{s} &= 0, \\
\int_{\Omega_e} \mathbf{r}_{\mathbf{u}_h} \cdot \eta \, d\Omega + \oint_{\partial \Omega_e} J_{\mathbf{u}_h}^\gamma \cdot \eta \, d\mathbf{s} &= 0,
\end{aligned}
\]

and the functional \( \mathcal{F}_{\Omega_e}(\eta) \) given by

\[
\mathcal{F}_{\Omega_e}(\eta) = \int_{\Omega_e} \mathbf{r}_{\mathbf{u}_h} \cdot \eta \, d\Omega + \oint_{\partial \Omega_e} J_{\mathbf{u}_h}^\gamma \cdot \eta \, d\mathbf{s}.
\]

The second condition in (2.15) is the consistency condition on the data of the following local Neumann problem

\[
\begin{aligned}
\text{div} \mathbf{C} \nabla s_{\Omega_e} &= \mathbf{r}_{\mathbf{u}_h} & \text{in } & \Omega_e, \\
\mathbf{C} \nabla s_{\Omega_e} \cdot \mathbf{n} &= J_{\mathbf{u}_h}^\gamma & \text{on } & \partial \Omega_e,
\end{aligned}
\]

which guarantees its well–posedness in \( [H^1(\Omega_e)]^3 \).
Different types of equilibrated element residual error estimates have been given in literature. These include the techniques proposed by Ladèvèze & Leguillon (1983), Kelly (1984), the first version of the error estimate of Bank & Weiser (1985) and the error estimate given in Ainsworth & Oden (1992), among others. These are differentiated between each other, basically, by the assumption on the splitting of the residual jump across the element boundaries.

A unifying theoretical framework for the equilibrated element residual error estimators has been developed by Ainsworth & Oden (1993). The gist of their analysis is a localization of the primal–hybrid variational formulation (Raviart & Thomas, 1977) of the problem (2.5) that characterizes the discretization error. The formulation is posed on the so–called broken Sobolev space $V_0(T)$\footnote{The broken Sobolev space $V_0(T)$ is the space of the functions $v$ of class $[H^1(\Omega)]^3$ over each element $\Omega \in T$ which meet homogeneous essential boundary conditions on $\partial \Omega \cap \partial \Omega_d$. As a result, an element $v \in V_0(T)$ may be discontinuous across $\gamma \in \mathcal{E}_{h,\Omega}$. If we let $V_0(\Omega) = \{ v \in [H^1(\Omega)]^3 \mid v = 0 \text{ on } \partial \Omega \cap \partial \Omega_d \}$, it is $V_0(T) = \prod_{\Omega \in T} V_0(\Omega)$.} associated with $T$ and it is obtained by relaxing the interelement continuity with the expense of introducing Lagrangian multipliers $\mu = \mu(v) \in \mathcal{M} \subset V_0^*(T)$. The latter are the linear and continuous functionals defined over $V_0(T)$ and vanishing over $V_0 \subset V_0(T)$. The elements of $\mathcal{M}$ permit the characterization of the interelement continuity of elements $v \in V_0(T)$. In this way, one can solve local problems which preserve the type of bound.

The main result is

$$-\frac{1}{2} |||e|||^2 = \inf_{v \in V_0(T)} \sup_{\mu \in \mathcal{M}} \mathcal{L}(v, \mu) = \sup_{\mu \in \mathcal{M}} \inf_{v \in V_0(T)} \mathcal{L}(v, \mu) \geq \inf_{v \in V_0(T)} \mathcal{L}(v, \mu) \forall \mu \in \mathcal{M},$$

where $\mathcal{L}(v, \mu)$ is the Lagrangian functional defined as follows

$$\mathcal{L}(v, \mu) = \sum_{\Omega \in T} \mathcal{J}_\Omega(v) - \mu^* + \mu$$

with

$$\mathcal{J}_\Omega(v) = \frac{1}{2} \int_{\Omega} \mathbf{C} \nabla_s v : \nabla v \mathrm{d} \Omega - \int_{\Omega} \mathbf{b} \cdot v \mathrm{d} \Omega - \int_{\partial \Omega \cap \partial \Omega_d} t \cdot v \mathrm{d} s +$$

$$+ \int_{\Omega} \mathbf{C} \nabla_s \mathbf{u}_h : \nabla v \mathrm{d} \Omega + \oint_{\partial \Omega_{h}} \mathbf{g}_{\partial \Omega_{h}} \cdot v \mathrm{d} s$$

and

$$\mu^*(v) = \sum_{\gamma \in \mathcal{E}_{h,\Omega}} \int_{\gamma} \mathbf{g}_{\gamma} \cdot [v]_\gamma \mathrm{d} s. \quad (2.16)$$

In equation (2.16), $\mathbf{g}_{\gamma}$ is a smooth vector field associated with each $\gamma \in \mathcal{E}_{h,\Omega}$ and $[v]_\gamma$ is the jump of $v \in V_0(T)$ across $\gamma$ (see note \footnote{The broken Sobolev space $V_0(T)$ is the space of the functions $v$ of class $[H^1(\Omega)]^3$ over each element $\Omega \in T$ which meet homogeneous essential boundary conditions on $\partial \Omega \cap \partial \Omega_d$. As a result, an element $v \in V_0(T)$ may be discontinuous across $\gamma \in \mathcal{E}_{h,\Omega}$. If we let $V_0(\Omega) = \{ v \in [H^1(\Omega)]^3 \mid v = 0 \text{ on } \partial \Omega \cap \partial \Omega_d \}$, it is $V_0(T) = \prod_{\Omega \in T} V_0(\Omega)$.}). The particular choice of $\mathbf{g}_{\gamma}$
determines the error estimation method. Once \( g \) has been chosen, the vector field \( g_{\partial \Omega_e} \) is defined on \( \partial \Omega_e \) for each element \( \Omega_e \in T_h \) such that

\[
\sum_{\Omega_e \in T_h} \oint_{\partial \Omega_e} g_{\partial \Omega_e} \cdot v \, ds = \sum_{\gamma \in \pi_h, \Omega} \int_{\gamma} g_{\gamma} \cdot [v]_\gamma \, ds.
\]

By choosing \( \mu = \mu^* \), one obtains

\[
|||e|||^2 \leq -2 \sum_{\Omega_e \in T_h} \inf_{v \in V_{0}(\Omega_e)} J_{\Omega_e}(v)
\]

where \( V_0(\Omega_e) \) is the restriction of \( V_0(T_h) \) to \( \Omega_e \) (see note ††).

The inequality (2.17) gives the link of the error estimate \( \eta \) to the solution of the following elemental primal problems

\[
\begin{align*}
\text{Find } w \in V_{0}(\Omega_e) \\
J_{\Omega_e}(w) = \inf_{v \in V_{0}(\Omega_e)} J_{\Omega_e}(v).
\end{align*}
\]

On the other hand, the dual formulation of (2.18) delivers (Mikhlin, 1964; Ekeland & Temam, 1976)

\[
\begin{align*}
\text{Find } p \in W_{\Omega_e} \\
G_{\Omega_e}(p) = \sup_{q \in W_{\Omega_e}} -\frac{1}{2} \int_{\Omega_e} q : q \, d\Omega, \\
\text{subject to } \forall q \in W_{\Omega_e},
\end{align*}
\]

where \( W_{\Omega_e} \) is the set of the stress tensors \( q \) solution of the following problem over \( \Omega_e \)

\[
\begin{align*}
\text{div } q &= r_{uh} \quad \text{in } \Omega_e \\
q : n &= g_{\partial \Omega_e}, \quad \text{on } \partial \Omega_e.
\end{align*}
\]

This set is not empty if the following condition is satisfied

\[
\int_{\Omega_e} r_{uh} \, d\Omega + \oint_{\partial \Omega_e} g_{\partial \Omega_e} \, ds = 0,
\]

which is the equilibration condition. Since it is

\[
\inf_{v \in V_0(\Omega_e)} J_{\Omega_e}(v) = \sup_{q \in W_{\Omega_e}} G_{\Omega_e}(q)
\]

then it follows

\[
|||e|||^2 \leq -2 \sum_{\Omega_e \in T_h} \sup_{q \in W_{\Omega_e}} G_{\Omega_e}(q) \leq -2 \sum_{\Omega_e \in T_h} G_{\Omega_e}(q) \quad \forall q \in W_{\Omega_e},
\]
that is, a concrete realization of an upper bound for $|||e|||$ depends on the definition of $g_{\partial \Omega_e}$ and on the choice of $q \in W_{\Omega_e}$.

The equilibration of the data (2.19), which is necessary for the model problem under consideration, is desirable to realize also when low order terms are present in the elliptic operator. In this case as the mesh size $h \to 0$, these terms can become preponderant and make the energy of the local solution blowing up. By imposing the equilibration of the data also in this case, the error estimator becomes finite (Ainsworth & Oden, 2000).

The error estimator introduced in Ladev`eze & Leguillon (1983) can be casted into the previous framework by choosing $g_{\partial \Omega_e}$ to be $J_{\Omega_e}^{\gamma}$ plus a suitable piecewise linear vector field on $\partial \Omega_e$ (Verfurth, 1999). However, this error estimate has been obtained by starting from other considerations which lead to the class of errors in the constitutive equations and they will be considered next. The previous analysis provides also theoretical support to the heuristic error estimate introduced by Kelly (1984) consisting in the solution of local complementary problems.

Finally, it is worth mentioning that Paraschivoiu et al. (1997) have developed an extension of this theory to the estimates of output of interest. Besides the relaxation of the interelement continuity, an additional constraint is introduced represented by the equilibrium equations over the broken Sobolev space, so that the admissible set is constituted by the only solution of the problem (Patera & Peraire, 2001). The value of this generalization lies in the application to problems which can be expressed in terms of minimization of a convex functional. An instance of such extension to a hyperelastic model has been given in Bonet et al. (2002).

The error in the constitutive equation. The Prager-Synge theorem

The notion of error in the constitutive equations has been introduced for the first time by Ladev`eze in 1975 (Ladev`eze, 1975; Lad`ev`eze, 1995) by exploiting the convex functional structure of the constitutive equations.

For linear elastic problems, this notion can be grasped quite easily. Let $\sigma_{ex} = \sigma_{ex}(x)$ and $u_{ex} = u_{ex}(x)$ be the exact stress field and the exact displacement field, respectively. This means that $\sigma_{ex}$ does satisfy the equilibrium equations, $u_{ex}$ is a kinematically admissible displacement field, that is, $u_{ex}$ meets the internal and external compatibility conditions and finally, $u_{ex}$ and $\sigma_{ex}$ are related to each other by the constitutive equation

$$\sigma_{ex} - C \nabla_s u_{ex} = 0.$$  \hfill (2.20)

Consider now a kinematically admissible displacement field $u_{ad} = u_{ad}(x)$. The energy norm of the error associated with $u_{ad} = u_{ad}(x)$ is defined as

$$|||u_{ad} - u_{ex}||| = \left( \int_{\Omega} [\sigma_{ex} - C \nabla_s u_{ad}] : C^{-1} [\sigma_{ex} - C \nabla_s u_{ad}] \, d\Omega \right)^{\frac{1}{2}}.$$ \hfill (2.21)

Rigorous upper bounds for (2.21) are obtained as

$$\eta(u_{ad}, \sigma_{ad}) = \left( \int_{\Omega} [\sigma_{ad} - C \nabla_s u_{ad}] : C^{-1} [\sigma_{ad} - C \nabla_s u_{ad}] \, d\Omega \right)^{\frac{1}{2}}.$$ \hfill (2.22)
where $\sigma_{ad} = \sigma_{ad}(x)$ is any statically admissible stress field, that is, a stress tensor field that satisfies the equilibrium equation. Note that the pair $(\sigma_{ad}, u_{ad})$ meets also the condition

$$
\int_{\Omega} (\sigma_{ex} - \sigma_{ad}) : (\nabla_s u_{ex} - \nabla_s u_{ad}) d\Omega = 0,
$$

which follows, by standard arguments, from the principle of the virtual work.

Equation (2.22) is a measure of the extent to which the pair $(\sigma_{ad}, u_{ad})$ fails to satisfy the constitutive equation (2.20) and is obtained by reformulating equation (2.20) into an equivalent form which uses the convex free elastic potential and its Legendre transform, given by the complementary potential.

The validity of the bound

$$
\|\| e_{ex} \|\| \leq \eta(\sigma_{ad}, u_{ad}), \quad \forall \sigma_{ad}
$$

is a simple consequence of the Prager–Synge’s theorem (Prager & Synge, 1947) which states the orthogonality between the fields $\sigma_{ex} - C\nabla_s u_{ad}$ and $\sigma_{ad} - \sigma_{ex}$. This reads as

$$
\int_{\Omega} [\sigma_{ad} - C\nabla_s u_{ad}] : C^{-1} [\sigma_{ad} - C\nabla_s u_{ad}] d\Omega =

= \int_{\Omega} [\sigma_{ex} - C\nabla_s u_{ex}] : C^{-1} [\sigma_{ex} - C\nabla_s u_{ex}] d\Omega +

+ \int_{\Omega} [\sigma_{ad} - C\nabla_s u_{ex}] : C^{-1} [\sigma_{ad} - C\nabla_s u_{ex}] d\Omega.
$$

**Proof.** In the case of linear elasticity, by accounting for the equivalence

$$
\sigma_{ex} - C(\nabla_s u_{ex}) = 0 \Leftrightarrow \frac{1}{2} \sigma_{ex} : C^{-1} \sigma_{ex} + \frac{1}{2} \nabla_s u_{ex} : C\nabla_s u_{ex} - \sigma_{ex} : \nabla_s u_{ex} = 0
$$

it is an easy matter to show the validity of the following equality

$$
\frac{1}{2} \sigma_{ad} : C^{-1} \sigma_{ad} + \frac{1}{2} \nabla_s u_{ad} : C\nabla_s u_{ad} =

= \frac{1}{2} \sigma_{ad} : C^{-1} \sigma_{ad} + \frac{1}{2} \nabla_s u_{ad} : C\nabla_s u_{ad} - \sigma_{ad} : \nabla_s u_{ad} +

+ \frac{1}{2} \sigma_{ex} : C^{-1} \sigma_{ex} + \frac{1}{2} \nabla_s u_{ex} : C\nabla_s u_{ex} - \sigma_{ex} : \nabla_s u_{ex} +

- (\sigma_{ex} - \sigma_{ad}) : (\nabla_s u_{ex} - \nabla_s u_{ad}),
$$

so that by integrating both sides of equation (2.26) and accounting for (2.23), equation (2.25) follows.

In the case of conforming finite element displacement approximations, one assumes $u_{ad} = u_h$ so that the actual realization of an upper bound $\eta$ for the energy
norm of the error $|||e_{ex}|||$ resolves in the definition of a statically admissible stress field $\sigma_{ad}$. However, for the efficiency of $\eta$ the definition of $\sigma_{ad}$ must be linked with the finite element solution $\sigma^h = C \nabla_s u_h$. This is realized with the so–called prolongation condition introduced by Ladevèze & Leguillon (1983). Such condition is a localization at each element $\Omega_e \in \mathcal{T}_h$ of the Galerkin orthogonality (2.9) which holds for the global residual $R_{uh}$. The prolongation condition distinguishes the statically admissible stress fields $\sigma_{ad}$ which satisfy the following equation (2.27) for every shape function $N_i$ and for all the elements $\Omega_e \in \mathcal{T}_h$,

$$\int_{\Omega_e} \left( \sigma_{ad} - C \nabla_s u_h \right) : \nabla N_i \, d\Omega = 0. \quad (2.27)$$

where $N_i$ stands for the vector of the shape functions associated with the node $i$. Condition (2.27), finally, corresponds to making an assumption on the splitting of the residual jump $J_{uh}$ across the interelement boundaries and $\sigma_{ad}$ is obtained as solution of the following local problem stated for each element $\Omega_e$:

$$\begin{cases}
\text{div } \sigma_{ad} + b = 0 \\
\sigma_{ad} : n = J_{uh}^\gamma, \Omega_e,
\end{cases}$$

where $J_{uh}^\gamma, \Omega_e$ is the part of the jump $J_{uh}^\gamma$ across $\gamma \in \partial \Omega_e$ which is assigned to $\Omega_e$.

Error indicators are obtained simply by the localization of the integral (2.22) as

$$\eta(u_{ad}, \sigma_{ad}) = \left( \sum_{\Omega_e \in \mathcal{T}_h} \eta^2_e \right)^{\frac{1}{2}}$$

where

$$\eta^2_e = \int_{\Omega_e} \left[ \sigma_{ad} - C \nabla_s u_{ad} \right] : C^{-1} \left[ \sigma_{ad} - C \nabla_s u_{ad} \right] \, d\Omega$$

The proof of (2.24) has large validity so that the error in the constitutive equations has also been applied to 2D and 3D elasticity by Ladevèze et al. (1991) and Coorevits et al. (1998), respectively; incompressible elasticity by Gastine et al. (1992) and to anisotropic meshes in Ladevèze (1994) and Ladevèze & Rougeot (1997). In each of these problems, the crunch of the estimation technique was always the definition of the equilibrating element tractions recovered by the finite element solution. A general procedure for such construction in the case of 2D finite element models has been developed in Ladevèze & Maunder (1996).

As we have mentioned earlier, local equilibrium problems with repartition of the residual jump have been proposed on heuristic basis also by Kelly (1984). It can be noted that the repartition that Kelly assumes in 1D corresponds to the prolongation condition introduced in Ladevèze & Leguillon (1983).

The concept of using two approximate solutions to build estimates of the error had been put forward also by Synge (1957) in establishing the hypercircle method. By using two approximate solutions located in spaces intersecting at the exact solution Synge builds estimates of solutions of the torsion problem.
We conclude this section by mentioning the work of Fraeijs de Veubeke (1965) that can be cast within the previous framework. Fraeijs de Veubeke provides estimates of the energy norm of the exact solution \( \| u_{\text{ex}} \| \) starting from the two-sided bounds for the exact free elastic energy in terms of the total elastic energy and the total complementary elastic energy. Let

\[
J(u_{\text{ad}}) = \frac{1}{2} \int_{\Omega} C \nabla_s u_{\text{ad}} : \nabla u_{\text{ad}} \, d\Omega - \int_{\Omega} b \cdot u_{\text{ad}} \, d\Omega - \int_{\partial\Omega} t \cdot u_{\text{ad}} \, ds
\]

be the total elastic energy defined over the affine space of the kinematically admissible displacement fields and

\[
G(\sigma_{\text{ad}}) = -\frac{1}{2} \int_{\Omega} \sigma_{\text{ad}} : C^{-1} \sigma_{\text{ad}} \, d\Omega
\]

the total complementary elastic energy defined over the affine space of the statically admissible stress fields. It is (Mikhlin, 1964)

\[
\forall u_{\text{ad}}, \quad J(u_{\text{ad}}) \geq J(u_{\text{ex}}) = G(\sigma_{\text{ex}}) \geq G(\sigma_{\text{ad}}), \quad \forall \sigma_{\text{ad}}.
\]

Thus, estimates of the energy norm of the exact solution \( \| u_{\text{ex}} \| \) were obtained as

\[
\eta(u_{\text{ad}}, \sigma_{\text{ad}}) = \sqrt{-2G(\sigma_{\text{ad}}) + 2J(u_{\text{ad}})}
\]

This procedure, however, failed to gain popularity being based on the global solution of the dual finite element method for the model under consideration, and also because the estimate cannot be expressed in terms of the contributions from each element, necessary for the optimization of the finite element meshes.

### 2.2.2 Recovery based error estimators

The recovery based error estimators represent certainly the class of error estimates that has met a big success in the engineering community for its relatively simple implementation. They were first introduced by Zienkiewicz & Zhu (1987) and since then many error estimators have been developed which employ the main idea. This relies on the following fact. The energy norm of the error, given by equation (2.3), can also be re-written as follows

\[
\| e \| = \left( \int_{\Omega} (\sigma_{\text{ex}} - C \nabla_s u_h) : C^{-1} (\sigma_{\text{ex}} - C \nabla_s u_h) \, d\Omega \right)^{\frac{1}{2}}, \quad (2.28)
\]

where the exact stress field \( \sigma_{\text{ex}} \) is unknown. Therefore, estimates to \( \| e \| \) are obtained by assuming in place of \( \sigma_{\text{ex}} \) in (2.28) approximations \( \sigma^* \) recovered by suitable postprocessing of the finite element solution \( \sigma^h = C \nabla_s u_h \), that is

\[
\| e \| \approx \eta = \left( \int_{\Omega} (\sigma^* - C \nabla_s u_h) : C^{-1} (\sigma^* - C \nabla_s u_h) \, d\Omega \right)^{\frac{1}{2}}.
\]
The quality and reliability of this type of error estimator is however dependent on the accuracy of the recovered solution. In general, it can be said that if \( \sigma^* \) is such that
\[
\int_{\Omega} (\sigma_{ex} - \sigma^*) : C^{-1}(\sigma_{ex} - \sigma^*) \, d\Omega \ll \int_{\Omega} (\sigma^* - C \nabla_s u_h) : C^{-1}(\sigma^* - C \nabla_s u_h) \, d\Omega
\]
then
\[
\int_{\Omega} (\sigma^* - C \nabla_s u_h) : C^{-1}(\sigma^* - C \nabla_s u_h) \, d\Omega \approx \int_{\Omega} (\sigma_{ex} - C \nabla_s u_h) : C^{-1}(\sigma_{ex} - C \nabla_s u_h) \, d\Omega.
\]
and one can define
\[
\eta = \left( \int_{\Omega} (\sigma^* - C \nabla_s u_h) : C^{-1}(\sigma^* - C \nabla_s u_h) \, d\Omega \right)^{\frac{1}{2}}
\]
as an a posteriori error estimator (with respect to the energy norm).

The procedures to build \( \sigma^* \) are, generally, referred to as the stress recovery or derivative recovery techniques. The definition of these methods finds their motivation in the observation that, under some conditions on the domain, mesh and regularity of the solution, there exist certain points of the domain where the derivatives of the finite element solution, \( C \nabla_s u_h \), which are usually one order lower than that of the finite element solution itself \( u_h \), have superior accuracy (Barlow, 1976). This phenomenon is known as superconvergence. If superconvergent derivatives can be recovered by a particular post–processing method, an asymptotically exact error estimator is then obtained (Ainsworth & Oden, 2000).

The recovery technique given initially in Zienkiewicz & Zhu (1987) assumes \( \sigma^* \) interpolated by the same functions as the displacements, i.e.
\[
\sigma^* = \mathbb{N} \bar{\sigma}^*
\]
where \( \bar{\sigma}^* \) are the nodal values of the continuous field \( \sigma^* \). The unknowns \( \bar{\sigma}^* \), in turn, are obtained by imposing that \( \sigma^* - C \nabla_s u_h \) is orthogonal to the space described by the shape functions \( \mathbb{N} \), that is,
\[
\int_{\Omega} (\sigma^* - C \nabla_s u_h) : \mathbb{N} \, d\Omega = 0.
\]

The Zienkiewicz–Zhu \( (Z^2) \) error estimator, whose corresponding error indicators are obtained simply by localization of the integral, was analysed in Ainsworth et al. (1989). It was found that while the estimator performs quite well for linear triangular and quadratic quadrilateral elements, it is not necessarily asymptotically exact. This property is shown to hold in the case of smooth solutions and parallel meshes by Babuska & Rodriguez (1993) and in Verfurth (1996) who refer to the analysis carried out by Rodriguez (1994). For other types of elements, the \( Z^2 \)-method was often found to behave poorly with the effectivity index converging to
zero in some cases. In others, the error indicators were misleading in steering the discretization process (Strouboulis & Haque, 1992). For this reason, Zhu & Zienkiewicz (1990) introduced first for one dimensional problems a new stress recovery procedure, termed as superconvergent patch recovery, by means of which superconvergent derivatives of the finite element solution are determined everywhere in the domain. The recovery procedure was then developed for 2D problems in Zienkiewicz & Zhu (1992a) and applied to error estimation in Zienkiewicz & Zhu (1992b). The continuous stress field $\sigma^*$ is, as usual, assumed to be given by equation (2.29). The nodal values $\bar{\sigma}^*$ are obtained by considering a continuous polynomial expansion on an element patch surrounding the nodes where the recovery is desired. This expansion is made to fit locally the superconvergent points, called also sampling points, in a least–squares manner or simply be an $L^2$–projection of the finite element derivatives. For the least–squares fitting, the superconvergent recovery is observed by the numerical test; for the local $L^2$–projection fitting, a considerable improvement for the nodal values is achieved.

Improvements of the method were contributed by Wiberg & Abdulwahab (1993) that include the governing equilibrium equation on the recovered derivatives. As in Zienkiewicz & Zhu (1992a), these are assumed to be interpolated by the same shape functions as the finite element displacement field, that is, $\sigma^*$ is assumed of the form (2.29). The nodal values of the recovered stress field are also here obtained by assuming for the stresses a polynomial expansion over the patch of elements around the given node. The coefficients of this expansion are then computed by minimizing, in a least–square sense, the residual of the stresses at the superconvergent points and the weighted residual in the equilibrium equation over the local patch of elements. This recovery techniques was successively improved by Wiberg et al. (1994) for the recovery of derivatives near the boundaries where either tractions or displacements are prescribed. This was obtained by including seemingly a weighted residual error at the boundary points in the patch recovery and a pronounced improvement in the post processed gradients of the finite element solution was finally observed.

A complete analysis of the several recovery based error estimators in terms of the operator that defines the improved stress as function of the consistent derivatives of the finite element solution can be found in Ainsworth & Oden (2000). Carstensen & Funken (2000) analyze, on the other hand, their robustness with respect to violated (local) symmetry of meshes or superconvergence and with respect to incompressible locking.

These error estimators, however, are justified to varying extent by superconvergence properties which are known to hold only in special cases. In Babuska & Strouboulis (2001), therefore, a new definition of superconvergence - the $\eta\%$–superconvergence - is considered which generalizes the classical idea of superconvergence to general meshes. By means of this property one can choose the best position of the sampling points when properties of superconvergence do not hold.

Remark 2.1. If the smoothed stress field $\sigma^*$ is chosen as an equilibrated stress field $\sigma_{eq}$, for instance, with the criteria given in the previous section, then one retrieves the equilibrated element residual error estimates. 

23
A numerical methodology which determines the quality of *a posteriori* error estimators has been set up by Babuska *et al.* (1994b) and Babuska *et al.* (1994a). The authors observe that the use of general benchmarks to validate *a posteriori* error estimates can lead to wrong conclusions if they are not properly chosen to isolate the basic factors which influence the performance of the estimator. As a result, an objective and standardized means to assess the robustness of an estimator that exercises all the feature of the particular estimator is given. However, this methodology presents its own limitations. The procedure allows the evaluation of the extreme bounds of the effectivity indices for the estimator when certain effects such as the influence of the singularities, the effect due to the boundary of the domain and mesh grading have been isolated. Moreover, the effectivity indices are those that would be obtained in the asymptotic limit when the mesh size approaches to zero. The presymptotic behaviour of the estimators might well lead to rather different conclusions concerning the suitability of a particular estimator. Thus, this methodology must be seen not as a means to justify an estimator, but rather as a minimal criterion the estimator must meet. For the details of the procedure and its motivating ideas we refer to the above works and to Ainsworth & Oden (2000).

The main general conclusions of the studies carried out in Babuska *et al.* (1994b) and Babuska *et al.* (1994a) on the quality of estimators for piecewise affine finite approximations on triangular elements can be summarized as follows

- The performance of an estimator depends on the class of meshes, solutions and materials of interest.

- Among the residual estimators tested by the above authors, the implicit element residual estimator with equilibration was the most robust, namely it gives good results for several mesh types, for highly orthotropic materials and arbitrary grid material orientations. In particular, the equilibration proposed in Ladevèze & Leguillon (1983) was recommended.

- The Superconvergence patch recovery error estimator developed in Zienkiewicz & Zhu (1992b) gives good results for the class of smooth solutions approximated on patchwise uniform grids of linear and quadratic elements.

- Asymptotic exactness for an estimator can occur for special uniform grids only and cannot give a measure of quality for the estimator for the general meshes employed in engineering computations.

- The quality of the analysed error estimators tends to deteriorate on anisotropic meshes.

The value of the methodology lies also in the fact that it requires only the solution of small problems in the region of interest; it is inexpensive and it can be used to check the quality of any new estimator even if it is only available as a black-box computer subroutine.
2.3 Nonlinear problems

Unlike the linear problems analysed in the previous section, where a certain maturity has been reached in the comprehension of the mechanisms of propagation of the error, in the case of nonlinear problems, and in particular for those dependent also on the time, the theory of error estimation can be considered still in its infancy. This is reflected in the paucity of studies dedicated to the matter and of originality of the approaches, which usually try to adapt ideas developed for linear problems. Although it is difficult to make a classification of the techniques of estimation, for the nonlinearities are involved in a quite different ways and for the different nature in the approximation of the time and space variable, it may be useful to distinguish the several contributions as follows (Gallimard, 1994):

- Error estimators for problems where the time variable does not appear;
- Error estimators which attempt to estimate also the effects of the time discretization;
- Error in the constitutive equations;
- Methods based on heuristic considerations and direct to the development of error indicators.

For each of this class, the more meaningful works will be outlined, especially in relation to plasticity.

2.3.1 Nonlinear incremental problem

An approach to a theoretically justified a posteriori error estimate for the finite element approximation of plasticity problems was given in Johnson & Hansbo (1992). These authors analysed the regularized version of the Hencky problem in small strain perfect plasticity with Von Mises yield criterion given by

\[
\begin{aligned}
\text{div} \sigma_\mu &= b & \text{in } \Omega \\
u_\mu &= 0 & \text{on } \partial\Omega \\
C^{-1}\sigma_\mu + \frac{1}{\mu}(\sigma_\mu - P\sigma_\mu) &= \nabla_s u_\mu & \text{in } \Omega ,
\end{aligned}
\]  

(2.30)

where \( u_\mu \) and \( \sigma_\mu \) denote the solutions of the regularized problem. In equation (2.30) \( \mu \) represents the regularization parameter, whereas \( P\tau \) is the projection of the second order stress tensor \( \tau \in S \) onto the convex elastic domain

\[ E = \{ \sigma \in S : |\sigma^D| - \sigma_y \leq 0 \} . \]

Here \( S \) is the space of the second order symmetric stress tensors, \( \sigma_y \) is the first yield stress, \( \sigma^D = \sigma - \frac{1}{3} \text{Tr}[\sigma]I \) is the deviator tensor of \( \sigma \) and \( |\sigma| = \langle \sigma, \sigma \rangle^{\frac{1}{2}} \), with
\( \langle \sigma, \tau \rangle = \sigma : C^{-1} \tau \). The projection \( P \tau \) of \( \tau \) onto \( E \) is defined as the solution of the following minimization problem

\[
P \tau = \arg \min_{\sigma \in \mathbb{E}} |\sigma - \tau|
\]

which is explicitly given by (Rannacher & Suttmeier, 1998)

\[
P \tau = \begin{cases} 
\tau & \text{if } |\tau^D| \leq \sigma_y \\
\frac{\tau^D}{|\tau^D|} \sigma_y + \frac{1}{3} \text{Tr}[\tau] I & \text{if } |\tau^D| > \sigma_y.
\end{cases}
\]

The projection \( P \tau \in \mathbb{E} \) is also solution of the following variational inequality

\[
\langle \tau - P \tau, \sigma - P \tau \rangle \leq 0, \quad \forall \sigma \in \mathbb{E},
\]

therefore, given \( \tau_1 \in S \), \( P \tau_1 \in \mathbb{E} \) is such that

\[
\langle \tau_1 - P \tau_1, \sigma - P \tau_1 \rangle \leq 0, \quad \forall \sigma \in \mathbb{E},
\]

and likewise, given \( \tau_2 \in S \), \( P \tau_2 \in \mathbb{E} \) is such that

\[
\langle \tau_2 - P \tau_2, \sigma - P \tau_2 \rangle \leq 0, \quad \forall \sigma \in \mathbb{E}.
\]

Set \( \sigma = P \tau_2 \in \mathbb{E} \) in (2.33) and \( \sigma = P \tau_1 \in \mathbb{E} \) in (2.34) and sum up both the sides, one obtains

\[
\langle \tau_1 - \tau_2, P \tau_1 - P \tau_2 \rangle \geq \langle P \tau_1 - P \tau_2, P \tau_1 - P \tau_2 \rangle \geq 0.
\]

That is, if we denote with \( P \) also the operator \( P : \tau \in S \rightarrow P \tau \) defined by (2.31), equation (2.35) shows that \( P \) is a monotone operator and is nonexpansive, i.e. \( |\tau_1 - \tau_2| \geq |P \tau_1 - P \tau_2| \).

---

\(^{\dagger}\)Let \( V \) and \( V^* \) be two linear topological spaces placed in duality by the separating bilinear form \( \langle \cdot, \cdot \rangle_{V, V^*} \). Denote with \( 2^{V^*} \) the space of subsets of \( V^* \). Let \( T: V \rightarrow 2^{V^*} \). We set \( D(T) = \{ v \in V: T(v) \neq \emptyset \} \subseteq V \), i.e. the effective domain of \( T \); \( R(T) = \{ w \in T(v) \in 2^{V^*}: v \in D(T) \} \subseteq 2^{V^*} \), i.e. the range of \( T \); and \( G(T) = \{ (v, w) \in V \times V^*: v \in V, w \in V^*, w \in T(v) \} \subseteq V \times V^* \) i.e. the graph of \( T \). The operator \( T \) is said to be monotone if \( \langle w - v_1, v - v_1 \rangle_{V, V^*} \geq 0 \) \( \forall v, v_1, v \in V, \forall w \in T(v) \subseteq V^* \) \( \forall w_1 \in T(v_1) \subseteq V^* \). The operator \( T \) is said to be cyclically monotone if \( \langle w_0, v_1 - v_0 \rangle + \cdots + \langle w_n, v_0 - v_n \rangle \leq 0 \) whenever \( w_i \in T(v_i) \) for \( i = 1, 2, \ldots, n \) \( (n \) arbitrary). The operator \( T \) is said to be maximal monotone if and only if \( \forall (v, w) \in G(T) \), \( \langle w - w_1, v - v_1 \rangle_{V, V^*} \geq 0 \) implies \( v_1 \in D(T) \) and \( w_1 \in T(v_1) \). If \( T \) is a maximal monotone operator, for any \( v \in D(T) \), the image \( T(v) \) is a closed convex subset of \( (V^*, \sigma(V^*, V^*)) \) where \( \sigma(V^*, V^*) \) denotes the weak topology on \( V^* \) generated by \( V \). Let \( H \) be a Hilbert space and \( T: H \rightarrow H \). The following three propositions are equivalent: (i) \( T \) is maximal monotone; (ii) \( T \) is monotone and \( R(I + T) = H \); (iii) \( \forall \lambda > 0, (I + \lambda T)^{-1} \) is a nonexpansive single valued map defined everywhere on \( H \). If \( T \) is a maximal monotone operator, the operator \( J_\lambda = (I + \lambda T)^{-1} \) is referred to as the resolvent of \( T \), whereas the operator \( \frac{1}{\lambda}(I - J_\lambda) \) is called the Yosida approximation of \( T \) (Brezis, 1986; Pascali & Sburlan, 1978).
Equation (2.30) describes also the physical model of viscoplasticity which is well-posed in the usual Sobolev spaces (Duvaut & Lions, 1976). For the original model of perfect plasticity, on contrary, the displacements must be sought in the more technical space of the bounded deformations (Temam, 1985) if one requires to guarantee their existence.

Let the complementary energy norm of the error on the stresses be defined as

$$
\| \sigma_{\mu} - \sigma_{\mu}^h \|_E^{\text{def}} = \int_{\Omega} |\sigma_{\mu} - \sigma_{\mu}^h|^2 \mathrm{d}\Omega,
$$

where $\sigma_{\mu}^h$ denotes the consistent finite element stress tensor obtained from the displacement finite element approximation of (2.30). From the monotony of the operator $I - P$ it follows

$$
\| \sigma_{\mu} - \sigma_{\mu}^h \|_E^2 \leq R_{\sigma_{\mu}^h}(e)
$$

where $e = u_{\mu} - u_{\mu,h}$ and the functional $R_{\sigma_{\mu}^h}(\eta)$ is the residual produced by $\sigma_{\mu}^h$ in the equilibrium equations, which is given by

$$
R_{\sigma_{\mu}^h}(\eta) = \int_{\Omega} b \cdot \eta \mathrm{d}\Omega - \int_{\Omega} \sigma_{\mu}^h : \nabla \eta \mathrm{d}\Omega.
$$

Further to a heuristic argument, Johnson & Hansbo (1992) distinguish two contributions into $R_{\sigma_{\mu}^h}$. One contribution comes from the part $\Omega^el$ of the domain $\Omega$ which remains elastic both in the continuous and in the finite element model, whereas the other contribution comes from the complementary part $\Omega^pl = \Omega - \Omega^el$.

As a result, they finally propose the following estimate

$$
\| \sigma_{\mu} - \sigma_{\mu}^h \|_E \leq \left( \sum_{j=1}^2 hC_j^2 R_j(\sigma_{\mu}^h) \right)^{\frac{1}{2}} \leq \left( \sum_{j=1}^{27} hC_j^2 R_j(\sigma_{\mu}^h) \right)^{\frac{1}{2}} (2.36)
$$

where

$$
R_1(\sigma_{\mu}^h) = r_{\sigma_{\mu}^h} = \mathrm{div} \sigma_{\mu}^h + b \quad \text{on } \Omega_e \in T_h
$$

$$
R_2(\sigma_{\mu}^h) = \max_{\gamma \subset \partial \Omega_e} \gamma \sup_{\gamma} \frac{1}{2} \left( \frac{1}{h} \frac{\| J_{\gamma} \sigma_{\mu}^h \|_{L^2(\gamma)}^2}{\| J_{\gamma} \sigma_{\mu}^h \|_{L^2(\gamma)}^2} \right) \quad \text{on } \Omega_e \in T_h
$$

$$
J_{\sigma_{\mu}^h}^{-1} = [\sigma_{\mu}^h : n]_{\gamma} \quad \text{on } \gamma \in \mathcal{E}^h,\Omega
$$

$$
C_s = \| \nabla s u_{\mu} \|_{L^1(\Omega)}^{3 \times 3} + \| \nabla s u_{\mu,h} \|_{L^1(\Omega)}^{3 \times 3}
$$

and $C_j$ are the usual interpolation constants.

The two contributions to the error estimate in (2.36) reflect the type of dependence on the mesh size present in the a priori error estimate found by Johnson (1976b) for finite element approximation of this problem. Therein, the estimate has the following structure

$$
\| \sigma_{\mu} - \sigma_{\mu}^h \|_E \leq O(h) + O(\sqrt{h})
$$
with the $O(h)$ and $O(\sqrt{h})$–terms related to $\Omega^e$ and $\Omega^d$, respectively. However, since the \textit{a priori} error estimate is sub–optimal, one expects the mesh will be more refined in the plastic part $\Omega^d$, where the stresses are suspected to be rather smooth (Fuchs & Seregin, 2000).

Furthermore, the estimate (2.36) is not a full \textit{a posteriori} error estimate, since $\Omega^e$ and $C^s$ depend on $u_\mu$. Therefore, the authors suggest to replace $u_\mu$ with $u_{\mu,h}$ for the computation of $C^s$ and assume for $\Omega^e$ only the part of the discrete model which remains elastic. This is quite arbitrary, for it implies that the plastification zone is already correctly captured on the current mesh.

The analysis of the Hencky problem in small strain perfect plasticity with Von Mises yield criterion has been considered also by Rannacher & Suttmeier (1998). As a special case of the control of output of interest, they obtain an \textit{a posteriori} error estimate of the energy norm of the error on the stresses via duality argument applied to a linearized dual problem. Starting from the non–linear variational equations which describe the regularized version of the Hencky problem and its corresponding finite element approximation, they obtain the non–linear Galerkin orthogonality relation

$$
\int_{\Omega} \left[ P(\nabla_s u_\mu) - P(\nabla_s u_{\mu,h}) \right] : \nabla \eta_h \, d\Omega = \quad (2.37)
$$

$$
= \int_0^1 \int_{\Omega} P'(\nabla (s u_\mu + (1 - s) u_{\mu,h})) \nabla (u_\mu - u_{\mu,h}) : \nabla \eta_h \, d\Omega \, ds = 0 \quad \forall \eta_h \in V_h
$$

where the mean integral theorem has been invoked and $P'$ is the tangent stiffness matrix sampled at $\nabla (s u_\mu + (1 - s) u_{\mu,h})$ with $s \in ]0,1[$. By computing the linearization (2.37) at $u_{\mu,h}$, they consider the solution of the following linear dual problem

$$
\begin{align*}
\text{Find } \varphi \in V \\
\int_0^1 \int_{\Omega} P'(\nabla u_{\mu,h}) \nabla \eta_h : \nabla \varphi \, d\Omega \, ds = J(\eta) \quad \forall \eta \in V,
\end{align*}
$$

where $J(\eta)$ is the output functional of interest which is taken equal to

$$
J_E(\eta) = \|\|e\|\|^{-1} \int_{\Omega} C \nabla_s \eta : \nabla e \, d\Omega
$$

in the case of control of the error in energy norm $\|\| \cdot \|\|$ given by equation (2.3). Following standard arguments, finally, they obtain

$$
\|\|u_\mu - u_{\mu,h}\|\| \leq \sum_{\Omega_e \in T_h} \omega_{\Omega_e} \rho_{\Omega_e}
$$

with the local residuals defined by

$$
\rho_{\Omega_e} = h_{\Omega_e} \|r_{\sigma_h}\|_{[L^2(\Omega_e)]^3} + h_{\Omega_e}^{\frac{1}{2}} \sum_{\gamma \subset \partial \Omega_e} \|J_{\gamma_h}\|_{[L^2(\partial \Omega_e)]^3}
$$
whereas the weights are approximated as in the linear case.

Since this estimation technique involves a linearization, the resulting estimate will be valid only asymptotically, that is for \( u_{\mu,h} \) close to \( u_\mu \). Moreover, one is required to provide an approximation for \( e \) in order to define the forcing \( J_\varepsilon(\eta) \) in the dual problem, which can provoke a deterioration of the quality of the bound.

Duality has also been employed to derive \textit{a posteriori} error estimates for nonlinear variational problems. The work of Repin & Xanthis (1996) represents an important contribution in this sense. The authors, indeed, develop a rigorous mathematical analysis based on duality theory of the calculus of variations which leads to the concept of \textit{duality error estimators} for approximations to nonlinear problems defined by a special class of convex functionals. In particular, the theory is presented for the Nadai deformation theory (Nadai, 1937) of hardening elasto–plastic material which gives rise to two variational problems: the primal for displacements and the dual for the stresses,

\[
\forall \tau \in \Sigma_{EQ}, \quad G(\tau) \leq G(\sigma_{ex}) = \sup_{\tau \in \Sigma_{EQ}} G(\tau) = \inf_{v \in V} J(v) \leq J(v), \quad \forall v \in V
\]

where \( J(v) \) corresponds to the potential energy of the elasto–plastic body defined over the affine space \( V \) of the kinematically admissible displacement fields and \( G(\tau) \) corresponds to the complementary energy of the elasto–plastic body defined over the affine space \( \Sigma_{EQ} \) of the statically admissible stress tensor fields. For the definition of the functionals \( J(v) \) and \( G(\tau) \) associated with the material model under consideration we refer to Repin & Xanthis (1996). Likewise Fraeijs de Veubeke (1965), Repin & Xanthis (1996) assume the difference \( E(v, \tau) = J(v) - G(\tau) \) as measure of the energy norm of the approximation error. However, due to the computational cost and difficulty for building equilibrated stress fields \( \tau \in \Sigma_{EQ} \), the functional \( E(v, \tau) \) is extended over the whole space \( \Sigma \) of the stress tensors. As a result, Repin & Xanthis (1996) obtain the following estimate

\[
\frac{1}{2} ||v - u||^2 \leq E(v, \tau) = E_\Lambda(v, \tau) + E_{eq}(\tau).
\]

For the expressions of \( E_\Lambda(v, \tau) \) and \( E_{eq} \) we refer again to Repin & Xanthis (1996). Here, in relation to further developments we note that the analysis of Repin & Xanthis (1996) shows that \( E_\Lambda(v, \tau) = 0 \) if and only if the constitutive equation is satisfied, whereas \( E_{eq}(\tau) = 0 \) if and only if \( \tau \) satisfies the equilibrium equations. For this reason, \( E_\Lambda(v, \tau) \) measures the error in the constitutive law and \( E_{eq}(\tau) \) measures the error in the equilibrium equations. The estimate, which is an extension of the one proposed by Ladevèze & Leguillon (1983) for linear problems, can, therefore, be applied by using the known approximate solution \( v \) of the primal problem and the corresponding stress tensor obtained by the constitutive relation, even though this will not meet the equilibrium equations. However, the actual computation of the estimate, which is developed for any conforming approximation, not necessarily meeting an orthogonality condition, requires the solution of a quadratic minimization problem posed over an infinite dimensional which renders its practical applica-
tion rather difficult. Furthermore, the estimate does not distinguish the elemental contributions, which form the basis of any adaptive process.

An \textit{a posteriori} error estimate for the primal variational formulation of elastoplasticity with linear hardening given in Han & Reddy (1999) is provided by Alberty \textit{et al.} (1999). The \textit{a posteriori} error estimate refers to the finite element approximation of displacement and plastic strain field in the nonlinear incremental boundary value problem obtained within one time step of the backward Euler. With this formulation, the continuous formulation and finite element discretization of the nonlinear incremental boundary value problem can equivalently be expressed as minimization of a Lipschitz-continuous non-smooth convex functionals. By exploiting this property Alberty \textit{et al.} (1999) obtain the following estimate

$$
\| \sigma - \sigma_h \|_{L^2(\Omega)}^2 + \| \epsilon^p - \epsilon^p_h \|_{L^2(\Omega)}^2 + \| \nabla u - \nabla u_h \|_{L^2(\Omega)}^2 \leq C \sum_{\Omega_e \in \mathcal{T}_h} \eta^2_{\Omega_e},
$$

where \( C \) is a constant depending on the hardening modulus, the discrete stress field \( \sigma_h \) is

$$
\sigma_h = C(\nabla u_h - \epsilon^p_h),
$$

whereas the elemental error indicators \( \eta^2_{\Omega_e} \) are given by

$$
\eta^2_{\Omega_e} = h_{\Omega_e}^2 \| r_{\sigma^h} \|_{L^2(\Omega_e)}^2 + \sum_{\gamma \subset \partial \Omega_e} h_{\gamma} \| J^{\gamma}_{\sigma^h} \|_{L^2(\gamma)}^2
$$

which are the same as in pure elasticity. This circumstance is motivated by the observation that the evolution law in the plastic material law (within one time step) is satisfied exactly on each element whence the material law has a vanishing residual. Thus, excluding the error accumulation for progressing time–steps, the only remaining residuals are those produced in the discrete equilibrium conditions.

\subsection*{2.3.2 Analysis of the time discretization error}

The error estimates presented in the previous section, though based on solid theoretical background, by definition do not account for the effects of time discretization. These estimates, indeed, have been developed by looking at the error associated with the finite element approximation of the non–linear incremental boundary value problem obtained from a one–time step discretization of the initial boundary value problem. Therefore, the error estimate cannot take into account the error deriving from the replacement of the rate quantities appearing in the initial boundary value problem with the difference quotients. It is, indeed, this replacement that produces an error, called the time discretization error, which in certain cases may not be negligible.

An inherent difficulty in obtaining a complete \textit{a posteriori} error estimation for the solution of the fully discrete scheme is due to the different nature of discretization: finite difference–type in time and finite element–type in space. With a finite element approximation–type, the error can be linked to the residual associated with
the approximation. This is obtained simply by inserting the approximate solution into the exact equation. With a finite difference approximation–type, on the other hand, it seems impossible to insert the discrete solution into the exact equation and compute the residual. In this case, indeed, the error estimates developed for finite difference approximations of ordinary differential equations are traditionally based on predictor–corrector algorithms. In these algorithms the difference in solutions obtained by schemes with different orders of truncation error is used as rough estimates of the error. This estimate is in turn used to adjust the time step (Gear, 1971; Reiher, 1987; Abbo & Sloan, 1996). An alternative is the estimate proposed by Nochetto et al. (2000) that develop a posteriori error estimates for the backward Euler approximations of a special class of abstract evolution equations in Hilbert space. In their method it is the accuracy of time interpolant functions of the nodal values, such as continuous piecewise linear or discontinuous piecewise constant functions, to be assessed.

This unsatisfactory state of affair, however, does already become clear in the development of a posteriori error estimates for parabolic equations which represent the simplest class of evolution problems. For this reason, it seems appropriate to first recall briefly some works for parabolic problems. This is considered useful in relation to the evolution of elastoplastic systems.

Eriksson & Johnson (1991) develop an adaptive algorithm for the heat equation based on a posteriori error estimates of approximations obtained by the discontinuous Galerkin method which is based on space–time discretization. In the context of the error analysis, one of the advantages of using the discontinuous Galerkin method is the availability of a unique global variational structure which provides directly the approximation in space and in time. In this manner, one can represent the error in terms of the residual produced by the approximation in the global variational formulation of the problem which will, therefore, account for the discretization effects in space and time (Estep et al., 2000). The use then of a space–time duality argument allows one to realize the control of the quantity of interest. The main idea of this technique, which extends to time dependent problems the procedure developed for elliptic problems, can be comprehended by considering the following abstract evolution equation in the Hilbert space \( \mathcal{H} \) (Nochetto et al., 2000)

\[
\begin{align*}
\text{Find } u : t \in [0, T] &\to \mathcal{H} \\
\dot{u} + \mathcal{F}(u) &= 0 \\
u(t = 0) &= u_0
\end{align*}
\]  

where \( \mathcal{F} : \mathcal{H} \to \mathcal{H} \) is a given (nonlinear) operator of \( \mathcal{H} \) in \( \mathcal{H} \) which is Fréchet–
differentiable in $\mathcal{H}$. Let $\mathcal{R}$ be the residual

$$\mathcal{R} = -\dot{U} - \mathcal{F}(U), \quad (2.39)$$

where $U$ is the approximate solution. By adding (2.38) and (2.39), we obtain the error equation for $e = u - U$

$$\dot{e} + \mathcal{U}e = \mathcal{R} \quad (2.40)$$

where

$$\mathcal{U} \overset{\text{def}}{=} \int_0^1 D\mathcal{F}[su + (1 - s)U]ds. \quad (2.41)$$

and $D\mathcal{F}(\bar{u})$ is the Fréchet derivative of $\mathcal{F}$ at $\bar{u} \in \mathcal{H}$. Multiplying (2.40) by $\varphi \in C^1([0,T];\mathcal{H})$ and integrating by parts over $[0,T]$ delivers the error representation formula

$$\langle e(T), \varphi(T) \rangle = \langle e(0), \varphi(0) \rangle + \int_0^T \langle e, \dot{\varphi} - \mathcal{U}^* \varphi \rangle dt + \int_0^T \langle \mathcal{R}, \varphi \rangle dt \quad (2.41)$$

where $\mathcal{U}^*$ is the adjoint of $\mathcal{U}$. The a posteriori error estimate follows then by selecting $\varphi$ in (2.41) as the solution of the backward dual problem

$$\begin{cases}
\text{Find } \varphi(t) : t \in [0,T] \to \mathcal{H} \\
\dot{\varphi} - \mathcal{U}^* \varphi = 0 \\
\varphi(T) = e(T)
\end{cases}$$

and using strong stability properties of $\varphi$, such as bound for $\dot{\varphi}$, for evaluating the initial error and the residual terms.

The implementation of these ideas to the discontinuous Galerkin finite element method $dG(0)cG(1)$, which combines continuous piecewise linear polynomial approximation in space with discontinuous piecewise constant polynomial approximation in time (which then reduces to the backward Euler with the rectangle rule applied to the integrals) delivers the estimate (Eriksson et al., 1996)

$$\|u(t_N) - U_N\|_{L^2(\Omega)} \leq L_N C_i \max_{1 \leq n \leq N} \left( \|h_n^2 R_2(U)\|_{I_n} + \|h_n^2 f\|_{I_n} + \|h_n^2 [U_{n-1}]\|_{L^2(\Omega)} \right)$$

\[ (2.42) \]

\[ \text{Let } \mathcal{X} \text{ and } \mathcal{Y} \text{ be Banach spaces and let } x_0 \text{ be a point in } \mathcal{X}. \text{ Let } \mathcal{F} \text{ be a mapping from a neighborhood } I_{x_0} \text{ of } x_0 \text{ into } \mathcal{Y}. \text{ Then } \mathcal{F} \text{ is called differentiable at } x_0 \text{ if there exists a linear and continuous operator } A \text{ with the property that } \mathcal{F}(x) = \mathcal{F}(x_0) + A(x - x_0) + g(x), \forall x \in I_{x_0} \text{ and } \lim_{x \to x_0} \|g(x)\|_{\mathcal{Y}} = 0. \text{ If such an } A \text{ exists, we call it Fréchet derivative of } \mathcal{F} \text{ at } x_0 \text{ and is usually denoted with } D\mathcal{F}[x_0] \text{ (Renardy & Rogers, 1996).} \]

\[ \text{Since } \mathcal{F} \text{ is Fréchet differentiable, one can apply the chain rule } \frac{d}{ds} \mathcal{F}(su + (1 - s)U) = D\mathcal{F}[su + (1 - s)U](u - U) \text{ and integrating both sides from } 0 \text{ to } 1 \text{ one finally obtains } \mathcal{F}(u) - \mathcal{F}(U) = \int_0^1 D\mathcal{F}[su + (1 - s)U]ds \ (u - U). \]
where
\[
\|\cdot\|_{I_n} = \max_{t_{n-1} \leq t \leq t_n} \|\cdot\|_{L^2(\Omega)},
\]
\[u(t_N) = u(\bullet, t_N),\]
\[C_i \text{ and } L_N \text{ are constants depending on } k_n = t_n - t_{n-1} \text{ and } t_n,\]
\[R_2(U) = \max_{\gamma \subset \partial \Omega^e} \frac{\|\nabla U\|_{L^2(\gamma)}}{h_n},\]
\[\Omega^e_n \in \mathcal{T}_{h_n}, \text{ with the finite element partition } \mathcal{T}_{h_n} \text{ associated with } I_n = [t_{n-1}, t_n],\]
\[h_n \text{ meshsize of } \mathcal{T}_{h_n},\]
and the starred term is different from zero only in presence of change between non–embedded finite element meshes. In equation (2.42) one can distinguish the terms that measure the residual error of the space discretization from those that measure the residual error of the time discretization. Among the latter, we consider further the starred term. This can be interpreted as error of the initial data of the single one step problem due to different interpolation assumption: in presence of non–embedded finite element spaces, \(V_{h-1} \not\subset V_{h_n}\), the \(L^2\) projection of \(U_{n-1} \in V_{h-1}\) onto \(V_{h_n}\) is different from \(U_{n-1}\).

Phase change phenomena represent a class of problems interesting to consider for they share some common features with the evolution of an elastoplastic medium, such as the presence of variational equations and differential inclusions in the governing equations. In the problem studied by Chen et al. (2000b), for example, we have
\[
\begin{align*}
\text{Find } & \theta, \chi \\
\frac{\partial \theta}{\partial t}, \eta + \frac{\partial \chi}{\partial t}, \eta + \langle \nabla \theta, \nabla \eta \rangle = \langle f, \eta \rangle, & \forall \eta \\
\theta & \in \epsilon \frac{\partial \chi}{\partial t} + \Lambda(\chi)
\end{align*}
\]
where \(\epsilon\) is a small scalar parameter, \(\theta\) stands for the temperature of a substance that occupies the domain \(\Omega\) and undergoes solidification, \(\chi\) is the phase variable and \(\Lambda\) is a multivalued operator given by the inverse of the sign function. The primary variables \(\theta\) and \(\chi\) are approximated with continuous piecewise linear finite elements in space and the backward Euler method is employed for the time discretization.

In the error analysis carried out by the same authors, time interpolation functions of the nodal values of \(\chi\) and \(\theta\), and also the constant interpolant function \(\bar{\theta}(t) = \theta_n\), with \(t_{n-1} < t \leq t_n\) are employed in the definition of the error. Starting from a representation of this error, the authors finally derive an estimate which is expressed as sum of several contributions. These include the term associated with the jump residual, the internal residual, the coarsening, the initial error, the time residual, the quadrature and the error on the data.

The extension of the previous procedures to the control of the error in the displacement formulation of an elastoplastic continuum is not straightforward. This is
due mainly to two reasons. One is the high nonlinearity involved in the elastoplastic behaviour while the other is the use of primary variables, such as displacements, which do not appear in rate form in the formulation of the problem. Consequently, studies on the global control of the error are rare in the current literature. Rannacher & Suttmeier (1999) present a fairly complete analysis for the mixed–dual formulation of the quasi–static Prandtl–Reuss model defined by the following equations

\[ \begin{align*}
\text{Find: } & \sigma \in \Pi W, \ v = \dot{u} \in V \\
& \langle \sigma, \nabla \varphi \rangle = \langle b, \varphi \rangle, \ \forall \varphi \in V \\
& \langle \sigma - C \nabla_s v, \sigma - \tau \rangle \geq 0, \ \forall \tau \in \Pi W
\end{align*} \] (2.43)

where \( V = \{ v \in [L^2(\Omega)]^3 : v = 0 \text{ on } \partial \Omega_d \} \) and \( \Pi W = \{ \tau \in W : ||\tau^D|| - \sigma_y \leq 0, \text{ a.e. in } \Omega \} \), with \( W = [L^2(\Omega)]^{3 \times 3} \).

After performing time discretization with the backward Euler method, and the introduction of the nonlinear operator \( P \) defined in (2.32), problem (2.43) is transformed into the nonlinear variational equation

\[ \begin{align*}
\text{Data: } & (\sigma_{n-1}^h, u_{n-1}^h) \in W_h \times V_h \\
\text{Find: } & (\sigma_n^h, u_n^h) \in W_h \times V_h \\
& \langle \sigma_n^h - P[\sigma_{n-1}^h + k_n C \nabla_s (u_n - u_{n-1})], \tau \rangle + k_n \langle \sigma_n^h, \nabla \varphi \rangle = \langle b_n, \varphi \rangle, \\
& \forall (\tau, \varphi) \in W_h \times V_h.
\end{align*} \] (2.44)

Equation (2.44) describes a material behaviour of Hencky–type in the case \( k_n = 1 \), \( \sigma_{n-1} = 0 \) and \( u_{n-1} = 0 \) which is discretized with standard finite elements in space as follows

\[ \begin{align*}
\text{Data: } & (\sigma_{n-1}^h, u_{n-1}^h) \in W_h \times V_h \\
\text{Find: } & (\sigma_n^h, u_n^h) \in W_h \times V_h \\
& \langle \sigma_n^h - P[\sigma_{n-1}^h + k_n C \nabla_s (u_n^h - u_{n-1}^h)], \tau^h \rangle + k_n \langle \sigma_n^h, \nabla \varphi^h \rangle = \langle b_n, \varphi^h \rangle, \\
& \forall (\tau^h, \varphi^h) \in W_h^h \times V_h^h.
\end{align*} \] (2.45)

For the control of the global stress error with respect to the energy norm, \( ||e_\sigma||_E = \langle e_\sigma, C^{-1} e_\sigma \rangle \), in order to account for effects of time and space discretization Rannacher & Suttmeier (1999) consider the split of the total error into the three components

\[ \sigma(t_n) - \sigma_n^h = \sigma(t_n) - \sigma_n + \sigma_n - \tilde{\sigma}_n + \tilde{\sigma}_n - \sigma_n^h \] (2.46)

where

- \( \sigma(t) \) is the exact solution of problem (2.45),
- \( \sigma_n \) is the solution of problem (2.44) with data \( \sigma_{n-1} \) and \( u_{n-1} \),
• $\tilde{\sigma}_n$ is the solution of problem (2.44) but with data $\sigma_{n-1}^h$ and $u_{n-1}^h$,
• $\sigma_n^h$ is solution of problem (2.45), thus
• $e_{n,\sigma}$ is the error produced by time discretization only,
• $\tilde{e}_{n,\sigma}$ is the error due to change of data in the solution of (2.44),
• $e_{n,\sigma}^h$ is the error produced by space discretization only.

While for $e_{n,\sigma}^h$, Rannacher & Suttmeier (1999) provide an \textit{a posteriori} estimate which is the same as for the discretization error of the Hencky problem, for the other two terms only a rough \textit{a priori} estimate is given. In particular, for $e_{n,\sigma}$ the following bound is obtained

$$
\max_{0 \leq n \leq N+1} \|e_{n,\sigma}\|_E \leq \|e^0_{\sigma}\|_E + T \max_{1 \leq n \leq N+1} \left\{ k_n \max_{t \in [t_n, t_{n+1}]} \|\tilde{\sigma}\|_E \right\}
$$

which shows that the error due to the incremental loading process grows at most linearly with time provided that the exact solution stayed bounded. As for the error $\tilde{e}_{n,\sigma}$ related to the use of inexact starting values in each incremental loading step, this depends on the stability properties of problem (2.44). Rannacher & Suttmeier (1999) give the following estimate

$$
\max_{1 \leq n \leq N+1} \|\tilde{e}_{n,\sigma}\|_E \leq \|\tilde{e}^0_{\sigma}\|_E + \max_{0 \leq n \leq N} \left\{ \|e_{n,\sigma}\|_E \right\}
$$

which assumes an unchanged propagation of the full size of the error of the data.

The error estimate is finally obtained by combining all the previous results. Nevertheless, the adaptive algorithm given in Rannacher & Suttmeier (1999) is based only on the estimate of the term $e_{n,\sigma}^h$, for the effects of time discretization are neglected.

The same authors set up also a theoretical framework for \textit{a posteriori} error analysis for the time discretization error based on duality. To this end, the solution of (2.45) is seen as arising from the use of a space–time approximation which uses discontinuous Galerkin method $dG(0)$ for the time discretization and standard finite element for the discretization in space. Further to a space–time duality argument on the linearized equation that defines the error the following estimate is obtained

$$
\|\sigma(T) - \sigma_{N+1}^h\|_E \leq \sum_{n=1}^{N+1} k_n \sum_{\Omega_n^e \in T_{h_n}} h_{n,\Omega_n^e}^2 \left\{ (\rho_{\Omega_n^e}^2 + k_n^2 \sum_{i=1}^4 \rho_{\Omega_n^e,\omega_{\Gamma_n^i}}^2) \right\}
$$

where the local residuals are given by

$$
\rho_{\Omega_n^e} = h_{n,\Omega_n^e}^{-1} \kappa_n \int_{t_{n-1}}^{t_n} \|R_1\|_{L^2(\Omega_n^i)}^3 \, dt, \quad \rho_{\Omega_n^e} = h_{n,\Omega_n^e}^{-2} \kappa_n \int_{t_{n-1}}^{t_n} \|J_{\sigma_n^h}^2\|_{L^2(\gamma)}^3 \, dt,
$$

$$
\rho_{\Omega_n^e} = h_{n,\Omega_n^e}^{-1} \kappa_n \int_{t_{n-1}}^{t_n} \|r_{\sigma_n^h}\|_{L^2(\Omega_n^i)}^3 \, dt, \quad \rho_{\Omega_n^e} = h_{n,\Omega_n^e}^{-1} \kappa_n \int_{t_{n-1}}^{t_n} \|C^2[\sigma_n^h]\|_{L^2(\Omega_n^i)}^3 \, dt.
$$
with
\[ R_1 \overset{\text{def}}{=} \dot{\sigma}^h + \frac{1}{\mu}(\sigma^h - P\sigma^h) - \nabla_s v_h, \]
whereas the respective weights are obtained by the solution of the linear continuous dual problem. Here, it is interesting to note that the term \( R_1^{n,t} \) reflects the low order regularity of the time constant piecewise function due to the presence of the discontinuity jump across the time instant \( t_n \).

A similar approach has also been adopted by Larsson et al. (2001) for the control of quantity of interest in the space–time discretization of viscoplasticity. Also here, the latter is realized with space–time finite elements that are based on the use of the discontinuous Galerkin method for the time discretization and standard finite elements for the space discretization. This formulation gives quite naturally the variational setting for the analysis of the global error. Consequently, the general principles of duality given at the beginning of this Section can be applied.

Both the theoretical analysis of Rannacher & Suttmeier (1999) and the study of Larsson et al. (2001) share the same limits. These are inherent to the technique of error estimation based on duality when it is applied to nonlinear problems. One limit comes from the need of linearization of the problem that defines the error, so that the eventual estimate has only an asymptotic character. Another one comes from the need of solving exactly the continuous dual problem. Finally, the lack of a sufficiently developed theory of problems of elasto–plasticity does not allow a complete regularity study, which is very important in the analysis of the weights. All this hampers the practical use of the method.

**Remark 2.2.** The discontinuous Galerkin method dG(0) applied to time discretization is the backward Euler method with \((\bullet)_n^\mp = (\bullet)_n^\pm\) and the rectangle rule applied to the integrals. This method, however, does not accommodate the case of using different starting values, that is, \((\bullet)_n^\mp \neq (\bullet)_n^\pm\). On the other hand, the use of the discontinuous Galerkin method dG(1) in place of dG(0), though it has \((\bullet)_n^\mp \neq (\bullet)_n^\pm\), would produce a different time scheme different from the backward Euler.

Following an heuristic argument based on the comparison of the Prandtl–Reuss elastoplasticity with Hencky’s plasticity, Barthold et al. (1997, 1998) split the total error into spatial and time discretization errors. The error associated with space discretization is captured by three terms. The first term is related to the lack of equilibrium and likewise in the work of Johnson & Hansbo (1992) this term is given by
\[ \eta_{eq}^2 = \sum_{\Omega_e \in \mathcal{T}_h} \left\{ C_1 h^2 \| r_{\sigma^h} \|_{(L^2(\Omega_e))^3}^2 + C_2 h^2 \sum_{\gamma \subset \partial \Omega_e} \| j_{\sigma^h} \|_{(L^2(\gamma))^3}^2 \right\} \]
with \( C_1 \) and \( C_2 \) interpolation constants. The second term is due to the violation of the consistency condition at points of the domain other than the Gauss points, as a result of the interpolation of the variables. This term has been introduced in Barthold et al. (1996) and is given by
\[ \eta_{KT}^2 = \| \dot{\lambda} f(\sigma, \alpha) - \dot{\lambda}_h f(\sigma_h, \alpha_h) \|_{L^2(\Omega)} = \| \dot{\lambda}_h f(\sigma_h, \alpha_h) \|_{L^2(\Omega)} \]
where \( f(\sigma_h, \alpha_h) \) denotes the yield function sampled at \( (\sigma_h, \alpha_h) \) with \( \alpha_h \) being the kinematic–internal variable and \( \dot{\lambda}_h \) the plastic multiplier, which are both obtained from the finite element computation. Finally, the last term is the error in the plastic dissipation given by

\[
\eta^2_{\text{dis}} = \| \sigma : (\dot{\varepsilon}^p - \dot{\varepsilon}_h^p) \|_{L^2(\Omega)}^2 + \|H(\dot{\alpha} - \dot{\alpha}_h)\|_{L^2(\Omega)}^2
\]

where \( H \) is the hardening modulus.

The time discretization error, on the other hand, is due to the numerical integration of the flow rule. The error produced within the time interval \([t_n, t_{n+1}]\) is defined as

\[
\eta^2_{\Delta t} = \| \sigma(t_{n+1}) \|_{L^2(\Omega)}^3 \int_{t_n}^{t_{n+1}} \dot{\varepsilon}^p \, dt - \Delta \varepsilon^p \|_{L^2(\Omega)}^3 \quad (2.47)
\]

where the weight \( \| \sigma(t_{n+1}) \|_{L^2(\Omega)}^3 \) has been added by Barthold et al. (1997) in order to be unit consistent with the space discretization error. Further to plausible assumptions, (2.47) is estimated by the maximal change of the normal (with respect to the yield function) between two time (load) steps. It is indeed this result to be interpreted physically as measure of the deviation of Prandtl–Reuss plasticity from Hencky plasticity in the current time step.

However, the aforementioned error measure appears to rely more on heuristic considerations motivated by the physics of the phenomenon which is not met, rather than arising from a theoretical argument typical of an error analysis. Furthermore, the computation of the several terms is realized by assuming recovered post–processed solutions in place of the unknown exact values. As a result, the estimation technique lends itself to the same type of criticism as for the recovery based error estimators.

A family of error measures with clear physical meaning and capable to account in a simple manner for effects of time and space discretization is represented on the other hand by the error in the constitutive equations, which is the objective of the next section.

### 2.3.3 Error in the Constitutive Equations

The error in the constitutive equations which has been applied successfully for the assessment of the accuracy of conforming approximations to linear problems is extended in a natural way to nonlinear dissipative problems of evolution.

Fundamental notion of this class of error measures is the definition of the so–called admissible solution. This is a particular approximate solution of the given initial boundary value problem. Its definition depends on the conditions and equations which are satisfied, whereas its quality is assessed in terms of the conditions and equations that are not. The application, then, of this theory to assess the quality of a finite element solution consists in building a corresponding admissible solution that reflects the approximations associated with the finite element solution.
The first error measure which exploits these concepts is the one based on the Drucker’s inequality (Drucker, 1964). The definition of this error is due to Ladevèze (1985) in the context of the nonincremental method LATIN applied to the solution of the evolution of elastoviscoplastic materials which follow the conditions of Drucker’s stability. This error measure was then further investigated in Coffignal (1987) and applied in Ladéveze et al. (1986) to the control of incremental finite element solutions of the same class of problems.

The error in the constitutive equations based on Drucker’s inequality applies to material models defined by the following functional constitutive equations

$$\sigma(t) = A(\epsilon(\tau), \tau \leq t), \quad (2.48)$$

where $A$ denotes the constitutive operator which is function of the strain history $\epsilon$. In particular, the material model is supposed to obey to the Drucker’s inequality. This property of the model can be formulated as follows (Bussy & Remond, 1985; Ladevèze & Pelle, 2001). Consider any pair of stress-strain history $(\sigma(t); \epsilon(\tau), \tau \leq t)$ and $(\tilde{\sigma}(t); \tilde{\epsilon}(\tau), \tau \leq t)$ which meet $\forall t \in [0,T]$ the constitutive equation (2.48) and the initial conditions, which are assumed equal to zero. The constitutive model defined by the operator $A$ is called stable according to Drucker if the following condition is satisfied

$$\forall (\sigma, \epsilon), (\tilde{\sigma}, \tilde{\epsilon}) \text{ meeting equation (2.48),}$$

$$\forall t \in [0,T], \quad \int_{0}^{t} [\sigma(\tau) - \tilde{\sigma}(\tau)] : [\dot{\epsilon}(\tau) - \dot{\tilde{\epsilon}}(\tau)] \, d\tau \geq 0,$$

whereas it is strictly stable if also the following additional condition is met

$$\forall t \in [0,T], \quad \int_{0}^{t} [\sigma(\tau) - \tilde{\sigma}(\tau)] : [\dot{\epsilon}(\tau) - \dot{\tilde{\epsilon}}(\tau)] \, d\tau = 0 \Rightarrow \sigma = \tilde{\sigma}, \quad \epsilon = \tilde{\epsilon} \quad (2.50)$$

The error in the constitutive equations based on Drucker’s inequality assesses the quality of admissible solutions $S_{ad} = (\sigma_{ad}, u_{ad})$ which are defined by assuming at any time instant $t \in [0,T]$, a kinematically admissible displacement field $u_{ad}$ and a statically admissible stress field $\sigma_{ad}$. Both $u_{ad}$ and $\sigma_{ad}$ are required to meet the initial conditions at $t = 0$. As a result, $S_{ad}$ is an approximate solution to the initial boundary value problem, for the constitutive equations (2.48) are generally not satisfied. The quality of $S_{ad}$ is characterized by defining first the following two processes meeting the constitutive equation (2.48)

$$\dot{\hat{S}} = (A(\nabla_s u_{ad}(\tau, \tau \leq t)); \epsilon_{ad}(\tau, \tau \leq t))$$

$$\dot{\tilde{S}} = (\sigma_{ad}(t) ; A^{-1}(\sigma_{ad}(\tau, \tau \leq t)))$$

$\dagger$Recall that the restriction of the function $f(\tau)$ to times $\tau$ not later than the current time $t$ is called the history of $f$ up to the time $t$. 
where $A^{-1}$ denotes the inverse operator of $A$, and then computing the following quantity

$$
\epsilon^2_{Drucker}(T) = \sup_{t \leq T} \int_{\Omega} \int_{0}^{t} \left[ (\hat{\sigma}(\tau) - \sigma_{ad}(\tau)) : (\hat{\epsilon}_{ad}(\tau) - \dot{\epsilon}(\tau)) \right] d\tau \, d\Omega \geq 0. \quad (2.51)
$$

Indeed, by means of equations (2.49) and (2.50), one can easily check that $\epsilon_{Drucker}$ meets the following properties

$$
\epsilon_{Drucker} \geq 0
$$

$$
\epsilon_{Drucker} = 0 \Rightarrow \hat{\sigma} = \sigma_{ad} \quad \forall x \in \Omega, \quad \forall t \in [0,T],
$$

that is, $\epsilon_{Drucker}$ can be assumed as measure of the extent to which $S_{ad}$ fails to satisfy the constitutive equations, and therefore of the extent to which $S_{ad}$ fails to coincide with $S_{ex}$, the exact solution of the problem. Elaborating on the expression of $\eta^2(x,t)$ in the case of material models described by internal variables, Ladevèze & Pelle (2001) show that $\eta^2(x,t)$ can be expressed as sum of the error in the state law and the error in the evolution law,

$$
\eta^2(x,t) = \frac{1}{2} \left[ (\sigma - \hat{\sigma}) : C^{-1} (\sigma - \hat{\sigma}) + (X - \hat{X}) : (Y - \hat{Y}) \right]_{t} + \frac{1}{2} \int_{0}^{t} \Gamma d\tau
$$

where $X$ denotes the set of kinematic–type internal variables, $Y$ are the respective thermodynamic conjugate variable and $\Gamma \geq 0$ is related to the residual in the evolution law. Without further details related to the definition of these quantities which will be analysed in Section 3.5.1.2, here, we want just to observe that this decomposition is consistent with the fact that $S_{ad}$ does not satisfy the complete constitutive equations. A similar structure of the error will be noted also in the extended dissipation error introduced in Ladevèze et al. (1999) for material models with internal variables which admit a standard formulation (Halphen & Nguyen, 1975). For this new measure of error, the admissibility conditions include the kinematic compatibility relations, the equilibrium equations and the initial conditions. As a result, the only equations left apart are the complete constitutive equations, i.e. the state laws and the evolution laws.

The error measure (2.51) has a global character in time and space. Furthermore, due to its definition, it is not possible to distinguish the contribution to the error arising from time and space discretization, thus it cannot be used to drive an adaptive process. To remedy this, Gallimard (1994) and Gallimard et al. (1996) have applied the concept of error in the constitutive equations to the problems obtained by using only time and space discretization, respectively. In this way, the authors have obtained error indicators that separate the effects of time and space discretization, respectively, which then have been used to control the discretization process.

39
The first use of the error in the constitutive equations to constitutive models described in terms of internal variables and having an associative flow rule is due to Ladevèze (1989). The admissibility conditions combine the kinematic compatibility relations, the equilibrium equations, the state law and the initial conditions. The only equation which is left out is the evolution law that governs the dissipative phenomenon. By exploiting the convexity structure of these equations, Ladevèze (1989) introduces, therefore, the concept of dissipation error given by the residual in the evolution law, which is appropriately reformulated in terms of the dissipation pseudo-potential and its Legendre–Fenchel transform. The theory of this error will be detailed in Section 3.5.1.1.

Likewise the error measure based on Drucker’s inequality, the dissipation error has then applied in Moës (1996) and Ladevèze & Moës (1997) to assess the accuracy of incremental finite element solutions of associative problems. Furthermore, error indicators which separate the sources of the different discretizations have been defined and used to drive the adaptive process in time and space.

As we have already mentioned, another measure of the error in the constitutive equations has been introduced in Ladevèze et al. (1999) for material models described by internal variable and admitting standard formulation. The new measure is an extension of the dissipation error obtained by removing the state laws from the admissibility conditions. Applications of this error measure have been given for an elastic-damage coupled model in Ladevèze et al. (1999) solved with the nonincremental LATIN method and to the Prandtl–Reuss plasticity model in Orlando & Peric (2000) solved with the classical incremental finite element method. The extended dissipation error will be detailed in Section 3.5.1.2 and will represent the tool employed to analyse the effects of change of finite element mesh in classical finite element incremental solution of elasto–plastic problems.

The measures of the error in the constitutive equations given in this section apply to associative material models. Their key property is the characterization of the constitutive equations as solution of a scalar equation defined by convex scalar potentials. If the equation is not satisfied, it assumes positive value. This same characterization of the constitutive equations has also been given by de Saxcè (1992) to a much larger class of materials, such as the non–associated model of cyclic–plasticity of Chaboque–Marquis and the non–associated model of Drucker–Prager, which are termed implicit standard models. The characterization is based on the introduction of a unique potential, function of the rates and the associated force, which is used by Ladevèze (1999) to extend the notion of dissipation error also to this class of material models, whereas applications of the new measure of error have been given in Hjiaj (1999).

2.3.4 Heuristic Error Indicators

Lately, the emergence of new supercomputer architectures has allowed large-scale simulations of engineering problems and the incorporation of more detailed physics in the model. This has led to a substantial increase in the complexity of the simulation
process that also incorporates non-linear mathematical models. As a result, adaptive strategies for the approximate solution of these problems are often based on heuristic error indicators or adaption indicators. A physically based argument is generally adopted as justification of their definition, which is typical for the problem under consideration. The motivation in using error indicators is that their primary quality is to be able to distinguish the elements and time steps which contribute mostly to the global accuracy of the approximation, even though they finally may fail to yield accurate global estimates. As a result, they represent a tool for the adaptive construction of the approximation, that is, they may be used as a guide for a sequence of discrete choices (to refine or not to refine a given element; to reduce or not to reduce a given time step) and not for the definition of the stopping criterion which should be based on an error estimate. It also appears clear that works in this area are much more abundant than in that of the development of error estimate. In the following we give only a brief overview of some error indicators that have been employed in practice.

The recovery based error estimate introduced by Zienkiewicz & Zhu (1987) has been applied in Zienkiewicz et al. (1988) to assess the finite element approximation of the flow formulation of incompressible plastic flow in which elastic effects are neglected. The above authors observe that in extrusion problems the energy norm has a definite meaning as it is simply the error in the rate of energy dissipation. Consequently, the total energy dissipation norm is defined as

$$||u|| = \left[ \int_{\Omega} \left( \sigma : \dot{\epsilon} \right) \, d\Omega \right]^{\frac{1}{2}}$$

where $u$ is the displacement field, and the error $e = u - u_h$ as

$$||e|| = \left[ \int_{\Omega} \left( \sigma - \sigma_h \right) : \left( \dot{\epsilon} - \dot{\epsilon}_h \right) \, d\Omega \right]^{\frac{1}{2}}.$$

Since the basic formulation is mixed and the pressure variable is introduced as a means of ensuring incompressibility, Zienkiewicz et al. (1988) consider the energy norm of the deviatoric part of $\sigma$, whose constitutive equation is

$$\sigma^D = \mu D \dot{\epsilon}$$

where $D$ is an appropriate material tensor and $\mu$ is the viscosity which in general is dependent on the strain rate.

As a result, the energy norm of the error is written as

$$||e|| = \left[ \int_{\Omega} \left( \sigma^D - \sigma^{h,D} \right) : \left( \mu D \right)^{-1} \left( \sigma^D - \sigma^{h,D} \right) \, d\Omega \right]^{\frac{1}{2}}. \quad (2.52)$$

The estimate of the measure of the error defined by equation (2.52) is obtained by replacing the exact solution $\sigma^D$ with an approximation $\sigma^{D,*}$ of higher order to
that given by the finite element solution and obtained with the recovery technique described in Zienkiewicz & Zhu (1987).

This error indicator by its definition accounts for only effects of space discretization. It is thus used as basis for the refinement of the finite element mesh in the current time step. Furthermore, it does not account for the error in the incompressibility condition, which is enforced in a weak sense in the mixed formulation.

A posteriori error estimates based on the Zienkiewicz–Zhu adaptive strategy and the energy norm have been appropriately modified by Perić et al. (1994) to account for the elastoplastic deformation of the conventional and Cosserat continuum model. The corresponding error estimates are based on the rate of plastic work and on the plastic dissipation. After introducing the following definitions of error,

\[ e_{W_p} = \left[ \sum_{\Omega_e \in T_h} \int_{\Omega_e} \left| (\sigma - \sigma^h) : (\dot{\epsilon}^p - \dot{\epsilon}^p_h) \right| d\Omega \right]^{\frac{1}{2}}, \]

\[ e_{\mathcal{D}} = \left[ \sum_{\Omega_e \in T_h} \int_{\Omega_e} \left| (\sigma - \sigma^h) : (\dot{\epsilon}^p - \dot{\epsilon}^p_h) + (X - X^h) : (\dot{\alpha} - \dot{\alpha}^h) \right| d\Omega \right]^{\frac{1}{2}}, \]

where \( \alpha \) denote the set of kinematic–type internal variables and \( X \) the respective conjugate thermodynamics forces, estimates for \( (2.53)_1 \) and \( (2.53)_2 \) are obtained by simply replacing the exact values with postprocessed solutions of the finite element approximation. The error based on the dissipation functional, in particular, has been then employed as basis for the finite element adaptive solution of a strain localization problem. The latter has been described by resorting to the Cosserat continuum in order to overcome serious limitations exhibited by classical continuous models in the post instability region. The expression of the estimate has been particularized for the classical model of \( J_2 \)-elastoplasticity which is generalized by introducing additional degrees of freedom within the Cosserat continua. In this case it is shown that the expression of the plastic dissipation involves only classical quantities. Assuming time discretization of the evolution problem is performed by the backward Euler algorithm, within a generic time step \( I_n = [t_n, t_{n+1}] \), the a posteriori error estimate \( \epsilon_{\mathcal{D}} \) based on the dissipation functional is given by the following expression

\[ \epsilon_{\mathcal{D}} = \sum_{\Omega_e \in T_h} \int_{\Omega_e} \left[ (\sigma^*_{n+1} - \sigma^h_{n+1}) : \frac{3}{2} \mathbf{P}(\sigma^*_{n+1} - \sigma^h_{n+1}) + \frac{2}{R^*_{n+1} - R^h_{n+1}} \left( \Delta \alpha^*_{n+1} - \Delta \alpha^h_{n+1} \right) d\Omega \right] \]

where \( \mathbf{P} \) is a second order tensor used to represent the yield function in the seven–dimensional stress space for plane strain \( J_2 \)-elastoplasticity within a Cosserat continuum, whereas \( \alpha \) denotes the accumulated plastic strain and \( R \) the conjugate variable. The starred quantities denote the postprocessed solutions. Likewise the previous error estimate, error indicators are simply obtained by localization of the
integrals at each element. These error indicators are, in turn, used only to drive mesh refinement, for they reflect only effects of space discretization.

A comparative evaluation of various error estimators for isotropic, elasto-plastic and viscoplastic solids undergoing large deformations has been carried out in Tetambe et al. (1995). Five distinct error estimators are studied. These are the $L^2$–norm of the stress error,

$$I_1 = \left[ \int_{\Omega} (\sigma - \sigma^h) : (\sigma - \sigma^h) \, d\Omega \right]^{\frac{1}{2}},$$

the $L^2$–norm of the total strain error,

$$I_2 = \left[ \int_{\Omega} (\epsilon - \epsilon^h) : (\epsilon - \epsilon^h) \, d\Omega \right]^{\frac{1}{2}},$$

the $L^2$–norm of the equivalent total strain error,

$$I_3 = \left[ \int_{\Omega} (\epsilon_{eq} - \epsilon_{eq}^h)^2 \, d\Omega \right]^{\frac{1}{2}},$$

where $\epsilon_{eq}$ is an equivalent scalar strain given by an appropriate norm of the total strain $\epsilon$; the $L^2$–norm of the incremental total strain given by

$$I_4 = \left[ \int_{\Omega} (\Delta \epsilon - \Delta \epsilon^h) : (\Delta \epsilon - \Delta \epsilon^h) \, d\Omega \right]^{\frac{1}{2}},$$

and finally the energy rate error norm introduced by Zienkiewicz et al. (1988). As usual, estimates for these measures of error are obtained by simply replacing the exact values with approximations of higher order, obtained by a suitable postprocessing of the finite element approximation. This study shows that the error computed using each of these estimators increases from its initial value as the deformation continues in the plastic zone in a large strain analysis. In particular, the error estimators based on the energy rate and the $L^2$–norm of the incremental strain were able to predict the region with the maximum error.

Since these error estimators are defined in terms of incremental quantities, they mirror to different extent the effects of only space discretization. As a result, they are not able to predict consistently a monotonically increasing error with the deformation.

The adaptive procedures for large–deformation finite element analysis of elastic and elasto–plastic problems implemented by Lee & Bathe (1994) is based on a pointwise indicator for error in stresses and a pointwise indicator error in plastic strain increments. The pointwise error in stress indicator is a pointwise version of the stress smoothing type of indicators. This indicator estimates the error in the
stresses by giving the differences between the unaveraged stress $\sigma^h$ and a smoothed stress $\sigma^\ast$. In particular, only the pressure and the effective stress (which are related to the first and second stress invariant) are considered in the evaluation of the accuracy of the finite element solution. The error in the plastic strain increment tensor is given by

$$\Delta e_p = \Delta e_p^\ast - \Delta e_p^h$$

(2.54)

where

$$\Delta e_p^\ast = \int_{t_n}^{t_{n+1}} \dot{\epsilon}^p dt.$$ 

An estimate of the error (2.54) is computed by considering the $L^2$-norm of the difference between the plastic strain increments obtained using the trapezoidal rule, which is second order accurate, and the Euler backward method. Though this is an indicator on the time discretization error, it is used in Lee & Bathe (1994) as indicator of the space discretization error along with the indicator for the error on the stresses.

In the context of adaptive strategies for problems with localization of deformations Ortiz & Quigley (1991) argue heuristically that localized solutions may reasonably expect to be of bounded variation. The total variation of the solution over each element is thus assumed as a suitable adaptor indicator to drive the adaptive process. For 1D problems, the bounded variation of a scalar function $v = v(x)$ is defined as follows

$$|v|_{BV,[a,b]} = \sup \sum_{k=1}^N |v(x_k) - v(x_{k-1})|$$

(2.55)

where the supremum is taken over all sequences $a = x_0 \leq x_1 \leq \ldots \leq x_N = b$, $\forall N \in \mathbb{N}$. Elements are targeted for refinement when the variation of the solution within each element is determined to be too high for the interpolation to adequately resolve it. Ortiz & Quigley (1991) also show that in 1D problems this error indicator can be derived from interpolation error bounds, that is, the equidistribution of the indicator (2.55) over the mesh is shown to minimize the $L^\infty$ interpolation error.

Use of error indicators based on minimization of interpolation error have also been employed by Demkowicz et al. (1985) and Radovitzky & Ortiz (1999), among others. In these works, the methodology set by Diaz et al. (1983) for grid optimization, that is, for relocation of the nodes within a fixed number of degree of freedom, is employed for mesh refinement. The estimation of the local errors is based on interpolation error bounds and extraction formulas for highly accurate estimates of the derivatives of the exact solution which appear in the bound. In the methodology proposed by Radovitzky & Ortiz (1999), starting with a very fine mesh, the discretization error can be bounded from above by the interpolation error, once a linearised analysis is carried out. In order to compute the size of the exact solution, which appears in the a priori error bound, Radovitzky & Ortiz (1999) consider the error of a lower order approximation $\tilde{\phi}_h^{p-1}$ such that the computed finite element approximation $\phi_h^p$ represents the finite element nodal interpolation of $\tilde{\phi}_h^{p-1}$.
These error indicators are based on \emph{a priori} error estimates, therefore, usually, they deliver crude estimates. However, they present the advantage since they can be applied for those problems where energy estimates based on the residual break down due to the loss of ellipticity of the governing equation. Therefore, they must be used with judgement, for they imply a certain regularity of the exact solution and require accurate techniques to calculate derivatives of the exact solution.

2.4 Concluding remarks

The considerations of the previous sections show that the formal structure of the theory of \emph{a posteriori} error estimation for linear problems is well understood, as opposite to the non linear problems. This mirrors the different maturity achieved in the theoretical analysis of the two classes of problems.

In the class of the linear elliptic problems, the recovery based error estimators represent perhaps the most widely used \emph{a posteriori} estimates in solid mechanics applications for its relatively simple implementation. Also the explicit residual type error estimates are quite easy to use, for they simply require computing norms of the residual, once a certain weight between the jump term and the internal residual has been set. However, the way that the residual influences the error is accounted for by the stability properties of the dual problem, whose solution can pose further difficulties. The implicit residual type estimation of the error, on contrary, deliver much more effective estimates at the fraction of cost of solving additional local finite element problems posed on local higher order finite element spaces. In this class, the error in the constitutive equations developed by Ladevèze & Leguillon (1983) appears to be the most robust.

For nonlinear problems, very few theoretically based error estimates have been developed. In particular, for time dependent problems, the measures of the error in the constitutive equations have clear physical meaning and are able to account for all the sources of discretization. In the finite element solution of elastoplastic problems discretized in time with the backward Euler method, which will be the class of problems hereafter considered, apart from time and space discretization, another source of error needs to be considered. This arises as a result of the change of finite element mesh from one time step to the other, which introduces a discontinuity jump in the time linear interpolation of the discrete values of the solution. As a result, only measures of error that account for time discretization effects can reflect the low order regularity of the approximation across the time $t_n$ when the change of mesh occurs. Thus, the extended dissipation error introduced by Ladevèze \emph{et al.} (1999) naturally lends itself for this aim.

The theoretical analysis of this measure of error and the way it is able to accommodate discontinuity jump in the admissible solution is the main objective of the next Chapter.
Chapter 3

The error in the constitutive equations for dissipative nonlinear problems

3.1 Introduction

In this chapter the general theory to assess the quality of the so called admissible solutions of dissipative nonlinear problems is presented following Ladevèze & Pelle (2001).

After introducing the set of conditions and equations that govern the behaviour of a standard generalized model with internal variables (Halphen & Nguyen, 1975), and noting the dissipative nature of the elastoplastic problem under examination, a simple error analysis of the following first order ordinary differential equation

\[ u'(t) + a(t) u(t) = f(t), \]
\[ u(t = 0) = u_0 \]

with \( a \geq 0 \), is performed. This is done in order to motivate the use of residual of approximate solutions of dissipative problems as indication of the error produced and also to show the influence on the error of the discontinuity jump which is present in the approximate solution at the time instant \( t_n \). An \textit{a posteriori} error estimation analysis is given, which delivers an upper bound expressed in terms of the discontinuity jump.

As introduction to the error in the constitutive equations, the fundamental notion of admissible solution is given. This is a particular approximate solution of the problem under exam. Its definition depends on the conditions and equations which are satisfied, whereas its quality is assessed in terms of the conditions and equations that are not. Consequently, the concepts of dissipation error and extended dissipation error for time continuous admissible solutions, and the one with discontinuity jump at the time instant \( t_n \) are given. A new measure of error in the constitutive equations of rate–independent plasticity models is defined which is called augmented...
extended dissipation error. Finally, a definition of error in solution concludes the chapter.

3.2 The reference continuum problem: The Initial Boundary Value Problem for a model with internal variables

3.2.1 Preliminaries

Consider a generic continuum body \( B \) with boundary \( \partial B \) occupying a regular domain \( \Omega \) of the three dimensional Euclidean space. Let \( \partial B_d \) and \( \partial B_t \) be disjoint parts of \( \partial B \), with \( \partial B_d \cup \partial B_t = \partial B \), where displacements and surface tractions are prescribed, respectively. We will assume displacements to be small in the quasi-static evolutive process of the body so that geometry changes and inertial effects may be neglected. As a result, the analysis will be performed in the reference configuration which is identified with the domain \( \Omega \) while the boundary \( \partial B \) is identified with \( \partial \Omega \). Likewise, the identifications \( \partial B_t \) with \( \partial \Omega_t \) and \( \partial B_d \) with \( \partial \Omega_d \) are assumed, as well.

3.2.2 Equilibrium Equation

Denote with \( S \) the linear space of the symmetric second order stress tensors. The statically admissible stress fields, \( \sigma = \sigma(x,t) \in S \), are such that the weak form of the equilibrium, given by the virtual work, is satisfied, that is,

\[
\int_{\Omega} \sigma : \nabla \eta \, d\Omega = \int_{\Omega} b \cdot \eta \, d\Omega + \int_{\partial \Omega_t} t \cdot \eta \, ds \quad \forall \eta \in V_0, \quad \forall t \in \mathcal{I} = [0,T]
\] (3.1)

where \( b = b(x,t) \) and \( t = t(x,t) \) are respectively the body force and surface traction fields and \( V_0 \) is the linear space of the virtual displacements which vanish on \( \partial \Omega_d \), whereas \( \mathcal{I} \) is the time interval of interest (Gurtin, 1972; Ladevèze, 1999). Whenever necessary, the equilibrium condition will also be written as

\[
\langle \sigma(x,t), \nabla \eta(x) \rangle = \langle b(x,t), \eta(x) \rangle + \langle t(x,t), \eta(x) \rangle_{\partial \Omega_t},
\]

with \( \langle \cdot, \cdot \rangle \) and \( \langle \cdot, \cdot \rangle_{\partial \Omega_t} \) denoting integrals over the domain \( \Omega \) and the boundary \( \partial \Omega_t \) where traction conditions have been assigned, respectively.

3.2.3 Compatibility Equations

The compatibility equations refer to the displacements and the associated deformation. The conditions which define a kinematically admissible displacement field guarantee internal and external compatibility. In mathematical terms, this means that a displacement field \( u = u(x,t) \) is kinematically admissible if it is time continuous, at least once differentiable in space and meets the boundary conditions on
We refer to $V$ as the space of the kinematically admissible displacements.

Let $E$ be the linear space of the symmetric second order strain tensors. The kinematically admissible strain fields, $\epsilon = \epsilon(x, t) \in E$, are continuous fields which are obtained from the kinematically admissible displacement fields $u = u(x, t) \in V$ as follows

$$\epsilon = \frac{1}{2}(\nabla u + \nabla u^T) \overset{\text{def}}{=} \nabla_s u,$$

(3.2)
i.e., $\epsilon$ is the symmetric part of the second order tensor $\nabla u$.

### 3.2.4 Constitutive Equations

#### 3.2.4.1 Thermodynamic Admissibility

In the constitutive modelling with internal variables, which is hereafter adopted, the functional dependence of the present state of the material upon the history of the total strain and temperature (which are the observable variables) is replaced by equations that define the current state of the material in terms of only the current value of both the observable variables and additional variables, called generally hidden or internal variables. They are macroscopic measures of irreversible phenomena (Lemaitre & Chaboche, 1990; Besson et al., 2001). The success of such formulation is due to the simplicity, in general, of the mathematical and numerical analysis of the problem which governs finally the evolution of the whole system.

Our interest, in particular, will focus on constitutive models of rate independent plasticity in isothermal conditions belonging to the class of the so-called standard generalised materials as introduced by Halphen & Nguyen (1975). By exploiting the convex structure of these models, emphasis will be placed especially on scalar equivalent formulations of the tensorial constitutive equations.

The physical validity of a constitutive model is usually expressed by its thermodynamic admissibility (Coleman & Gurtin, 1967; Maugin, 1992), which is obtained by imposing the validity of the Clausius-Duhem inequality. For isothermal processes, the aforementioned inequality represents the difference between the external total power and the rate of variation of the reversible stored energy, that is,

$$-\dot{\psi} + \sigma : \dot{\epsilon} \geq 0,$$

(3.3)

where $\psi$ is the free Helmholtz energy per unit volume defined in terms of the state variables which describe the model, and $\sigma : \dot{\epsilon}$ is the total external power.

The inequality (3.3) is obtained by combining the local form of the first principle of thermodynamics, which expresses the conservation of energy, and the second principle, which refers notably to the direction of irreversible processes.

A thermodynamically consistent model is, therefore, obtained by specifying the state variables along with the functional form of the free Helmholtz energy and the complementary laws which describe the time evolution of the internal variables in respect of (3.3).
3.2.4.2 State variables

In the definition of the state variables, which characterise uniquely the state of the system, we refer only to the so called kinematic type variables whereas the corresponding static one are obtained by duality as introduced by the total energy and the dissipation. In particular, we assume the following: (i) temperature to be constant with time and uniform in space so that it will not be considered hereafter; (ii) that the total strain can be uniquely decomposed additively into its elastic and plastic part, that is,
\[ \epsilon = \epsilon^e + \epsilon^p, \]
\[ \text{(3.4)} \]
and finally (iii) that the local state of the material is described by means of additional internal variables \( \alpha \) which may only include scalar and/or second order tensors (Coleman & Gurtin, 1967; Reddy & Martin, 1994), characterizing the internal changes of the material.

3.2.4.3 Equations of State

The reversible energy density stored under any form in the material is represented by the free Helmholtz energy \( \psi \) which is assumed to depend on the variables \( (\epsilon^e, \alpha) \). In particular, we assume \( \psi(\epsilon^e, \alpha) \) to be expressed as a sum of two proper strictly convex and lower semicontinuous functions\(^3\) of each of its arguments: \( \psi_e(\epsilon^e) \) which

---

\(^{3}\)Let \( \mathcal{V} \) be a linear space. A subset \( K \subseteq \mathcal{V} \) of \( \mathcal{V} \) is convex if \( \forall \mathbf{x}, \mathbf{y} \in K, \forall t \in [0, 1], t\mathbf{x} + (1-t)\mathbf{y} \in K \). Let \( f : \mathcal{V} \to \bar{\mathbb{R}} = \mathbb{R} \cup \{-\infty\} \cup \{+\infty\} \), \( f \) is convex if \( \forall \mathbf{x}, \mathbf{y} \in \mathcal{V}, \forall t \in [0, 1], f(t\mathbf{x} + (1-t)\mathbf{y}) \leq tf(\mathbf{x}) + (1-t)f(\mathbf{y}) \), with the convention rules \(+\infty + (-\infty) = +\infty\), \( 0 \cdot (+\infty) = 0 \cdot (-\infty) = 0 \). The function is strictly convex if the inequality holds strictly. A convex function is proper if it nowhere takes the value \(-\infty\) and is not identically equal \(+\infty\). The epigraph of a convex function, that is, the set \( \text{epi} f = \{ (\mathbf{x}, \alpha) \in \mathcal{V} \times \bar{\mathbb{R}} | f(\mathbf{x}) \leq \alpha \} \) is a convex subset of \( \mathcal{V} \times \bar{\mathbb{R}} \) and so is its effective domain, that is, the set \( \text{dom} f = \{ \mathbf{x} \in \mathcal{V} | f(\mathbf{x}) \in \mathbb{R} \} \). Let \( \mathcal{V} \) be a linear topological space and \( \mathbf{x}_0 \in \text{dom} f \). \( f \) is lower semicontinuous at \( \mathbf{x}_0 \) if \( f(\mathbf{x}_0) \leq \lim \inf_{\mathbf{x} \to \mathbf{x}_0} f(\mathbf{x}) \), where \( \lim \inf_{\mathbf{x} \to \mathbf{x}_0} f(\mathbf{x}) \overset{\text{def}}{=} \inf \{ \alpha \in \bar{\mathbb{R}} | \exists \mathbf{x}_n \to \mathbf{x}_0 \text{ with } f(\mathbf{x}_n) \to \alpha \} \) is the lower limit of \( f \) at \( \mathbf{x}_0 \). A function

---

Figure 3.1: Energy repartition for a linear elastic and linear isotropic hardening model
is the stored energy due to elastic strain and \( \psi_p(\alpha) \) which is the stored energy due to plastic and internal parameters related to hardening effects (Nguyen, 2000), cf. Figure 3.1, i.e.,

\[
\psi(e^e, \alpha) = \psi_e(e^e) + \psi_p(\alpha).
\]

By expanding the Clausius-Duhem inequality which is required to hold for any admissible thermodynamic process \((e, e^p, \alpha)\) (Coleman & Gurtin, 1967), we obtain the state equations

\[
\sigma = \frac{\partial \psi_e}{\partial e^e}(e^e), \quad A = \frac{\partial \psi_p}{\partial \alpha}(\alpha),
\]

and the associated intrinsic mechanical dissipation

\[
\sigma : \dot{e}^p - A : \dot{\alpha} \geq 0.
\]

The state equations (3.5) are also called state laws and in particular we refer to (3.5) as the elastic law, whereas we refer to (3.5) as the hardening law. The force-type variable \( A \), defined by the hardening law (3.5), is termed the thermodynamic force conjugate to \( \alpha \) (Lemaitre & Chaboche, 1990; Besson et al., 2001). If the functions \( \psi_e = \psi_e(e^e) \) and \( \psi_p = \psi_p(\alpha) \) are not differentiable, then the concept of subgradient \(^\dagger\) is introduced (Germain et al., 1983).

For our subsequent developments, it is useful to consider the following equivalent formulation of the state equations (3.5) (Germain et al., 1983; Ladevèze & Pelle, 2001),

\[
\psi_e(e^e) + \psi_e^*(\sigma) - \sigma : e^e = 0 \iff \sigma - \frac{\partial \psi_e}{\partial e^e}(e^e) = 0,
\]

\[
\psi_p(\alpha) + \psi_p^*(A) - A : \alpha = 0 \iff A - \frac{\partial \psi_p}{\partial \alpha}(\alpha) = 0,
\]

where \( \psi_e^*(\sigma) \) and \( \psi_p^*(A) \) are the conjugate functions or Legendre-Fenchel transforms of \( \psi_e(e^e) \) and \( \psi_p(\alpha) \), respectively, cf. Figure 3.2. These are defined as

\[^\dagger\] Let \( V \) and \( V^* \) be two linear topological spaces placed in duality by the separating bilinear form \( \langle \cdot, \cdot \rangle_{V^*, V} \). Given a real valued convex function \( f : v \in V \to \mathbb{R} \) and \( v_0 \in V \), if there exists a vector \( a \in V^* \) such that \( f(v) - f(v_0) \geq \langle a, v - v_0 \rangle \), \( \forall v \in V \), \( f \) is said to be subdifferentiable at \( v_0 \) and \( a \) is referred to as a subdifferential of \( f \) at \( v_0 \). The set of vectors \( a \in V^* \) which satisfy the previous property is referred to as the subgradient of \( f \) at \( v_0 \) and is denoted with \( \partial f(v_0) \). One can easily show that \( \partial f(v_0) \subseteq 2^{V^*} \) is convex and closed in \((V^*, \sigma(V^*, V))\). The above definition of subdifferential \( \partial f(\cdot) \) of a convex function \( f \) has many of the useful properties of the derivative. If the function \( f \) is not convex, the aforementioned definition of subdifferential is not a particularly helpful idea. Thus, several other definitions of subdifferential of a non convex function have been proposed (Borwein & Lewis, 2000).
Figure 3.2: Value at $\sigma$ of the Legendre-Fenchel transform of $\psi_e(\epsilon)$

\[ \psi^*_e : \sigma \in S \rightarrow \psi^*_e(\sigma) = \sup_{\epsilon \in E} \{ \sigma : \epsilon \in E \} \in \mathbb{R} \cup \{ +\infty \}, \]

with $\sigma : \epsilon$ denoting the duality pairing of the two spaces $S$ and $E$, whereas $A : \alpha$ is the duality pairing of the space $\Lambda$ of the strain-type internal variables $\alpha$ and the space $A$ of the thermodynamic forces $A$.

Upon the definition of the Legendre-Fenchel transforms, it follows that for any pair $(\sigma, \epsilon) \in S \times E$ and $(\alpha, A) \in \Lambda \times A$,

\[ \psi^*_e(\sigma) + \psi_e(\epsilon) - \sigma : \epsilon \geq 0 \]

\[ \psi^*_p(A) + \psi_p(\alpha) - A : \alpha \geq 0 \]

where the respective equality applies if and only if $(\sigma, \epsilon)$ is the solution of (3.5) and $(A, \alpha)$ is the solution of (3.5)², respectively, cf. Figure 3.3.

---

*Let $X$ and $Y$ be two linear spaces over the same scalar field $\mathbb{R}$. We say that $X$ and $Y$ are dual or that $(X, Y)$ is a dual system if a bilinear form $\langle \cdot, \cdot \rangle : X \times Y \rightarrow \mathbb{R}$ is given. A dual system is called separated if (i) $\forall x \in X - \{0\}, \exists y \in Y : \langle x, y \rangle \neq 0$, (ii) $\forall y \in Y - \{0\}, \exists x \in X : \langle x, y \rangle \neq 0$. As a result, the weak topology $\sigma(X, Y)$ [resp. $\sigma(Y, X)$] on $X$ [resp. $Y$] generated by $Y$ [resp. $X$] is a Hausdorff locally convex (Hlc) topology and it is the weakest locally convex topology on $X$ [resp. $Y$] such that its topological dual $X^*$ [resp. $Y^*$] (i.e. the set of linear continuous functionals over $(X, \tau)$ [resp. $(Y, \mu)$]) is $Y$ [resp. $X$]. Here, $\tau$ and $\mu$ denote Hlc topologies on $X$ and $Y$, respectively (Brezis, 1986). In a Hlc space the weakly closed convex sets are identical with the closed convex sets. Thus, as far as a dual pair of linear spaces is given and they are supplied with separated locally convex topologies compatible with the duality, we can refer to convex lower semicontinuous functionals without specifying the topology (Ekeland & Temam, 1976).

²Since it is $\text{epi}(\sup_{i \in I} f_i) = \bigcap_{i \in I} \text{epi} f_i$, every intersection of closed convex sets is a closed convex set.
Figure 3.3: Geometrical interpretation of the inequalities (3.9) for linear elastic and linear isotropic hardening model

Figure 3.4: Definition of subdifferential at $\bar{\epsilon}^e$. (a) Differentiable function. (b) Non Differentiable function

In conclusion, given its importance and for completeness, we sketch the equivalence between (3.5)$_1$ and (3.7)$_1$.

**Proof.** Given $\sigma \in \mathcal{S}$, let $\bar{\epsilon}^e \in \mathcal{E}$ be such that, (cf. Figure 3.2),

$$
\psi_e(\bar{\epsilon}^e) = \sigma : \bar{\epsilon}^e - \psi_e(\bar{\epsilon}^e) = \sup_{\epsilon^e \in \mathcal{E}} \{\sigma : \epsilon^e - \psi_e(\epsilon^e)\}.
$$

It follows

$$
\sigma : \bar{\epsilon}^e - \psi_e(\bar{\epsilon}^e) \geq \sigma : \epsilon^e - \psi_e(\epsilon^e), \quad \forall \epsilon^e \in \mathcal{E}
$$

that is,

$$
\psi_e(\epsilon^e) - \psi_e(\bar{\epsilon}^e) \geq \sigma : (\epsilon^e - \bar{\epsilon}^e), \quad \forall \epsilon^e \in \mathcal{E}
$$

This, from the definition of subdifferential (cf. note † and Figure 3.4) means

$$
\sigma \in \partial \psi_e(\bar{\epsilon}^e).
$$

If the function $\psi_e(\epsilon^e)$ is differentiable at $\bar{\epsilon}^e$, then

$$
\partial \psi_e(\bar{\epsilon}^e) = \left\{\frac{\partial \psi_e(\epsilon^e)}{\partial \epsilon^e} \right\}
$$

which is (3.5)$_1$. 

52
Vice versa, if $\sigma \in \partial \psi_e(\bar{\epsilon}^e)$, then it follows

$$
\psi_e(\epsilon^e) - \psi_e(\bar{\epsilon}^e) \geq \sigma : (\epsilon^e - \bar{\epsilon}^e), \quad \forall \epsilon^e \in \mathcal{E}
$$

that is,

$$
\sigma : \bar{\epsilon}^e - \psi_e(\bar{\epsilon}^e) \geq \sigma : \epsilon^e - \psi_e(\epsilon^e), \quad \forall \epsilon^e \in \mathcal{E}.
$$

Thus,

$$
\psi^*_e(\sigma) = \sigma : \bar{\epsilon}^e - \psi_e(\bar{\epsilon}^e)
$$

which is (3.7). \qed

The equivalence between (3.5) and (3.7) can be proved using the same arguments.

### 3.2.4.4 Complementary Equations. Associative Plasticity

In isothermal processes where irreversible phenomena occur, that is, the dissipation $-\dot{\psi} + \sigma : \dot{\epsilon}$ does not vanish, the state equations alone are not sufficient to define the current state of the material. Supplementary equations are needed, which allow one to characterize the history of the observable variables in terms of the internal variables (Bataille & Kestin, 1979). These equations, therefore, must have differential character, that is, they have to relate the rate of the internal variables to the associated thermodynamic forces. The conjugacy is here meant in the sense of power as defined by the intrinsic mechanical dissipation $\sigma : \dot{\epsilon}^p - A : \dot{\alpha}$ which is used to relate by duality the space $\Sigma = \dot{\mathcal{E}} \times \dot{\Lambda}$ of the rate of internal variables $(\dot{\epsilon}^p, -\dot{\alpha})$ with the space $\Sigma = \mathcal{S} \times \mathcal{A}$ of the thermodynamic forces $(\sigma, A)$. Here, we have denoted with $\dot{\mathcal{E}}$ the linear space of the rate of the strain tensors $\dot{\epsilon}^p$, and with $\dot{\Lambda}$ the linear space of the tensorial quantities represented by $\dot{\alpha}$.

The complementary equations are only restricted to meet the intrinsic mechanical inequality (3.6). Since $\sigma : \dot{\epsilon}^p - A : \dot{\alpha}$ represents the part of the plastic power which is actually dissipated in heat form, assuming the non negativity of the intrinsic mechanical dissipation means that we require the irreversible processes described by the constitutive model under consideration to produce heat (Besson et al., 2001).

There are several approaches to the definition of the complementary laws in the respect of (3.6) (Chaboche, 1996). A simple way to ensure a priori the thermodynamic consistency of the model is given by the class of the standard generalised materials introduced in Halphen & Nguyen (1975). In this model, one assumes the existence of a potential of dissipation $\varphi(\dot{\epsilon}^p, -\dot{\alpha})$ in the space $\Sigma = \dot{\mathcal{E}} \times \dot{\Lambda}$ of rate of dissipative variables, which is non negative, convex in its variables and such that $\varphi(0, 0) = 0$. The complementary laws are then given by

$$
(\sigma, A) \in \partial \varphi(\dot{\epsilon}^p, -\dot{\alpha})
$$

where the symbol $\partial$ denotes the subdifferential operator (Rockafellar, 1970a; Ekeland & Temam, 1976). As a result of the properties of $\varphi(\dot{\epsilon}^p, -\dot{\alpha})$ and the definition (3.10) of the complementary laws, the thermodynamic consistency of the material model follows quite easily:

53
Thus, for \( \dot{\varphi} \) and for the properties of established by the mechanical intrinsic dissipation, it follows
\[
\varphi(\dot{\varphi}', -\dot{\alpha}') - \varphi(\dot{\varphi}^p, -\dot{\alpha}) \geq \sigma : (\dot{\varphi}' - \dot{\varphi}^p) - A : (\dot{\alpha}' - \dot{\alpha}), \quad \forall (\dot{\varphi}', -\dot{\alpha}') \in \dot{\Sigma}.
\]
Thus, for \((\dot{\varphi}', -\dot{\alpha}') = (0, 0)\) one obtains
\[
\varphi(0, 0) - \varphi(\dot{\varphi}^p, -\dot{\alpha}) \geq -\sigma : \dot{\varphi}^p + A : \dot{\alpha}
\]
and for the properties of \(\varphi(\dot{\varphi}^p, -\dot{\alpha})\), it follows finally
\[
\sigma : \dot{\varphi}^p - A : \dot{\alpha} \geq \varphi(\dot{\varphi}^p, -\dot{\alpha}) \geq 0
\]

\[\square\]

The definition of the complementary laws (3.10) in terms of the subdifferential allows the important class of non differentiable dissipation potentials associated with time independent dissipative processes, which is hereafter considered, such as dry friction, plasticity, brittle fracture and damage, to be included in the same theoretical framework (Panagiotopoulos, 1985; Nguyen, 1994). The rate independent phenomena, in fact, are characterised by assuming the dissipation potential as a lower semicontinuous convex positive homogeneous function of degree one (Panagiotopoulos, 1985; Nguyen, 1994). In geometrical terms, such function presents a convex closed set as epigraph, and for the property,
\[
\varphi(m(\dot{\varphi}^p, \dot{\alpha})) = m \varphi(\dot{\varphi}^p, \dot{\alpha}) \quad \forall m > 0,
\]
which defines the positivity homogeneity of degree one of the function \(\varphi\), the epigraph is, in particular, a cone with vertex in the origin\(^\text{II}\). Consequently, therein, the function is not differentiable, cf. Figure 3.5. A function with the above properties is called a closed gauge\(^*\). In this case, it is possible to show (Maugin, 1992; Han & Reddy, 1999) that the dissipation potential \(\varphi(\dot{\varphi}^p, -\dot{\alpha})\) may be characterized as support function of a closed convex domain \(E \subseteq \Sigma\), containing the origin \((\sigma, A) = (0, 0)\), that is,
\[
\varphi(\dot{\varphi}^p, -\dot{\alpha}) = \sup_{(\sigma, A) \in E} \{ \sigma : \dot{\varphi}^p - A : \dot{\alpha} \}, \quad \forall (\dot{\varphi}^p, -\dot{\alpha}) \in \dot{\Sigma}
\]
with \(E\), called the elastic domain, defined by
\[
E = \{(\sigma, A) \in \Sigma \mid \sigma : \dot{\varphi}^p - A : \dot{\alpha} \leq \varphi(\dot{\varphi}^p, -\dot{\alpha}), \quad \forall (\dot{\varphi}^p, -\dot{\alpha}) \in \dot{\Sigma}\}. \tag{3.12}
\]
\(^\text{II}\)A cone \(K \subseteq \mathcal{V}\) is a set such that the closed half line \(\{\alpha x : \alpha \geq 0\}\) is entirely contained in \(K\) whenever \(x \in K\).
\(^*\)A function \(\gamma : \mathcal{V} \rightarrow [0, +\infty]\) is called a gauge if \(\gamma\) is a non–negative positively homogeneous convex functional such that \(\gamma(0) = 0\), i.e. if the epigraph of \(\gamma\) is a convex cone in \(\mathcal{V} \times \mathbb{R}_+\), where \(\mathbb{R}_+ = \{\alpha \in \mathbb{R} : \alpha \geq 0\}\). If \(\gamma\) is a gauge, there exists a nonempty convex set \(K\) of \(\mathcal{V}\) such that \(\gamma(x) = \gamma_K(x) \equiv \inf_\lambda \{\lambda > 0 : x \in \lambda K\}\). The set \(K\) is not uniquely determined by \(\gamma\) in general, although one always has \(\gamma_C(x) = \gamma(x)\) for \(C = \{x \in \mathcal{V} : \gamma(x) \leq 1\}\). If the gauge \(\gamma\) is also lower semicontinuous, \(C\) is the unique closed convex set containing the origin such that \(\gamma_C(x) = \gamma(x)\) (Rockafellar, 1970a).
In the special case of a regular domain, in the space $\Sigma = S \times A$ of the generalised stresses $q = (\sigma, A)$, the elastic domain may be expressed as

$$\mathbb{E} = \{ q = (\sigma, A) \in \Sigma \mid f(q) \leq 0 \}$$

where $f = f(\sigma, A)$, named the yield criterion, is lower semicontinuous, convex and such that $f(0, 0) \leq 0$.

The set $\mathbb{E}$ defines the locus of the admissible generalised stresses. The name derives from the observation that the thermodynamic forces conjugate to $(\dot{\varepsilon}_p, -\dot{\alpha}) = (0, 0)$, i.e., to an instantaneous elastic response, belong to the elastic domain $\mathbb{E}$, that is,

$$\sigma, A \in \partial\varphi(0, 0) = \mathbb{E}$$

and also, if $(\dot{\varepsilon}_p, -\dot{\alpha}) \neq (0, 0)$, we have

$$\sigma, A \in \partial\varphi(\dot{\varepsilon}_p, -\dot{\alpha}) \subseteq \partial\mathbb{E}.$$  (3.14)

As a result, for any $(\dot{\varepsilon}_p, -\dot{\alpha})$ the associated thermodynamic forces $(\sigma, A)$ in this model are constrained to belong always to the elastic domain $\mathbb{E}$. Next, we prove only (3.13), whereas (3.14) can be easily obtained once the equivalent formulation of (3.10) in terms of the Legendre-Fenchel transform of $\varphi$ is given.

**Proof.** By definition of subdifferential,

$$\partial\varphi(0, 0) = \{ (\sigma, A) \in \Sigma^* \mid \varphi(\dot{\varepsilon}_p, -\dot{\alpha}) - \varphi(0, 0) \geq \sigma : \dot{\varepsilon}_p - A : \dot{\alpha}, \ \forall (\dot{\varepsilon}_p, -\dot{\alpha}) \in \hat{\Sigma} \}$$

and since $\varphi(0, 0) = 0$ we obtain finally (3.12), thus $\partial\varphi(0, 0) = \mathbb{E}$. \hfill \qed

In place of equation (3.10), it is usually much more convenient to refer to the inverse relations obtained by introducing the Legendre-Fenchel transform of $\varphi$ defined as

$$\varphi^* : (\sigma, A) \in S \times A \rightarrow \varphi^*(\sigma, A) = \sup_{(\dot{\varepsilon}_p, -\dot{\alpha}) \in \mathbb{E}} \{ \sigma : \dot{\varepsilon}_p - A : \dot{\alpha} - \varphi(\dot{\varepsilon}_p, -\dot{\alpha}) \}.$$
Since $\varphi(\dot{\epsilon}_p, -\dot{\alpha})$ is the support function of $E$, the dual dissipation potential $\varphi^*(\sigma, A)$ is then the indicator function of $E$ defined as

$$
\varphi^*(\sigma, A) = I_E \overset{\text{def}}{=} \begin{cases} 
0 & \text{if } (\sigma, A) \in E \\
+\infty & \text{if } (\sigma, A) \not\in E,
\end{cases}
$$

The evolution equations for the internal variables are therefore given by (Maugin, 1992),

$$(\dot{\epsilon}_p, -\dot{\alpha}) \in \partial \varphi^*(\sigma, A) = \begin{cases} 
\emptyset & \text{if } (\sigma, A) \not\in E \\
N_E(\sigma, A) & \text{if } (\sigma, A) \in E
\end{cases}
$$

(3.15)

where $\emptyset$ is the empty set whereas the symbol $N_E(\sigma, A)$ denotes the normal cone at $(\sigma, A)$ to $E$. This is a subset of the space $\dot{\Sigma}$ dual to $\Sigma$ where $E$ belongs to, and is defined as

$$
N_E(\sigma, A) \overset{\text{def}}{=} \{ (\dot{\epsilon}_p, -\dot{\alpha}) \in \dot{\Sigma} \mid (\sigma - \tau) : \dot{\epsilon}_p - (A - B) : \dot{\alpha} \leq 0, \ \forall (\tau, B) \in E \}
$$

which is a closed convex cone in $\dot{\Sigma}$.

Since $\Sigma$ and $\dot{\Sigma}$ are finite dimensional spaces, the normal cone can be identified with the cone of the outwards normals at $(\sigma, A)$ to $E$, cf. Figure 3.6, which is a subset of $\Sigma$. Therefore, it is easy to conclude that there is a non zero $(\dot{\epsilon}_p, -\dot{\alpha})$ at each $(\sigma, A) \in \partial E$ whereas $N_E(\sigma, A) = \{(0, 0)\}$ if $(\sigma, A) \in E$. In particular, then, at regular points $(\sigma, A)$ of the boundary $\partial E$ the normal cone $N_E(\sigma, A)$ reduces to the one dimensional set spanned by the outward normal at $(\sigma, A)$, whereas at nonsmooth boundary points $N_E(\sigma, A)$ is a nontrivial cone, cf. Figure 3.6.

The evolution laws given as in (3.15) are usually described as hypothesis of normal dissipation or associativity of the plastic model.

**Remark 3.1.** By definition of subdifferential, equation (3.15) can be interpreted as
the following time inequality

\begin{align}
\text{Given: } & (\sigma, A) \\
\text{Find: } & (\dot{\epsilon}^p, -\dot{\alpha}) \\
\text{Such That: } & \varphi^*(\tau, B) - \varphi^*(\sigma, A) \geq (\tau - \sigma) : \dot{\epsilon}^p - (B - A) : \dot{\alpha} \quad \forall (\tau, B) \in \mathbb{E}. \\
\end{align}

Since \( \varphi^*(\tau, B) = I_{\mathbb{E}} \), then (3.16) reads as

\begin{align}
\sigma : \dot{\epsilon}^p - A : \dot{\alpha} \geq \tau : \dot{\epsilon}^p - B : \dot{\alpha} \quad \forall (\tau, B) \in \mathbb{E},
\end{align}

which is an extension of the principle of maximum plastic dissipation, well known in perfect plasticity (Hill, 1950), for the class of materials with hardening. \( \square \)

**Remark 3.2.** For implementation purposes, one usually resorts to an equivalent formulation of (3.15) given by (Simo & Hughes, 1998; de Souza Neto et al., 2002)

\begin{align}
-\dot{\epsilon}^p + \lambda \frac{\partial f}{\partial \sigma}(\sigma, A) &= 0 \\
-\dot{\alpha} - \lambda \frac{\partial f}{\partial A}(\sigma, A) &= 0 \\
\lambda &\geq 0, \quad \lambda f(\sigma, A) = 0, \quad f(\sigma, A) \leq 0
\end{align}

which is complemented by the consistency condition

\begin{align}
\lambda \dot{f}(\sigma, A) = 0,
\end{align}

where \( \lambda \) is called plastic multiplier.

The equations (3.18) are often referred to as the Kuhn-Tucker conditions (Luenberger, 1984) as they express the optimality conditions for the solution of the maximum plastic dissipation principle described by (3.17). \( \square \)

For our subsequent developments, likewise for the state equations, the evolution laws are recast into an equivalent formulation (Germain et al., 1983; Ladevèze, 1999) which exploits the convexity of the model,

\begin{align}
\varphi(\dot{\epsilon}^p, -\dot{\alpha}) + \varphi^*(\sigma, A) - \sigma : \dot{\epsilon}^p + A : \dot{\alpha} = 0 \iff (\dot{\epsilon}^p, -\dot{\alpha}) \in \partial \varphi^*(\sigma, A) \\
\end{align}

From the properties of the Legendre-Fenchel transform, it follows that for any state \((\sigma, A; \dot{\epsilon}^p, -\dot{\alpha}) \in \Sigma \times \hat{\Sigma}\)

\begin{align}
\varphi(\dot{\epsilon}^p, \dot{\alpha}) + \varphi^*(\sigma, A) - \sigma : \dot{\epsilon}^p + A : \dot{\alpha} \geq 0,
\end{align}

where the equality holds if and only if \((\sigma, A; \dot{\epsilon}^p, \dot{\alpha}) \) is a solution of (3.15). The proof of this equivalence follows the same pattern as for the state equations.
In summary, the constitutive model of standard generalised material is obtained by specifying: (i) by means of the Helmholtz free energy the equations of state of the material, which comprise a relationship between the current value of the independent state variables and their conjugates, and (ii) by means of the dissipation potential the evolution laws, which describe the time evolution of the independent internal variables by means of differential inclusions and confer heredity property to the material behaviour (Germain et al., 1983; Besson et al., 2001).

Finally, for the quasi-static process where inertial effects are neglected, the initial conditions for the variables appearing in rate form have to be given to complete the initial boundary value problem. These are given by

$$
\varepsilon^p(x, t = 0) = \varepsilon^p_0(x) \quad \text{and} \quad \alpha(x, t = 0) = \alpha_0(x)
$$

(3.21)

### 3.2.4.5 Examples of standard rate independent plasticity models

In this section, we present two examples of standard rate independent plasticity models. The first one is a plasticity model with isotropic hardening whereas the second one is a model with isotropic-kinematic hardening.

The state laws (3.7) and the evolution laws (3.19) for the internal variables are formulated within the framework of convex analysis presented in the previous section. For the evolution laws, in particular, given the associativity of the model, it will be sufficient to specify only the elastic domain $E \subseteq \Sigma$, for the dual of the dissipation potential is then the indicator function of $E$.

**The Prandtl–Reuss plasticity model with linear elasticity**

The Prandtl–Reuss plasticity model is a standard model obtained by using the Von Mises yield criterion and an isotropic hardening law (Besson et al., 2001). The internal variables are the plastic strain tensor $\varepsilon^p$ and the accumulated plastic strain $p$, while the conjugate variables are $\sigma$ and $R$, respectively.

**State Laws**

**Linear Elasticity**

The model of linear elasticity is obtained by introducing the free elastic potential

$$
\psi_e(\varepsilon) = \frac{1}{2} C \varepsilon : \varepsilon
$$

(3.22)

with $C$ being a second order positive definite tensor, namely the Hooke tensor. The complementary free energy then follows as

$$
\psi^*(\sigma) = \frac{1}{2} C^{-1} \sigma : \sigma,
$$

(3.23)

which is defined as the Legendre transform of (3.22).

The pair $(\sigma, \varepsilon)$ is said to define a model of linear elasticity if (see equation (3.7)$_1$)

$$
\psi_e(\varepsilon) + \psi^*_e(\sigma) - \sigma : \varepsilon = 0,
$$

58
which, by accounting for (3.22) and (3.23), can also be written as

\[
\frac{1}{2}(\sigma - C\varepsilon): C^{-1}(\sigma - C\varepsilon) = 0.
\]

Finally the familiar Hooke law is obtained as (see equation (3.5))

\[
\sigma - C\varepsilon = 0.
\]

**Hardening Law**

The hardening law is the state equation that relates the internal variables to the thermodynamically conjugate ones as a result of imposing the requirements of the second law of thermodynamics. In the following, we give the details on how to compute the Legendre–Fenchel transform of the free plastic energy for the more general isotropic hardening law. In particular, it will be concluded that the Legendre–Fenchel transform reduces to the Legendre transform.

Let \( g(p) \) be a positive and increasing scalar function of the accumulated plastic strain \( p \) with \( g(p) = 0 \). The free plastic energy is defined as

\[
\psi_p(p) = \int_0^p g(\varepsilon) d\varepsilon
\]

and is a strictly convex function.

The Legendre-Fenchel transform of \( \psi_p(p) \) is given by

\[
\psi_p^*(R) = \sup_{p \geq 0} \{ Rp - \psi_p(p) \}.
\]

For given \( R \geq 0 \), solve for \( \bar{p} \) such that

\[
R\bar{p} - \psi_p(\bar{p}) = \sup_{p \geq 0} \{ Rp - \psi_p(p) \}.
\]

Since \( F(p) = Rp - \psi_p(p) \) is differentiable and concave, \( \bar{p} \) is obtained by solving the equation \( \frac{dF}{dp} \bigg|_{p=\bar{p}} = 0 \), i.e.,

\[
R - g(\bar{p}) = 0,
\]

and because of the invertibility of \( g(p) \), it follows

\[
\bar{p} = g^{-1}(R).
\]

Thus, with \( \bar{p} \geq 0 \), the Legendre–Fenchel transform of \( \psi_p(p) \) is obtained as follows

\[
\psi_p^*(R) = F(\bar{p}) = Rg^{-1}(R) - \psi_p(\bar{p}^{-1}(R)),
\]

which is the Legendre transform of \( \psi_p(p) \).

**Example 3.2.1: Linear Hardening**

Let

\[
g(p) = Hp; \quad \psi_p(p) = \frac{1}{2}Hp^2.
\]
Then equation (3.24) writes as
\[ R - H\bar{p} = 0; \quad \bar{p} = \frac{R}{H}. \]
Hence,
\[ \psi^*_p(R) = R\left[\frac{R}{H} - \frac{1}{2H}\left(\frac{R}{H}\right)^2\right] = \frac{1}{2H}R^2. \]

Thus, in summary, the hardening law can be defined in one of the following equivalent expressions, (3.7)_2 and (3.5)_2,

**Given:** \( \psi_p(p) = \frac{1}{2}Hp^2; \quad \psi^*_p(R) = \frac{1}{2H}R^2 \)
\[ \psi_p(p) + \psi^*_p(R) - Rp = 0 \Leftrightarrow \]
\[ \frac{1}{2H}(R - Hp)^2 = 0 \Leftrightarrow R - Hp = 0. \]

**Evolution Laws**

Given the closed convex elastic domain,
\[ \mathbb{E} = \{ (\sigma, R) \mid \|\sigma_D\| - (R + R_0) \leq 0, \quad R \geq 0 \}, \]
the dual of the dissipation potential for this standard model is the indicator function of \( \mathbb{E} \),
\[ \varphi^*(\sigma, R) = I_{\mathbb{E}}. \]

The Legendre-Fenchel transform of \( \varphi^*(\sigma, R) \), which represents the dissipation potential, is therefore the support function of \( \mathbb{E} \) defined as
\[ \varphi(\dot{\epsilon}^p, -\dot{p}) = \sup_{(\sigma, R) \in \mathbb{E}} \{ \sigma : \dot{\epsilon}^p - R\dot{p} \}, \]
which can be transformed in the following form (Ladevèze, 1999; Han & Reddy, 1999)
\[ \varphi(\dot{\epsilon}^p, -\dot{p}) = R_0\|\dot{\epsilon}^p\| + I_{\mathbb{C}} \]  
(3.25)
with \( I_{\mathbb{C}} \) being the indicator function of the following closed convex set
\[ \mathbb{C} = \{ (\dot{\epsilon}^p, -\dot{p}) \mid \|\dot{\epsilon}^p\| - \dot{p} \leq 0 \text{ and } \text{Tr}[\dot{\epsilon}^p] = 0 \}. \]

**Proof.** Consider
\[ \sup_{(\sigma, R) \in \mathbb{E}} \left\{ \sigma : \dot{\epsilon}^p - R\dot{p} \right\} = \sup_{\|\sigma_D\| - (R_0 + R) \leq 0, \quad R \geq 0} \left\{ \sigma_D : \dot{\epsilon}^p + \frac{1}{3}\text{Tr}[\sigma]\text{Tr}[\dot{\epsilon}^p] - R\dot{p} \right\}. \]
Using the Schwarz’s inequality and noting that the constraint involves only the norm of \( \|\sigma_D\| \) and not its direction, in the search for the supremum we can equivalently
consider only the stress tensors $\sigma$ such that their deviatoric part maximizes $\sigma_D : \dot{\varepsilon}^p$, that is,
\[
\sup_{\|\sigma_D\| - (R_0 + R) \leq 0} \left\{ \sigma_D : \dot{\varepsilon}^p + \frac{1}{3} \text{Tr}[\sigma] \text{Tr}[\dot{\varepsilon}^p] - R \dot{p} \right\} = 
\]
\[
\sup_{\|\sigma_D\| - (R_0 + R) \leq 0} \left\{ \|\sigma_D\| \|\dot{\varepsilon}^p\| + \frac{1}{3} \text{Tr}[\sigma] \text{Tr}[\dot{\varepsilon}^p] - R \dot{p} \right\}
\]
Let
\[
f = \|\sigma_D\| - (R_0 + R)
\]
with the constraint that $f \leq 0$, it is also
\[
\sup_{\|\sigma_D\| - (R_0 + R) \leq 0} \left\{ \|\sigma_D\| \|\dot{\varepsilon}^p\| + \frac{1}{3} \text{Tr}[\sigma] \text{Tr}[\dot{\varepsilon}^p] - R \dot{p} \right\} = 
\]
\[
\sup_{\|\sigma_D\| - (R_0 + R) \leq 0} \left\{ R(||\dot{\varepsilon}^p|| - \dot{p}) + f\|\dot{\varepsilon}^p\| + R_0\|\dot{\varepsilon}^p\| + \frac{1}{3} \text{Tr}[\sigma] \text{Tr}[\dot{\varepsilon}^p] \right\}
\]
and by using simple properties of the supremum (Hiriart-Urruty & Lemaréchal, 2001) it follows also
\[
\varphi(\dot{\varepsilon}^p, -\dot{p}) = R_0\|\dot{\varepsilon}^p\| + \sup_{f \leq 0} f\|\dot{\varepsilon}^p\| + \sup_{R \geq 0} R(||\dot{\varepsilon}^p|| - \dot{p}) + \sup_{\text{Tr}[\sigma] \in R} \frac{1}{3} \text{Tr}[\sigma] \text{Tr}[\dot{\varepsilon}^p],
\]
that is,
\[
\sup_{f \leq 0} f\|\dot{\varepsilon}^p\| = 0,
\]
\[
\sup_{R \geq 0} R(||\dot{\varepsilon}^p|| - \dot{p}) = \left\{ \begin{array}{ll}
0 & \text{if } ||\dot{\varepsilon}^p|| - \dot{p} \leq 0 \\
\infty & \text{if } ||\dot{\varepsilon}^p|| - \dot{p} > 0
\end{array} \right.,
\]
\[
\sup_{\text{Tr}[\sigma] \in R} \frac{1}{3} \text{Tr}[\sigma] \text{Tr}[\dot{\varepsilon}^p] = \left\{ \begin{array}{ll}
0 & \text{if } \text{Tr}[\dot{\varepsilon}^p] = 0 \\
\infty & \text{if } \text{Tr}[\dot{\varepsilon}^p] \neq 0
\end{array} \right.,
\]
which finally delivers (3.25). □

Thus, we say that the set $(\sigma(x, t), R(x, t); \varepsilon^p(x, t), \rho(x, t))$ satisfies the evolution law at the point $x \in \Omega$ if the following differential problem is satisfied
\[
\forall t \leq T,
\]
\[
\|\sigma^D(x, t)\| - [R_0 + R(x, t)] \leq 0
\]
\[
||\dot{\varepsilon}^p(x, t)|| - \dot{\rho}(x, t) \leq 0
\]
\[
\text{Tr}[\dot{\varepsilon}^p(x, t)] = 0
\]
\[
R_0||\dot{\varepsilon}^p(x, t)|| - \sigma(x, t): \dot{\varepsilon}^p(x, t) + R(x, t)\dot{p}(x, t) = 0
\]
The standard Marquis–Chaboche modified plasticity model with Linear Elasticity

The non–associated Marquis–Chaboche plasticity model has been modified into an associated model by Ladèveze & Rougee (1984) and Coffignal (1987). The model accounts for both isotropic and kinematic hardening. The internal variables are, therefore, the plastic strain tensor $\varepsilon^p$, the accumulated plastic strain $p$ and a second order symmetric tensor $\alpha$ with zero trace. The conjugate variables are $\sigma$, $R$, $X$, respectively. The position of the elastic domain in the space of the generalised stresses is controlled by the internal variable $X$ whereas its amplitude by $R$.

State Laws

The free elastic and complementary potential are, as usual, given by

$$\psi_e(\varepsilon^e) = \frac{1}{2} C \varepsilon^e : \varepsilon^e;$$
$$\psi^*_e(\sigma) = \frac{1}{2} C^{-1} \sigma : \sigma;$$

whereas the free plastic potential and its Legendre transform are

$$\psi_p(\alpha, p) = \frac{1}{2} \Lambda \alpha : \alpha + \int_0^p g(\varepsilon) d\varepsilon;$$
$$\psi^*_p(X, R) = \frac{1}{2} \Lambda^{-1} X : X + \left[ R g^{-1}(R) - \int_0^{g^{-1}(R)} g(\varepsilon) d\varepsilon \right],$$

where $\Lambda$ is a second order positive definite tensor, and $g(p)$ is the same as defined for the previous Prandtl–Reuss plasticity model.

The state equations can therefore be equivalently expressed as follows

$$\psi_e(\varepsilon^e) + \psi^*_e(\sigma) - \sigma : \varepsilon^e = 0 \iff \sigma - C \varepsilon^e = 0$$

$$\psi_p(\alpha, p) + \psi^*_p(X, R) - \{ X : \alpha + Rp \} = 0 \iff \begin{vmatrix} R - g(p) = 0 \\ X - \Lambda \alpha = 0 \end{vmatrix}$$

Evolution Laws

The closed convex elastic domain of this model is defined as follows

$$E = \{ (\sigma, X, R) \mid \| \sigma - X \| + \frac{a}{2c} \| X \|^2 - (R + R_0) \leq 0, \quad R \geq 0 \},$$

where $a \geq 0, c > 0$ are material constants. For the standard formulation of the evolution laws, the dual of the dissipation potential is obtained by setting

$$\varphi^*(\sigma, X, R) = I_{\mathbb{R}}.$$
The Legendre–Fenchel transform of $\varphi^*(\sigma, X, R)$ is therefore the support function of the domain $E$, $$\varphi(\dot{\epsilon}^p, -\dot{\alpha}, -\dot{p}) = \sup_{(\sigma, R, X) \in E} \left\{ \sigma : \dot{\epsilon}^p - R\dot{p} - X : \dot{\alpha} \right\}$$ (3.26) which can be given by the following explicit expression

$$\varphi(\dot{\epsilon}^p, -\dot{\alpha}, -\dot{p}) = \begin{cases} R_0\|\dot{\epsilon}^p\| + \frac{c}{2a}\|\dot{\epsilon}^p - \dot{\alpha}\|^2 + \psi_C & \text{if } \dot{\epsilon}^p \neq 0, \\ I_C & \text{if } \dot{\epsilon}^p = 0, \dot{\alpha} = 0, \\ \infty & \text{if } \dot{\epsilon}^p = 0, \dot{\alpha} \neq 0, \end{cases}$$ (3.27) with the domain $C$ given in the Prandtl–Reuss plasticity model.

**Proof.** Following arguments similar to the one shown in the Prandtl–Reuss model, we obtain

$$\varphi(\dot{\epsilon}^p, -\dot{\alpha}, -\dot{p}) = \sup_{(\sigma, R, X) \in E} \left\{ \sigma : \dot{\epsilon}^p - R\dot{p} - X : \dot{\alpha} \right\} =$$

$$= \sup_{(\sigma, R, X) \in E} \left\{ (\sigma_D - X + X) : \dot{\epsilon}^p + \frac{1}{3} \text{Tr}[\sigma] \text{Tr}[\dot{\epsilon}^p] - R\dot{p} - X : (\dot{\epsilon}^p - \dot{\alpha}) \right\} =$$

$$= \sup_{(\sigma, R, X) \in E} \left\{ \|\sigma_D - X\|\|\dot{\epsilon}^p\| + \frac{1}{3} \text{Tr}[\sigma] \text{Tr}[\dot{\epsilon}^p] - R\dot{p} + \|X\|\|\dot{\epsilon}^p - \dot{\alpha}\| \right\} =$$

$$= \sup_{R \geq 0, \|X\| \geq 0, \text{Tr}[\sigma] \in \mathbb{R}, f \leq 0} \left\{ R(\|\dot{\epsilon}^p\| - \dot{p}) + f\|\dot{\epsilon}^p\| + R_0\|\dot{\epsilon}^p\| + \frac{1}{3} \text{Tr}[\sigma] \text{Tr}[\dot{\epsilon}^p] + \|X\|\|\dot{\epsilon}^p - \dot{\alpha}\| - \frac{a}{2c}\|X\|^2\|\dot{\epsilon}^p\| \right\},$$

where we have let

$$f = \|\sigma_D - X\| + \frac{a}{2c}\|X\|^2 - (R + R_0)$$

and made use of the Schwarz’s inequality.

Since the variables may vary independently on each other, it follows

$$\varphi(\dot{\epsilon}^p, -\dot{\alpha}, -\dot{p}) = R_0\|\dot{\epsilon}^p\| + \sup_{R \geq 0} \left\{ R(\|\dot{\epsilon}^p\| - \dot{p}) \right\} + \sup_{\text{Tr}[\sigma] \in \mathbb{R}} \frac{1}{3} \text{Tr}[\sigma] \text{Tr}[\dot{\epsilon}^p] +$$

$$+ \sup_{f \leq 0} f\|\dot{\epsilon}^p\| + \sup_{\|X\| \geq 0} \left\{ \|X\|\|\dot{\epsilon}^p - \dot{\alpha}\| - \frac{a}{2c}\|X\|^2\|\dot{\epsilon}^p\| \right\}. $$
As for the last term, we note
\[
\sup_{\|X\| \geq 0} \left\{ \|X\| \|\dot{\epsilon}^p - \dot{\alpha}\| - \frac{a}{2c} \|X\|^2 \|\dot{\epsilon}^p\| \right\} = \begin{cases} \frac{c}{2a} \|\dot{\epsilon}^p - \dot{\alpha}\|^2 & \text{if } \dot{\epsilon}^p \neq 0, \\ I_C & \text{if } \dot{\epsilon}^p = 0, \dot{\alpha} = 0, \\ \infty & \text{if } \dot{\epsilon}^p = 0, \dot{\alpha} \neq 0, \end{cases}
\]
whereas for the other terms the same considerations as expressed for the Prandtl–Reuss model hold, so that finally, the expression (3.27) is obtained.

### 3.3 Contractivity of the elastoplastic flow

Object of this section is to show the contractivity of the elastoplastic flow for standard generalised materials described by a quadratic Helmholz energy. For the more general case of strongly convex energy, we refer to Laborde & Nguyen (1990) where, however, the Lipschitz property of the elastoplastic flow with respect to the initial data is shown to hold.

Denote by \((\sigma, A; \epsilon, \epsilon^p, \alpha)\) the solution of the following problem
\[
\begin{align*}
\langle \sigma, \nabla \eta \rangle &= \langle b, \eta \rangle + \langle t, \eta \rangle_{\partial \Omega}, \quad \forall \eta \in V_0 \\
\epsilon &= \epsilon^e + \epsilon^p \\
\sigma &= C \epsilon^e \\
A &= H \alpha \\
(\dot{\epsilon}^p, -\dot{\alpha}) &\in \partial \varphi^*(\sigma, A) \\
\epsilon^p(t = 0) &= \epsilon^p_0 \\
\alpha(t = 0) &= \alpha_0
\end{align*}
\]
and by \((\tilde{\sigma}, \tilde{A}; \tilde{\epsilon}, \tilde{\epsilon}^p, \tilde{\alpha})\) the solution of the same problem but with initial state given by \(\epsilon^p_0, \alpha_0\). The monotony of the operator \(\partial \varphi^*\), which is the subdifferential of a convex function\(^8\), gives
\[
(\sigma - \tilde{\sigma}) : (\dot{\epsilon}^p - \dot{\tilde{\epsilon}}^p) - (A - \tilde{A}) : (\dot{\alpha} - \dot{\tilde{\alpha}}) \geq 0,
\]
that is,
\[
(\sigma - \tilde{\sigma}) : (\dot{\epsilon}^e - \dot{\tilde{\epsilon}}^e) + (A - \tilde{A}) : (\dot{\alpha} - \dot{\tilde{\alpha}}) \leq 0, 
\]
(3.28)
where we have accounted for the additivity of the total strain and the equality
\[
(\sigma - \tilde{\sigma}) : (\dot{\epsilon} - \dot{\tilde{\epsilon}}) = 0,
\]
\(^8\)Let \(V\) be a real Banach space and \(V^*\) its dual. If \(f\) is a lower semicontinuous proper convex function on \(V\), then \(\partial f : v \in V \rightarrow \partial f(v) \in 2^{V^*}\) is a maximal cyclically monotone operator. The monotone cyclically operator property follows easily from the definition of subdifferential. For proving the maximality of the operator, refer to Rockafellar (1970b).
since $\sigma - \tilde{\sigma}$ is self equilibrated and $\epsilon - \tilde{\epsilon}$ corresponds to the difference of two kinematically admissible displacement fields for the same problem.

By accounting for the state equations in (3.28), it follows

$$\frac{d}{dt}\left[C(\epsilon^e - \tilde{\epsilon}^e) + H(\alpha - \tilde{\alpha}) : (\alpha - \tilde{\alpha})\right] \leq 0,$$

which shows that the time dependent function

$$C(\epsilon^e(t) - \tilde{\epsilon}^e(t)) : (\epsilon^e(t) - \tilde{\epsilon}^e(t)) + H(\alpha(t) - \tilde{\alpha}(t)) : (\alpha(t) - \tilde{\alpha}(t))$$

is nonincreasing with time, thus

$$C(\epsilon^e(t) - \tilde{\epsilon}^e(t)) : (\epsilon^e(t) - \tilde{\epsilon}^e(t)) + H(\alpha(t) - \tilde{\alpha}(t)) : (\alpha(t) - \tilde{\alpha}(t)) \leq C(\epsilon^e_0 - \tilde{\epsilon}^e_0) : (\epsilon^e_0 - \tilde{\epsilon}^e_0) + H(\alpha_0 - \tilde{\alpha}_0) : (\alpha_0 - \tilde{\alpha}_0) \forall t \geq 0,$$

which expresses the contractivity of the elastoplastic flow with respect to the norm associated with the Helmholtz energy. In the following section, we show how this property allows the use of the time accumulated residual as indication of the error in solution.

### 3.4 Residual versus Error in Solution

For the considerations in this section, we basically follow Eriksson et al. (1996), which we refer to for further details.

Consider the scalar initial value problem,

$$\begin{align*}
\dot{u}(t) + a(t)u(t) &= f(t) \quad t \in [0, T] \\
u(0) &= u_0
\end{align*}$$

(3.29)

with $a(t) \geq 0$.

The solution of (3.29) is given by

$$u(t) = \exp[-A(t)]u_0 + \int_0^t \exp[-(A(t) - A(\tau))]f(\tau)\,d\tau$$

(3.30)

where $A(t) = \int_0^t a(\tau)d\tau$, so that the following $a priori$ estimate can be easily obtained

$$|u(t)| \leq |u_0| + \int_0^t |f(\tau)|\,d\tau \quad \forall t \leq T,$$

(3.31)

for the nondecreasing character of $A = A(t)$.

Let $0 = t_1 < \ldots < t_n < \ldots < t_{N+1} = T$ be a partition of the time interval $[0, T]$ of interest and consider a function $U = U(t)$ to be approximation of the problem (3.29), which is differentiable over the intervals $[t_n, t_{n+1}]$. The function
$U = U(t)$ may have jump discontinuities at the time instants $t_n$, thus we let $U(t_n^+)-U(t_n^-) = \Delta_n$. For $n = 1$, we assume $U(t_1^-) = u_0$, thus $\Delta_1 = U_0 - u_0$. This means that $U = U(t)$ is solution of the following problem

For $n = 1, \ldots, N$

\[
\begin{align*}
\dot{U}(t) + a(t)U(t) &= f(t) + R(t) \quad t \in [t_n, t_{n+1}] \\
U(t_n^+) &= U(t_n^-) + \Delta_n
\end{align*}
\]

where $R = R(t)$ is the residual produced by $U = U(t)$ within each time interval $[t_n, t_{n+1}]$ where $U = U(t)$ is differentiable.

The error $e(t) = u(t) - U(t)$ associated with the approximation $U = U(t)$ is, therefore, solution of the following problem

For $n = 1, \ldots, N$

\[
\begin{align*}
\dot{e}(t) + a(t)e(t) &= R(t) \quad t \in [t_n, t_{n+1}] \\
e(t_n) &= \Delta_n.
\end{align*}
\] (3.32)

Using for each subinterval $[t_n, t_{n+1}]$ the result given in (3.30), we obtain

\[
e(t) = \sum_{n=1}^{N} \exp[-A(t-t_n)]\Delta_n\beta_n + \int_{0}^{t} \exp[-(A(t) - A(\tau))]R(\tau)\,d\tau
\] (3.33)

where

\[
\beta_n = \begin{cases} 
0 & \text{if } t \leq t_n \\
1 & \text{if } t > t_n.
\end{cases}
\]

In equation (3.33) the term

\[
\exp[-A(t-t_n)]\Delta_n\beta_n
\]

gives the propagation at $t(\geq t_n)$ of the discontinuity jump $\Delta_n$ in the approximate solution $U = U(t)$, whereas

\[
\int_{0}^{t} \exp[-(A(t) - A(\tau))]R(\tau)\,d\tau
\]

can be interpreted as the sum of the time–elemental contributions to the total error at the time $t$. The time–elemental contributions are obtained by the propagation at time $t$ of the residual error $R(\tau)\,d\tau$ produced within the time–elemental interval $[\tau, \tau + d\tau]$ at time $\tau \leq t$.

**Remark 3.3.** Equation (3.33) shows the influence of the jump discontinuities on the error. Also, note that for a continuous approximation solution $U = U(t)$, that is, $\Delta_n = 0$, for $n = 1, \ldots, N$, the error depends only on the residual produced within the time intervals where the approximation is differentiable.
Applying (3.31) and the triangular inequality, we obtain the following a priori estimate of the solution (3.33),

\[ |e(t)| \leq \sum_{n=1}^{N} |\Delta_n| \beta_n + \int_{0}^{t} |R(\tau)| \, d\tau \quad \forall t \leq T, \tag{3.34} \]

which shows the accumulation in time of the jump discontinuities and of the residual as indication of the pointwise error. From (3.34), it is immediate to obtain also the following global estimate in time,

\[ \sup_{t \leq T} |e(t)| \leq \sum_{n=1}^{N} |\Delta_n| + \int_{0}^{T} |R(\tau)| \, d\tau. \tag{3.35} \]

**Remark 3.4.** If \( R(t) = 0, \forall t \in [t_n, t_{n+1}] \), \( \forall n \), that is, the approximate solution \( U(t) \) does satisfy exactly equation (3.29) over each time interval \([t_n, t_{n+1}]\), the second term on the r.h.s. of equation (3.33) disappears and the error is due to the occurrence of the jumps in \( U = U(t) \) across the time nodes \( t_n \). Finally, this means that the error of the approximate solution \( U = U(t) \) is related to the error in the initial data over each time interval.

The property of problem (3.29), which has allowed development of an estimate in terms of the jumps and of the residual such as (3.35), is its dissipativity resulting from \( a(t) \geq 0 \). Dissipativity is represented by the contraction of the solution with respect to the initial state, i.e.,

\[ |u(t) - \tilde{U}(t)| \leq |u_0 - \tilde{U}_0| \]

where \( \tilde{U} = \tilde{U}(t) \) denotes now a solution of problem (3.29) with the same \( f = f(t) \) but different initial state \( \tilde{U}_0 \).

The contractivity of the elastoplastic flow is enjoyed by the standard generalised materials described by a quadratic Helmholtz energy as shown in the previous section. This, therefore, suggests the use of an error estimate (3.35) for the aforementioned class of problems.

**Remark 3.5.** The link between residual and error is well known in linear algebra. The residual of an approximate solution of the linear system, \( Ax = b \), influences the error in solution not only by the residual size but also by the condition number of the system matrix. For those problems which have a condition number not greater than one the residual of the approximate solution can be assumed as an estimate of its error (Stewart, 1973; Estep et al., 2000). In fact, we have the following result (Stewart, 1973)

**Theorem.** Let \( A \) be nonsingular, \( Ax = b \neq 0 \), and \( A\tilde{x} = b + r \). Then

\[ \frac{\|x - \tilde{x}\|}{\|x\|} \leq \kappa(A) \frac{\|r\|}{\|b\|} \]

where \( \kappa(A) = \|A\|\|A^{-1}\| \) is the condition number of \( A \) and \( r \) is the residual associated with the approximation \( \tilde{x} \) of the linear system \( Ax = b \).
3.4.1 A simple \textit{a posteriori} error estimate via discrete energy dissipation

In this Section we develop a simple \textit{a posteriori} error estimate of the discretization error obtained by solving an ordinary differential equation of the first order with the backward Euler method. This technique extends the one developed by Nochetto et al. (2000) for \textit{a posteriori} error estimation of the backward Euler approximations of abstract evolution equations in Hilbert space. In Nochetto et al. (2000) it is assumed that $U_n^- = U_n^+$, whereas in what follows, this hypothesis is removed. The meaning of the notation is kept the same as in the previous Section. Also, we assume that $u(t)$ and $\mathcal{F}(u)$ are scalar functions.

The theory is presented for the model problem

\begin{align}
\text{Find: } & u = u(t) \\
\dot{u}(t) + \mathcal{F}(u) = 0 \\
& u(t = 0) = u_0
\end{align}

(3.36)

where $\mathcal{F}(u)$ is assumed to admit a convex potential $\phi = \phi(u)$, that is,

\begin{align}
\mathcal{F}(u) = \frac{d}{du} \phi(u).
\end{align}

(3.37)

For the problem (3.29), for example, $\phi(u) = \frac{1}{2}au^2$.

The use of the backward Euler method as time discretization scheme of (3.36) gives

\begin{align}
\text{For } n = 1, \ldots, N \\
\text{Data: } U_n^+ \\
\text{Find: } U_{n+1}^- \\
\frac{U_{n+1}^- - U_n^+}{k_n} + \mathcal{F}(U_{n+1}^-) = 0
\end{align}

(3.38)

where $k_{n+1} = t_{n+1} - t_n$ and the data $U_n^+$ of the algebraic equation relative to the time step $[t_n^+, t_{n+1}^-]$ is assumed to be different from the solution $U_n^-$ of the previous time step.

From the definition of differential given in note (†) of Section 3.2.4.3, problem (3.36) is equivalent to the following evolution variational inequality,

\begin{align}
\text{Find: } & u = u(t) \\
\langle \dot{u}(t), u(t) - v(t) \rangle + \phi(u(t)) - \phi(v(t)) \leq 0, & \forall v(t) \in C^0(0,T)
\end{align}

(3.39)

where $C^0(0,T)$ is the space of the continuous functions over $[0,T]$. Hence, the
problem (3.38) is equivalent to the inequality

$$\frac{1}{k_{n+1}} \langle U_{n+1}^- - U_{n+1}^+, U_{n+1}^- - v \rangle + \phi(U_{n+1}^-) - \phi(v) \leq 0, \quad \forall v \in \mathbb{R}. \quad (3.40)$$

Hereafter, $U = U(t)$ is the discontinuous function obtained as piecewise linear interpolant of the values $U_n^+$ and $U_n^-$ over each time step $[t_n, t_{n+1}]$ for $n = 1, \ldots, N$.

The error estimate given in Nochetto et al. (2000) is related to the amount of energy dissipation associated with $U = U(t)$ and it is obtained by exploiting the Lyapunov properties of $\phi = \phi(t)$, which decreases along solution paths of both (3.36) and (3.38). In fact, it is known that $\phi$ satisfies the energy identity (Eriksson et al., 1996)

$$\frac{1}{2} |\dot{u}(t)|^2 + \frac{d}{dt} \phi(u(t)) = 0, \quad \text{a.e. } t \in [0, T] \quad (3.41)$$

and the discrete energy inequality

$$-\mathcal{E}_{n+1} \overset{\text{def}}{=} \left| U_{n+1}^- - U_n^+ \right|^2 + \frac{\phi(U_{n+1}^-) - \phi(U_n^+)}{k_{n+1}} \leq 0, \quad \forall 0 \leq n \leq N, \quad (3.42)$$

which follows directly from (3.40) upon choosing $v = U_n^+$. The discrete quantity $\mathcal{E}_{n+1}$ in (3.42) is thus a measure of the residual produced by $U = U(t)$ in the energy equation (3.41).

An a posteriori error bound for the solution (3.40) can be computed as follows

$$\max_{t \in [0, t_{N+1}]} |u(t) - U(t)| \leq \sum_{n=0}^{N} k_{n+1} \sqrt{\mathcal{E}_{n+1}} + \sum_{n=1}^{N} |U_n^- - U_n^+| + |U_0 - u_0|, \quad (3.43)$$

where the estimate depends on both $\mathcal{E}_{n+1}$ and $|U_n^- - U_n^+|$, with the latter vanishing in the classical use of the backward Euler.

**Proof.** The linear interpolant $U = U(t)$ over $[t_n, t_{n+1}]$ is given by

$$U(t) = \frac{t - t_n}{k_{n+1}} U_{n+1}^- + \frac{t_{n+1} - t}{k_{n+1}} U_n^+. \quad \text{Thus,}$$

$$\dot{U}(t) = \frac{U_{n+1}^- - U_n^+}{k_{n+1}}, \quad \forall t \in [t_n, t_{n+1}].$$

As a result, it follows

$$\frac{1}{k_{n+1}} \langle U_{n+1}^- - U_n^+, U_{n+1}^- - v \rangle = \langle \dot{U}, U - v \rangle - \langle \dot{U}, U - U_{n+1}^- \rangle. \quad \text{69}$$
Thus, equation (3.40) can be re-written as

\[ \langle \dot{U}, U - v \rangle + \phi(U) - \phi(v) \leq \underbrace{\langle \dot{U}, U - U_{n+1}^- \rangle + \phi(U) - \phi(U_{n+1}^-)}_{\mathcal{R}} \]

which resembles equation (3.39). The next step is to estimate \( \mathcal{R} \). With this regard, note that

\[ U - U_{n+1}^- = (t - t_{n+1}) \dot{U} \]

and for the convexity of \( \phi \), it follows

\[ \phi(U) \leq \frac{t - t_n}{k_{n+1}} \phi(U_{n+1}^-) + \frac{t_{n+1} - t}{k_{n+1}} \phi(U_n^+) \]

Hence,

\[ \phi(U) - \phi(U_{n+1}^-) \leq (t - t_{n+1}) \frac{\phi(U_{n+1}^-) - \phi(U_n^+)}{k_{n+1}} \]

Thus,

\[ \mathcal{R} \leq (t - t_{n+1}) \langle \dot{U}, U \rangle + (t - t_{n+1}) \frac{\phi(U_{n+1}^-) - \phi(U_n^+)}{k_{n+1}} = (t - t_{n+1}) \mathcal{E}_{n+1}, \]

where \( \mathcal{E}_{n+1} \) has been defined in (3.42). Therefore, equation (3.40) can be written as follows

\[ \langle \dot{U}, U - v \rangle + \phi(U) - \phi(v) \leq (t - t_{n+1}) \mathcal{E}_{n+1}, \quad \forall v \in \mathbb{R}. \]

For \( v = U \), one obtains

\[ \langle \dot{U}, U - u \rangle + \phi(U) - \phi(u) \leq (t - t_{n+1}) \mathcal{E}_{n+1}, \]

whereas equation (3.39) for \( v = U \) gives

\[ \langle \dot{u}, u - U \rangle + \phi(u) - \phi(U) \leq 0. \]

Summing up the last two inequalities delivers

\[ \langle \dot{u} - \dot{U}, u - U \rangle \leq (t - t_{n+1}) \mathcal{E}_{n+1}, \quad \text{a.e. } t \in [t_n, t_{n+1}], \]

which can also be written as

\[ \frac{1}{2} \frac{d}{dt} |u(t) - U(t)|^2 \leq (t - t_{n+1}) \mathcal{E}_{n+1}, \quad \text{a.e. } t \in [t_n, t_{n+1}]. \]

Integration over the time step under consideration delivers

\[ |u(t_{n+1}) - U_{n+1}^-|^2 \leq k_{n+1}^2 \mathcal{E}_{n+1} + |U_n^+ - u(t_n)|^2 \]

that is,\(^\text{5}\)

\[ |u(t_{n+1}) - U_{n+1}^-| \leq k_{n+1} \sqrt{\mathcal{E}_{n+1}} + |U_n^+ - u(t_n)| \leq k_{n+1} \sqrt{\mathcal{E}_{n+1}} + |U_n^+ - U_{n-1}^-| + |U_{n-1}^- - u(t_n)| \]

and by induction one finally obtains (3.43). \( \square \)

\(^\text{5}\)If \( a^2 \leq b^2 + c^2 \), with \( b, c \geq 0 \), then it is \( b^2 + c^2 \leq (b + c)^2 \), thus it follows \( a \leq b + c \).
3.5 Admissible solution and measure of the error

In section 3.2 we have described the properties and equations that define the behaviour of the standard generalised material model, which for the reader’s convenience are summarized in Box 3.1.

We assume that the problem of computing the response of such model to given external actions is posed in the set of functions, $(\sigma(x,t), A(x,t); \epsilon(x,t), \epsilon^p(x,t), \alpha(x,t))$, which gives a finite value to the global energy

$$\int_{\Omega} \epsilon^T \gamma_{\epsilon,\epsilon}^{\eta} (\sigma; \epsilon^e) \, d\Omega + \int_{\Omega} \epsilon^T \gamma_{\epsilon,\epsilon}^{\eta} (A; \alpha) \, d\Omega + \int_0^T \int_{\Omega} \gamma_{\epsilon,\epsilon}^{\eta} (\sigma, A; \dot{\epsilon}^p, \dot{\alpha}) \, dt \, d\Omega < \infty.$$

Also, we assume that the formulation is such that the problem has a solution which is unique.

In this class of functions, we distinguish a subset given by those functions which satisfy only some properties and equations given in Box 3.1. Any element of this set is referred to, in general, as an admissible solution. It is, therefore, clear that an admissible solution is the exact solution if and only if also the remaining equations are satisfied. If in the conditions defining the admissible solutions all the qualitative properties are included, the approximation quality of an admissible solution is described by the residual, which is produced in the equations defining the admissible subset.

Given the dissipative character of the problem under consideration, as shown in Section 3.3, and following from arguments given in Section 3.4, a direct measure of the residual can be used as an indication of the error associated with the problem.

The notion of error in the constitutive equations for non linear problems as introduced by Ladéveze et al. (1986) implements the above ideas by splitting the equations that govern the behaviour of the continuum in two groups: One group is used to define an admissible solution and combines the kinematic compatibility conditions, the additivity of the strain tensor, the equilibrium equations and the initial conditions. The second group comprises, on the other hand, the constitutive equations and is used to quantify the approximation of the admissible solution by means of the residual produced therein. As far as the measure of this residual is concerned, this depends on the type of constitutive formulation which is adopted. In the constitutive formulation with internal variables we exploit the convexity structure of the state laws and evolution laws, as described in Section 3.2.4, by referring to equivalent scalar formulations of the tensorial constitutive equations. These equivalent formulations, in turn, can be interpreted as offset of the energetic balance which is not met by the admissible solution. The notion of dissipation error and extended dissipation error are in this way introduced according to whether or not we include the state laws in the conditions that define the admissibility of a solution, respectively.
Box 3.1. Initial Boundary Value Problem for Standard Generalised Models with Internal Variables

Find \( \sigma(x, t), A(x, t), u(x, t), e(x, t), e^p(x, t), \alpha(x, t) \) such that the following conditions are satisfied:

**Kinematic Compatibility:**
- Continuity of the Displacement Field, \( u(x, t) \).
- Time continuity of the Total Strain, \( e(x, t) = \nabla_s u(x, t) \).
- Time continuity of the Plastic Strain, \( e^p(x, t) \).
- Time continuity of the Internal Variables, \( \alpha(x, t) \).
- Displacement Boundary Conditions.

**Additivity of the Strain Tensor:**
\[
\epsilon(x, t) = \epsilon^e(x, t) + \epsilon^p(x, t),
\]
\( \forall x \in \Omega, \ \forall t \in [0, T] \).

**Equilibrium:**
\[
\langle \sigma(x, t), \nabla \eta(x) \rangle = \langle b(x, t), \eta(x) \rangle + \langle t(x, t), \eta(x) \rangle_{\partial \Omega},
\]
\( \forall \eta \in V_0, \ \forall t \in [0, T] \).

**Initial Conditions:**
\[
e^p(x, t = 0) = 0, \quad \forall x \in \Omega
\]
\[
\alpha(x, t = 0) = 0, \quad \forall x \in \Omega
\]

**State Laws:**
\[
\psi_e (\epsilon^e(x, t)) + \psi_e^*(\sigma(x, t)) - \sigma(x, t) : \epsilon^e(x, t) = 0,
\]
\[
\psi_p (\alpha(x, t)) + \psi_p^*(A(x, t)) - A(x, t) : \alpha(x, t) = 0,
\]
\( \forall x \in \Omega, \ \forall t \in [0, T] \).

**Evolution Laws:**
\[
\varphi (\dot{e}^p(x, t), -\dot{\alpha}(x, t)) + \varphi^*(\sigma(x, t), A(x, t)) +
\]
\[
-\sigma(x, t) : \dot{e}^p(x, t) + A(x, t) : \dot{\alpha}(x, t) = 0,
\]
\( \forall x \in \Omega, \ \forall t \in [0, T] \).
If a functional formalism is adopted for the constitutive modelling, for materials that strictly follow the conditions of Drucker stability (Drucker, 1964) the notion of Drucker’s error is introduced (Ladèveze et al., 1986; Coffignal, 1987; Gallimard, 1994). Here, the Drucker’s inequality is used to quantify the quality of the admissible solution, which is not required to satisfy the constitutive equations.

The violation of a qualitative property, on the other hand, such as time continuity, for instance, requires a more specific treatment and this will be object of a further subsection.

3.5.1 Error in the constitutive equations for time continuous admissible solution

Throughout this section, in the definition of the admissible conditions we will always consider functions which are time continuous over the time interval of interest. In order to be more specific in the treatment, in the following the internal variables are denoted as \( p \) and \( \alpha \) to indicate scalar and tensorial quantities, respectively. The associated thermodynamic forces are thus referred to as \( R \) and \( X \), respectively.

3.5.1.1 Dissipation Error

This measure of the error has been introduced for the first time by Ladevèze (1989) within the context of the LATIN method applied to the solution of the initial boundary value problem of a material model formulated with internal variables. Its numerical performance has been assessed in Moës (1996); Ladevèze & Moës (1997) and Ladevèze & Moës (1999).

**Definition of the Admissibility Conditions**

The dissipation error is the error in the constitutive equations for a formulation with internal variables obtained by including the state laws in the definition of the admissibility conditions. More precisely, the field \((\sigma_{ad}(x,t), \mathbf{X}_{ad}(x,t), R_{ad}(x,t); u_{ad}(x,t), \epsilon_{ad}(x,t), \epsilon_{ad}^{p}(x,t), \alpha_{ad}(x,t), p_{ad}(x,t))\) is an admissible solution with respect to the computation of the dissipation error if the following conditions are met:

<table>
<thead>
<tr>
<th>Kinematic Compatibility:</th>
</tr>
</thead>
<tbody>
<tr>
<td>Continuity of the Displacement Field, ( u_{ad}(x,t) ).</td>
</tr>
<tr>
<td>Time continuity of the Total Strain, ( \epsilon_{ad}(x,t) = \nabla_{s}u_{ad}(x,t) ).</td>
</tr>
<tr>
<td>Time continuity of the Plastic Strain, ( \epsilon_{ad}^{p}(x,t) ).</td>
</tr>
<tr>
<td>Time continuity of the Internal Variables, ( \alpha_{ad}(x,t), p_{ad}(x,t) ).</td>
</tr>
<tr>
<td>Displacement Boundary Conditions.</td>
</tr>
</tbody>
</table>
Additivity of the Strain Tensor:
\[ \epsilon_{ad}(x, t) = \epsilon_{ad}^e(x, t) + \epsilon_{ad}^p(x, t), \]
\[ \forall x \in \Omega, \quad \forall t \in [0, T]. \]

Equilibrium:
\[ \langle \sigma_{ad}(x, t), \nabla \eta(x) \rangle = \langle b(x, t), \eta(x) \rangle + \langle t(x, t), \eta(x) \rangle_{\partial \Omega}, \]
\[ \forall \eta \in \mathcal{V}_0, \quad \forall t \in [0, T]. \]

Initial Conditions:
\[ \epsilon_{ad}^p(x, t = 0) = 0, \quad \forall x \in \Omega \]
\[ \alpha_{ad}(x, t = 0) = 0, \quad p_{ad}(x, t = 0) = 0 \]

State Laws:
\[ \psi_e(\epsilon_{ad}^e(x, t)) + \psi_p^e(\sigma_{ad}(x, t)) - \sigma_{ad}(x, t) : \epsilon_{ad}^e(x, t) = 0, \]
\[ \psi_p(\alpha_{ad}(x, t), p_{ad}(x, t)) + \psi_p(\mathbf{X}_{ad}(x, t), R_{ad}(x, t)) + \]
\[ -\left[ \mathbf{X}_{ad}(x, t) : \alpha_{ad}(x, t) + R_{ad}(x, t)p_{ad}(x, t) \right] = 0, \]
\[ \forall x \in \Omega, \quad \forall t \in [0, T]. \]

**Definition of Error**

The only equation which in general is not satisfied by an admissible solution is therefore the evolution law. The quality of its approximation does then depend upon the residual produced therein. A natural way to measure this residual is therefore the evolution law. The quality of its approximation does then depend upon the residual produced therein. A natural way to measure this residual is obtained by resorting to a scalar equivalent formulation of the evolution law due to the convexity nature of the law. This formulation is discussed in Section 3.2.4.4 and especially notable are its properties (3.19) and (3.20). As a result, it is quite natural to assume the following definition of an error

\[ e_{dis}^2(T) = 2 \int_\Omega \int_0^T d^2 \eta_{x,t}(\sigma_{ad}, \mathbf{X}_{ad}, R_{ad}; \epsilon_{ad}^p, \alpha_{ad}, \dot{\alpha}_{ad}) \, dt \, d\Omega \]  

(3.44)

where we have let
\[ d^2 \eta_{x,t}(\sigma_{ad}, \mathbf{X}_{ad}, R_{ad}; \epsilon_{ad}^p, \alpha_{ad}, \dot{\alpha}_{ad}) = \]
\[ = \varphi^e(\sigma_{ad}(x, t), \mathbf{X}_{ad}(x, t), R_{ad}(x, t)) + \varphi(\epsilon_{ad}^p(x, t), -\dot{\alpha}_{ad}(x, t), -\dot{\alpha}_{ad}(x, t)) + \]
\[ -\sigma_{ad}(x, t) : \epsilon_{ad}^p(x, t) + \mathbf{X}_{ad}(x, t) : \alpha_{ad}(x, t) + R_{ad}(x, t)\dot{\alpha}_{ad}(x, t) \]

and we recall, once again, that the state laws are satisfied by the admissible solution.
Remark 3.6. Equation (3.44) defines the global error at time $T$ as a sum of elemental contributions arising from the unaltered propagation at $t$ of the error produced by the residual \( \dot{\eta}^2_{x,t} \, d\Omega \, d\tau \) within $\Omega$ and $[\tau, \tau + d\tau]$ at time $\tau \leq t$. This is in agreement with the dissipative nature of the problem under consideration as discussed in Section 3.3 and Section 3.4.

If we denote by

\[
s_{ad}(x,t) = \left( \sigma_{ad}(x,t), X_{ad}(x,t), R_{ad}(x,t), \epsilon_{ad}(x,t), \alpha_{ad}(x,t), p_{ad}(x,t) \right),
\]

and

\[
s_{ex}(x,t) = \left( \sigma_{ex}(x,t), X_{ex}(x,t), R_{ex}(x,t), \epsilon_{ex}(x,t), \alpha_{ex}(x,t), p_{ex}(x,t) \right),
\]

an admissible and the exact solution of the initial boundary value problem, respectively, the definition (3.44) can be assumed as a global measure of the error of the (kinematic) admissible solution in the following sense:

Theorem 3.1.

Given an admissible solution $s_{ad} = s_{ad}(x,t)$ with respect to the computation of the dissipation error, it follows

\[
e_{\text{dis}}^2(T) \geq 0
\]

\[
e_{\text{dis}}^2(T) = 0 \iff s_{ad}(x,t) = s_{ex}(x,t) \quad \forall x \in \Omega, \quad \forall t \leq T.
\]

Proof. Because of the properties of the Legendre-Fenchel inequality, it follows that $d\eta^2_{x,t} \geq 0$ and $d\eta^2_{x,t} = 0$ if and only if the admissible solution does satisfy the evolution law at time $t$. As a result, $e_{\text{dis}}^2(t)$ is a non negative increasing scalar function. Therefore if $e_{\text{dis}}^2(T) = 0$, it is also $e_{\text{dis}}^2(t) = 0 \quad \forall t \leq T$, which then means that $s_{ad}(x,t)$ does satisfy the evolution law $\forall x \in \Omega, \quad \forall t \leq T$, i.e., $s_{ad}(x,t) = s_{ex}(x,t) \quad \forall x \in \Omega, \quad \forall t \leq T$.

Remark 3.7. The dissipation error $e_{\text{dis}}^2(T)$ is finite if and only if

\[
(\sigma_{ad}, X_{ad}, R_{ad}) \in \text{dom} \varphi^* \quad \forall x \in \Omega, \quad \forall t \leq T
\]

\[
(\epsilon_{ad}^p, \alpha_{ad}, p_{ad}) \in \text{dom} \varphi^* \quad \forall x \in \Omega, \quad \forall t \leq T
\]

where $\text{dom}$ stands for the effective domain of the function. These conditions are easy to impose for a model with linear hardening and linear elasticity in case of the convex elastic domain. For more general hardening laws, however, the model is required to be expressed first in normal form as introduced in Ladevèze (1989), that is, the state laws must be transformed in a linear form. For further details on the meaning of transformation of internal variables and conditions under which the above transformation is feasible, we refer to Ladevèze (1999) and Nguyen (2000).
Finally, note that if we let $t_n \in ]0, T[$, for the additivity of the integral, equation (3.44) can be also written as

$$e_{\text{dis}}^2(T) = e_{\text{dis}}^2(t_n) + 2 \int_\Omega \int_{t_n}^T d \int_{\Omega \times t} d t d \Omega (3.45)$$

which presents the global error at the time $T$ as a sum of the error at the time $t_n < T$ and the error associated with the admissible solution over $[t_n, T]$.

### 3.5.1.2 Extended Dissipation Error

This error measure has been introduced for the first time in Ladevèze et al. (1999) and Ladevèze (2001), where an application is given for an elastic-damage coupled model.

**Definition of the Admissibility Conditions**

An immediate extension of the dissipation error introduced in the previous section is obtained by removing the state laws from the definition of the admissibility conditions. This allows the recovery of the error in the constitutive equations for an admissible solution which is elastic, as introduced in Section 2.2.1.2 at page 18. More precisely, the field $(\sigma_{ad}(x, t), X_{ad}(x, t), R_{ad}(x, t), u_{ad}(x, t), \epsilon_{ad}(x, t), \epsilon_{ad}^p(x, t), \alpha_{ad}(x, t), p_{ad}(x, t))$ is an admissible solution with respect to the computation of the extended dissipation error if the following conditions are met

**Kinematic Compatibility:**

- Continuity of the Displacement Field, $u_{ad}(x, t)$.
- Time continuity of the Total Strain, $\epsilon_{ad}(x, t) = \nabla_s u_{ad}(x, t)$.
- Time continuity of the Plastic Strain, $\epsilon_{ad}^p(x, t)$.
- Time continuity of the Internal Variables, $\alpha_{ad}(x, t), p_{ad}(x, t)$.
- Displacement Boundary Conditions.

**Additivity of the Strain Tensor:**

$$\epsilon_{ad}(x, t) = \epsilon_{ad}^e(x, t) + \epsilon_{ad}^p(x, t),$$

$\forall x \in \Omega, \forall t \in [0, T]$. 

**Equilibrium:**

$$\langle \sigma_{ad}(x, t), \nabla \eta(x) \rangle = \langle b(x, t), \eta(x) \rangle + \langle t(x, t), \eta(x) \rangle_{\partial \Omega},$$

$\forall \eta \in V_0, \forall t \in [0, T]$. 

76
Initial Conditions:
\[ \epsilon_{ad}^p(x, t = 0) = 0, \]
\[ \alpha_{ad}(x, t = 0) = 0, \quad p_{ad}(x, t = 0) = 0 \quad \forall x \in \Omega \]

**Definition of Error**

In this case, the equations that are not satisfied by an admissible solution are the state laws and the evolution laws. Therefore, besides the residual in the evolution laws for which the same considerations as in the previous section apply, now also the residual in the state laws must be considered to assess the quality of the approximation associated with the given admissible solution. A natural measure of this residual is provided by the equivalent formulation of the state equations which exploits the convexity properties of the law, as expressed notably by the equations (3.7) and (3.9). Furthermore, given the nature of the state laws that relate the current value of the kinematic variables to the corresponding static one, a global measure of the error is obtained by assuming an \( L^\infty \) accumulation in time of the current value of the error in the state laws. Therefore, it is quite natural to assume the following definition of error

\[ e_{ext}^2(T) = \sup_{t \leq T} \left\{ 2 \int_{\Omega} s_l^2 \eta_{x,t}^2(\sigma_{ad}, X_{ad}, R_{ad}; \epsilon_{ad}, \alpha_{ad}, p_{ad}) d\Omega + \right. \]
\[ \left. \rightarrow \eta_{x,t}^2(t) d\Omega \right\} \]

where, in general, the quantity

\[ s_l^2 \eta_{x,t}^2(\sigma_{ad}, X_{ad}, R_{ad}; \epsilon_{ad}, \alpha_{ad}, p_{ad}) = \]
\[ = \psi^*(\sigma_{ad}(\mathbf{x}, t), X_{ad}(\mathbf{x}, t), R_{ad}(\mathbf{x}, t)) + \psi^*(\epsilon_{ad}(\mathbf{x}, t), \alpha_{ad}(\mathbf{x}, t), p_{ad}(\mathbf{x}, t)) + \]
\[ - \left\{ \sigma_{ad}(\mathbf{x}, t) : \epsilon_{ad}(\mathbf{x}, t) + X_{ad}(\mathbf{x}, t) : \alpha_{ad}(\mathbf{x}, t) + R_{ad}(\mathbf{x}, t)p_{ad}(\mathbf{x}, t) \right\} \]

is the residual in the state laws, and, likewise to the dissipation error, the term

\[ d^2 \eta_{x,t}^2(\sigma_{ad}, X_{ad}, R_{ad}; \epsilon_{ad}^p, \alpha_{ad}, \dot{p}_{ad}) = \]
\[ = \varphi^*(\sigma_{ad}(\mathbf{x}, t), X_{ad}(\mathbf{x}, t), R_{ad}(\mathbf{x}, t)) + \varphi(\epsilon_{ad}(\mathbf{x}, t), -\alpha_{ad}(\mathbf{x}, t), -\dot{p}_{ad}(\mathbf{x}, t)) + \]
\[ -\sigma_{ad}(\mathbf{x}, t) : \epsilon_{ad}^p(\mathbf{x}, t) + X_{ad}(\mathbf{x}, t) : \alpha_{ad}(\mathbf{x}, t) + R_{ad}(\mathbf{x}, t)p_{ad}(\mathbf{x}, t). \]

describes the residual produced in the evolution laws.
Remark 3.8. If we recall the result**, 
\[
\text{Given } f(t) \geq 0 \quad \forall t \leq T \\
(sup_{t \leq T} f(t))^{\frac{1}{2}} = \sup_{t \leq T} f^2(t),
\]
we can also write
\[
e_{ext}(T) = \sup_{t \leq T} \left\{ 2 \int_{\Omega} \eta_{ex,t}^2(\sigma_{ad}, \mathbf{X}_{ad}, R_{ad}; e_{ad}, \alpha_{ad}, p_{ad}) d\Omega + \right.
\]
\[
\left. + 2 \int_{\Omega} \int_0^T \eta^2_{ex,t}(\sigma_{ad}, \mathbf{X}_{ad}, R_{ad}; \dot{e}_{ad}, \dot{\alpha}_{ad}, \dot{p}_{ad}) d\tau d\Omega \right\}^{\frac{1}{2}}.
\]

Thus, the extended dissipation error has the form of an \(L^\infty\) norm in time of an energy type norm of the error in the state variables. \(\square\)

Definition (3.46) can be assumed as a global measure of the error of the (kinematic) admissible solution in the following sense:

**Theorem 3.2.**

Given an admissible solution \(s_{ad} = s_{ad}(\mathbf{x}, t)\) with respect to the computation of the extended dissipation error, i.e., \(s_{ad}\) is not required to meet the state laws, it follows
\[
e_{ext}^2(T) \geq 0
\]
\[
e_{ext}^2(T) = 0 \iff s_{ad}(\mathbf{x}, t) = s_{ex}(\mathbf{x}, t) \quad \forall \mathbf{x} \in \Omega, \quad \forall t \leq T.
\]

**Proof.** Because of the properties of the Legendre-Fenchel inequality, we recall that \(sl_{ex,t}^2 \geq 0\) and \(d_{ex,t}^2 \geq 0\). Also, \(sl_{ex,t}^2 = 0\), \(d_{ex,t}^2 = 0\) if and only if the given time continuous admissible solution \(s_{ad}(\mathbf{x}, t)\) satisfies the state laws and the evolution laws, respectively. As a result, \(e_{ext}^2(T)\) appears as supremum of \(\theta_{ad}^2(t)\) and \(\theta_{ad}^2(t)\) which are non negative increasing scalar functions. Therefore, if \(e_{ext}^2(T) = 0\), it follows \(\theta_{ad}^2(t) = 0 \quad \forall t \leq T\) and \(\theta_{ad}^2(t) = 0 \quad \forall t \leq T\). Finally, due to the definition of \(\theta_{ad}^2(t)\) and \(\theta_{ad}^2(t)\), it follows
\[
sl_{ex,t}^2 = 0, \quad \forall \mathbf{x} \in \Omega, \quad \forall t \leq T
\]
\[
d_{ex,t}^2 = 0, \quad \forall \mathbf{x} \in \Omega, \quad \forall t \leq T.
\]

**From 0 \leq f(t) \leq sup_{t \leq T} f(t) \quad \forall t \leq T\) it follows sup_{t \leq T} f^2(t) \leq (sup_{t \leq T} f(t))^2 whereas from f^2(t) \leq sup_{t \leq T} f^2(t) \quad \forall t \leq T,\) one easily obtains (sup_{t \leq T} f(t))^2 \leq sup_{t \leq T} f^2(t). Also, given g(t) \geq 0 \quad \forall t \leq T,\)

let g(t) = f^2(t), it follows (sup_{t \leq T} g(t))^2 = sup_{t \leq T} f^2(t) \leq sup_{t \leq T} f(t) = sup_{t \leq T} f^2(t).
Thus, the given time continuous admissible solution, $s_{ad}(x, t)$ satisfies the state laws and the evolution laws, respectively, that is, it coincides with the exact solution, $s_{ex}(x, t)$.

The viceversa is trivial. □

**Remark 3.9.** The extended dissipation error $e_{ext}^2(T)$ is finite if and only if

$$(\sigma_{ad}, X_{ad}, R_{ad}) \in \text{dom } \varphi^*$$

$$(\epsilon_{ad}^p, \dot{\alpha}_{ad}, \dot{p}_{ad}) \in \text{dom } \varphi.$$  

Unlike the dissipation error, where the constraint of the state laws was imposed between kinematic and the conjugate static admissible variables, in this case the model is not required to be a priori transformed into normal form in order to compute the error, for the meeting of the above conditions is quite easy to realize due to the convexity of $\text{dom } \varphi^*$ and $\text{dom } \varphi$. □

Likewise equation (3.45), for any $t_n \in [0, T]$, the extended dissipation error (3.46) can also be expressed as follows

$$e_{ext}^2(T) = \text{Max} \left\{ e_{ext}^2(t_n), \right.$$  

$$\sup_{t_n \leq t \leq T} \left\{ 2 \int_{\Omega} \eta^2_{ext}(\sigma_{ad}, X_{ad}, R_{ad}; \epsilon_{ad}^p, \alpha_{ad}, p_{ad}) d\Omega + \theta^2_{d}(t_n) + 

+2 \int_{\Omega} \int_{t_n}^{t} \eta^2_{ext}(\sigma_{ad}, X_{ad}, R_{ad}; \epsilon_{ad}^p, \alpha_{ad}, p_{ad}) d\tau d\Omega \right\} \right\}.$$  

(3.47)

This presents the global error at the time $T$ in terms of the error at the time $t_n < T$ and the admissible solution over $[t_n, T]$.

### 3.5.2 Error in the constitutive equations for admissible solution with jump across time instant $t_n$

In the admissible conditions listed in the preceding section, time continuity had been always assumed. Object of this section is to show how the previous definitions of error in the constitutive equations can be extended to the case in which the hypothesis of time continuity is removed so that admissible solutions may include a discontinuity jump at a given time instant $t_n$, i.e., $s_{ad}(x, t_n^-) \neq s_{ad}(x, t_n^+)$. The need for relaxing continuity may arise, for example, in presence of a finite element solution having discontinuity jump at the time instant $t_n$ because of change of mesh, as will be seen in the following chapters.

In rate independent plasticity, the solution of the initial boundary value problem which governs the evolution of the continuum depends only on the sequence of load levels whereas time has just the function of ordering this sequence. This means...
that the response of the system under the loading paths depicted in Figure 3.7, for instance, is the same with respect to any value of the fictitious time step $\Delta t$ during which the load level is kept constant. In agreement with this behaviour, it can be assumed that the value of the admissible solution at $t_n^+$ is also the value at $t_n + \Delta t$ and is independent on $\Delta t$. In this way, a fictitious time continuous process over the time interval $[t_n, t_n + \Delta t]$ along which the discontinuity is assumed to be taking place can be defined, and one can analyse the error in the evolution law as the time step $\Delta t$ shrinks to zero.

An example of this process is illustrated in Figure 3.8 which refers to the time variation of an admissible plastic strain with discontinuity jump across the time instant $t_n$. Figure 3.8 also hints to the procedure used for the above extension which shows the formation of the $\delta$-Dirac at $t_n$ for the plastic strain rate because of the discontinuity jump in $\epsilon^p_{ad}(t)$ at $t_n$.

Under constant load level equal to $b(x, t_n)$, we consider a family of fictitious time continuous admissible solutions over $[t_n, t_n + \Delta t]$ and parameterized by $\Delta t$ having as limit the given admissible solution, that is, we consider

$$\sigma_{ad,\Delta t}(x, \tau), X_{ad,\Delta t}(x, \tau), R_{ad,\Delta t}(x, \tau);$$

$$\epsilon_{ad,\Delta t}(x, \tau), \epsilon^p_{ad,\Delta t}(x, \tau), \alpha_{ad,\Delta t}(x, \tau), p_{ad,\Delta t}(x, \tau),$$

such that, $\forall x \in \Omega$,

$$\lim_{\Delta t \to 0^+} \sigma_{ad,\Delta t}(x, \tau) = \sigma_{ad}(x, \tau); \quad \lim_{\Delta t \to 0^+} X_{ad,\Delta t}(x, \tau) = X_{ad}(x, \tau);$$

$$\lim_{\Delta t \to 0^+} R_{ad,\Delta t}(x, \tau) = R_{ad}(x, \tau);$$

$$\lim_{\Delta t \to 0^+} \epsilon_{ad,\Delta t}(x, \tau) = \epsilon_{ad}(x, \tau); \quad \lim_{\Delta t \to 0^+} \epsilon^p_{ad,\Delta t}(x, \tau) = \epsilon^p_{ad}(x, \tau);$$

$$\lim_{\Delta t \to 0^+} \alpha_{ad,\Delta t}(x, \tau) = \alpha_{ad}(x, \tau); \quad \lim_{\Delta t \to 0^+} p_{ad,\Delta t}(x, \tau) = p_{ad}(x, \tau).$$

Figure 3.7: Fictitious Load Time Variations
where \((\bullet)_{ad}(x, \tau)\) denote the functions with the time discontinuity jump.

With regard to each member of this family, the error in the evolution law can now be computed. Thus, if \(\forall x \in \Omega\) the following limit exists and is finite,

\[
\Delta \varepsilon_d^2(x, t_n) \equiv \lim_{\Delta t \to 0^+} \int_{t_n}^{t_n+\Delta t} \left\{ \varphi^*(\sigma_{ad,\Delta t}(x, \tau), X_{ad,\Delta t}(x, \tau), R_{ad,\Delta t}(x, \tau)) + 
+ \varphi(\dot{\varepsilon}_{ad,\Delta t}(x, \tau), -\dot{\alpha}_{ad,\Delta t}(x, \tau), -\dot{p}_{ad,\Delta t}(x, \tau)) + 
- \sigma_{ad,\Delta t}(x, \tau) : \dot{\varepsilon}_{ad,\Delta t}(x, \tau) + 
+ X_{ad,\Delta t}(x, \tau) : \dot{\alpha}_{ad,\Delta t}(x, \tau) + R_{ad,\Delta t}(x, \tau)\dot{\rho}_{ad,\Delta t}(x, \tau) \right\} d\tau
\]

(3.48)

it seems natural to assume the limit to be an error in the constitutive equations at the point \(x\) in presence of discontinuity.

Figure 3.8: Definition of Fictitious State Variables Variation
Remark 3.10. The jump in the state variables modifies only the error component that is associated with the evolution law, since these are the equations that involve the rate of the internal variables.

Furthermore, the additional term, \( \Delta \zeta_2^2(x, t_n) \), is always non negative as a result of limit of non negative functions due to the Legendre-Fenchel inequality. However, for the material models taken into account, the non negativity of the term \( \Delta \zeta_2^2(x, t_n) \) will be checked directly. Since \( \Delta \zeta_2^2(x, t_n) \geq 0 \), the jump in the admissible solution will always produce an increase of the error component associated with the dissipation.

3.5.2.1 Augmented Dissipation Error

The dissipation error at the time \( T \) of the admissible solution with jump across the time \( t_n \in ]0, T[ \) is given by

\[
\Delta e_{\text{dis}}^2(T) = 2 \int_0^T \int_\Omega \eta_\text{x,t}^2(\sigma_{\text{ad}}, \sigma_{\text{ad}}, \Delta \epsilon_{\text{p,ad}}, \Delta \epsilon_{\text{p,ad}}, \Delta \dot{p}_{\text{ad}}) \, dt \, d\Omega + 2 \int_\Omega \Delta \zeta_2^2(x, t_n) \, d\Omega.
\]

In order to compute \( \Delta \zeta_2^2(x, t_n) \), the admissible solution is required to satisfy the state equations, which here we present in the following form

\[
\sigma_{\text{ad},\Delta t}(x, \tau) = C[\epsilon_{\text{ad},\Delta t}(x, \tau) - \epsilon_{\text{ad},\Delta t}(x, \tau)];
\]

\[
X_{\text{ad},\Delta t}(x, \tau) = \Lambda \alpha_{\text{ad},\Delta t}(x, \tau); R_{\text{ad},\Delta t}(x, \tau) = g(\dot{p}_{\text{ad},\Delta t}(x, \tau))
\]

where \( \Lambda \) and \( g(p) \) are the same as for the Marquis–Chaboche plasticity model.

In the following, we provide the expression for \( \Delta \zeta_2^2(x, t_n) \) for the rate independent plasticity models introduced in section 3.2.4.5 by assuming linear hardening. This condition is here invoked in order to guarantee that by assuming a time linear interpolation of the kinematic admissible variables, the corresponding conjugate forces, given by (3.49), will also appear as time linear interpolation of the values at the ends of the time step. This results in the static admissibility of the conjugate forces because of the admissibility of the interpolant values and the convexity of the elastic domain.

Also, for each model, it will be shown that \( \Delta e_{\text{dis}}^2(x, t_n) \) is non negative and that by setting \( \Delta e_{\text{dis}}^2(x, t_n) \) equal to zero, we infer the time continuity at \( t_n \) of the admissible solution. This, finally, means that \( \Delta \zeta_2^2(x, t_n) \) characterizes effectively the discontinuity jump across \( t_n \).

The Prandtl–Reuss plasticity model

Continuous Admissible Fictitious Solutions.

We assume, \( \forall x \in \Omega \), continuous admissible fictitious solutions defined as follows

- Kinematic Admissible Solution:
  \( \epsilon_{\text{ad,\Delta t}}(x, \tau), \epsilon_{\text{ad,\Delta t}}'(x, \tau), p_{\text{ad,\Delta t}}(x, \tau) \) are obtained as linear interpolation over \([t_n, t_n + \Delta t]\) of the values at \( t_n \) and \( t_n + \Delta t \).
We also impose that

\[ F(\mathbf{v}, \mathbf{p}) \]

\( \Delta t \) are obtained by imposing the state laws. Because of the linearity of these laws, \( \sigma_{ad,\Delta t}(\mathbf{x}, \tau) \) and \( R_{ad,\Delta t}(\mathbf{x}, \tau) \) are given by linear interpolation over \([t_n, t_n + \Delta t] \)

\[ \sigma_{ad,\Delta t}(\mathbf{x}, \tau) \]

\[ \sigma_{ad,\Delta t}(\mathbf{x}, \tau) \]

\( \sigma_{ad,\Delta t}(\mathbf{x}, \tau) \)

\[ \left( \sigma_{ad}(\mathbf{x}, t_n^+), R_{ad}(\mathbf{x}, t_n^+) \right) \]

\[ \left( \sigma_{ad}(\mathbf{x}, t_n^-), R_{ad}(\mathbf{x}, t_n^-) \right) \]

\[ \Rightarrow \left( \sigma_{ad}(\mathbf{x}, \tau), R_{ad}(\mathbf{x}, \tau) \right) \in \mathbb{E}, \forall \tau \in [t_n, t_n + \Delta t], \]

\[ p_{ad}(\mathbf{x}, t_n^+ - p_{ad}(\mathbf{x}, t_n^-) \geq \| \epsilon_{ad}^p(\mathbf{x}, t_n^+) - \epsilon_{ad}^p(\mathbf{x}, t_n^-) \|, \]

\[ \text{Tr}(\epsilon_{ad}^p(\mathbf{x}, t_n^+) - \epsilon_{ad}^p(\mathbf{x}, t_n^-)) = 0. \]

\[ (3.50) \]

Condition (3.50) occurs because of the linearity of the state laws and convexity of the elastic domain.

**Dissipation Error across the time discontinuity**

Consider the expression for the dissipation error for this model given by equation (3.44), we have,

\[ \forall \mathbf{x} \in \Omega \]

\[ \Delta \epsilon_{ad}^2(\mathbf{x}, t_n) = \lim_{\Delta t \to 0^+} \int_{t_n}^{t_n + \Delta t} \left\{ R_0 \| \epsilon_{ad}^p(\mathbf{x}, \tau) \| + \right. \]

\[ -\sigma_{ad,\Delta t}(\mathbf{x}, \tau): \epsilon_{ad}^p(\mathbf{x}, \tau) + R_{ad,\Delta t}(\mathbf{x}, \tau) p_{ad,\Delta t}(\mathbf{x}, \tau) \}

\[ d\tau = \]

\[ = R_0 \| \epsilon_{ad}^p(\mathbf{x}, t_n^+) - \epsilon_{ad}^p(\mathbf{x}, t_n^-) \| + \]

\[ -\sigma_{ad}(\mathbf{x}, t_n^+) + \sigma_{ad}(\mathbf{x}, t_n^-) \]

\[ \frac{1}{2} \left( \epsilon_{ad}^p(\mathbf{x}, t_n^+) - \epsilon_{ad}^p(\mathbf{x}, t_n^-) \right) + \]

\[ + \frac{R_{ad}(\mathbf{x}, t_n^+) + R_{ad}(\mathbf{x}, t_n^-)}{2} (p_{ad}(\mathbf{x}, t_n^+) - p_{ad}(\mathbf{x}, t_n^-)) \]

\[ 83 \]
with the State Equations being satisfied at $t_n^-$ and $t_n^+$. 

**Theorem 3.3.**

Given the admissible solutions $s_{ad}(t_n^-)$ and $s_{ad}(t_n^+)$, which satisfy the state equations at $t_n^-$ and $t_n^+$ respectively, and the finite value error requirements, it follows that

$$\Delta \zeta_2^2(x, t_n) \geq 0$$

**Proof.** To keep the notation simple, let

$$\sigma^+ = \sigma_{ad}(x, t_n^+); \sigma^- = \sigma_{ad}(x, t_n^-);$$

$$\Delta \sigma = \sigma^+ - \sigma^-;$$

$$R^+ = R_{ad}(x, t_n^+), R^- = R_{ad}(x, t_n^-);$$

$$\Delta R = R^+ - R^-;$$

$$\Delta \epsilon = \epsilon_{ad}(x, t_n^+) - \epsilon_{ad}(x, t_n^-);$$

$$\Delta \epsilon^p = \epsilon_{ad}^p(x, t_n^+) - \epsilon_{ad}^p(x, t_n^-);$$

$$\Delta p = p_{ad}(x, t_n^+) - p_{ad}(x, t_n^-).$$

From the conditions giving a finite value to the error, it follows

$$\Delta p \geq \|\Delta \epsilon^p\| \quad \text{and} \quad R \geq 0.$$ 

Thus we get

$$R_0 \|\Delta \epsilon^p\| - \sigma^+: \Delta \epsilon^p + R^+ \Delta p \geq R_0 \|\Delta \epsilon^p\| - \sigma^+: \Delta \epsilon^p + R^+ \Delta \epsilon^p.$$ 

Since $\text{Tr} [\Delta \epsilon^p] = 0$, we can also write,

$$\sigma^+: \Delta \epsilon^p = \sigma_D^+: \Delta \epsilon^p \leq \|\sigma_D^+\| \|\Delta \epsilon^p\|.$$ 

Hence,

$$R_0 \|\Delta \epsilon^p\| - \sigma^+: \Delta \epsilon^p + R^+ \Delta \epsilon^p \geq$$

$$\geq R_0 \|\Delta \epsilon^p\| - \|\sigma_D^+\| \|\Delta \epsilon^p\| + R^+ \Delta \epsilon^p =$$

$$= (R_0 + R^+ - \|\sigma_D^+\|) \|\Delta \epsilon^p\| \geq 0,$$

that is,

$$R_0 \|\Delta \epsilon^p\| - \sigma^+: \Delta \epsilon^p + R^+ \Delta p \geq 0. \quad (3.52)$$

Likewise, it can be proven that

$$R_0 \|\Delta \epsilon^p\| - \sigma^-: \Delta \epsilon^p + R^- \Delta p \geq 0. \quad (3.53)$$
Thus, summing up term by term and for the linearity of the double contraction operator, it follows
\[
\Delta \zeta^2_d(x, t_n) = R_0 \| \Delta e^p \| - \frac{\sigma^+ + \sigma^-}{2} \cdot \Delta e^p + \frac{R^+ + R^-}{2} \Delta p \geq 0 \quad (3.54)
\]

\[\blacksquare\]

**Theorem 3.4.**

Given the admissible solutions \( s_{ad}(t_n^-) \) and \( s_{ad}(t_n^+) \), satisfying the state equations at \( t_n^- \) and \( t_n^+ \) respectively, and the finite value error requirements, the following expressions are valid

\[
\Delta \zeta^2_d(x, t_n) = 0 \Rightarrow \begin{cases} 
\sigma_{ad}(x, t_n^+) = \sigma_{ad}(x, t_n^-) \\
R_{ad}(x, t_n^+) = R_{ad}(x, t_n^-) \\
e_{ad}(x, t_n^+) = e_{ad}(x, t_n^-) \\
e^p_{ad}(x, t_n^+) = e^p_{ad}(x, t_n^-) \\
p_{ad}(x, t_n^+) = p_{ad}(x, t_n^-)
\end{cases}
\]

**Proof.** Because of (3.54), by accounting for (3.52) and (3.53), it follows

If 
\[\Delta \zeta^2_d(x, t_n) = 0 \Rightarrow \begin{cases} 
R_0 \| \Delta e^p \| - \sigma^+ : \Delta e^p + R^+ \Delta p = 0 \\
R_0 \| \Delta e^p \| - \sigma^- : \Delta e^p + R^- \Delta p = 0
\end{cases}\]

Thus,
\[
\Delta \sigma : \Delta e^p = \Delta R \Delta p \quad (3.55)
\]

From equilibrium at \( t_n^- \) and \( t_n^+ \) it follows

\[\int_{\Omega} \sigma^- : \nabla \eta d\Omega = \int_{\partial \Omega} b(x, t_n) \eta d\Omega + \int_{\partial \Omega} t(x, t_n) \eta d\Omega \quad \forall \eta \in V_0\]

\[\int_{\Omega} \sigma^+ : \nabla \eta d\Omega = \int_{\partial \Omega} b(x, t_n) \eta d\Omega + \int_{\partial \Omega} t(x, t_n) \eta d\Omega \quad \forall \eta \in V_0.\]

Hence, since \( \Delta e \in V_0 \), subtracting term by term, we get

\[\int_{\Omega} \Delta \sigma : \Delta e d\Omega = 0 \leftrightarrow \int_{\Omega} \Delta \sigma : (\Delta e^e + \Delta e^p) d\Omega = 0\]

\[\leftrightarrow \int_{\Omega} \Delta \sigma : \Delta e^e d\Omega + \int_{\Omega} \Delta \sigma : \Delta e^p d\Omega = 0 \quad (3.56)\]

In the dissipation error the state laws are satisfied, i.e.

\[
\Delta \sigma = C \Delta e^e
\]

\[
\Delta R = H \Delta p.
\]

85
Hence, it follows

\[ \Delta \sigma : \Delta \varepsilon = C \Delta \varepsilon : \Delta \varepsilon \geq 0 \]  (3.57)

\[ \Delta \sigma : \Delta \varepsilon^p = \Delta R \Delta p = H \Delta p^2 \geq 0. \]

By accounting for (3.55), from (3.56) the following results are obtained

\[ \Delta \sigma : \Delta \varepsilon = 0 \Rightarrow \Delta \sigma = 0 \Rightarrow \Delta \varepsilon = 0 \]

\[ \Delta \sigma : \Delta \varepsilon^p = \Delta R \Delta p = 0 \Rightarrow H \Delta p^2 = 0 \Rightarrow \Delta p = 0 \overset{(a)}{\Rightarrow} \Delta \varepsilon^p = 0 \]

where implication (a) follows from the condition imposed on the admissible solution to deliver a finite error, given by (3.50).

The standard Marquis–Chaboche modified plasticity model

**Continuous Admissible Fictitious Solutions.**

We assume, \forall \mathbf{x} \in \Omega, continuous admissible fictitious solutions defined as follows

- **Kinematic Admissible Solution:**
  \[ e_{ad,\Delta t}(\mathbf{x}, \tau), \dot{e}_{ad,\Delta t}^p(\mathbf{x}, \tau), \alpha_{ad,\Delta t}(\mathbf{x}, \tau), p_{ad,\Delta t}(\mathbf{x}, \tau) \] are obtained as linear interpolation over \([t_n, t_n + \Delta t]\) of the values at \(t_n\) and \(t_n + \Delta t\).

- **Static Admissible Solution:**
  \[ \sigma_{ad,\Delta t}(\mathbf{x}, \tau) = C[\varepsilon_{ad,\Delta t}(\mathbf{x}, \tau) - \dot{e}_{ad,\Delta t}^p(\mathbf{x}, \tau)], X_{ad,\Delta t}(\mathbf{x}, \tau) = \Lambda \alpha_{ad,\Delta t}(\mathbf{x}, \tau) \text{ and } R_{ad,\Delta t}(\mathbf{x}, \tau) = H p_{ad,\Delta t}(\mathbf{x}, \tau). \]

- **\sigma_{ad,\Delta t}(\mathbf{x}, \tau)** is in equilibrium with \(b(\mathbf{x}, t_n)\) because of the convexity of the equilibrium condition.

- \((\sigma_{ad,\Delta t}(\mathbf{x}, \tau), X_{ad,\Delta t}(\mathbf{x}, \tau), R_{ad,\Delta t}(\mathbf{x}, \tau)) \in \mathbb{E} \quad \forall \tau \in [t_n, t_n + \Delta t]\) because of the convexity of the elastic domain.

- **\varepsilon_{ad,\Delta t}(\mathbf{x}, \tau)** is kinematically admissible because of the convexity of the compatibility conditions.

**Finite Value Error Requirements**

We also impose that \forall \mathbf{x} \in \Omega the admissible solutions satisfy the following conditions

\[ \left\{ \begin{array}{l}
(\sigma_{ad}(\mathbf{x}, t_n^-), X_{ad}(\mathbf{x}, t_n^-), R_{ad}(\mathbf{x}, t_n^-)) \in \mathbb{E} \\
(\sigma_{ad}(\mathbf{x}, t_n^+), X_{ad}(\mathbf{x}, t_n^+), R_{ad}(\mathbf{x}, t_n^+)) \in \mathbb{E}
\end{array} \right\} \Rightarrow \\
\Rightarrow \left\{ \begin{array}{l}
(\sigma_{ad}(\mathbf{x}, \tau), X_{ad}(\mathbf{x}, \tau), R_{ad}(\mathbf{x}, \tau)) \in \mathbb{E} \\
\forall \tau \in [t_n, t_n + \Delta t].
\end{array} \right. \]  (3.58)

\[ p_{ad}(\mathbf{x}, t_n^+) - p_{ad}(\mathbf{x}, t_n^-) \geq \|\varepsilon_{ad}^p(\mathbf{x}, t_n^+) - \varepsilon_{ad}^p(\mathbf{x}, t_n^-)\|, \]
\[
\text{Tr}[e_{ad}^p(x, t^+_n) - e_{ad}^p(x, t^-_n)] = 0,
\]
\[
e_{ad}^p(x, t^+_n) - e_{ad}^p(x, t^-_n) = 0 \Rightarrow \alpha_{ad}(x, t^+_n) - \alpha_{ad}(x, t^-_n) = 0. \quad (3.59)
\]

Condition (3.58) occurs because of the linearity of the state laws and convexity of the elastic domain, whereas condition (3.59) derives from the definition (3.27) of \( \varphi(e^p, -\alpha, -\dot{p}) \).

**Dissipation Error across the time discontinuity**

For this model we have to distinguish the following two cases in relation to the definition (3.27)

\[\forall x \in \Omega \]

- If \( e_{ad}^p(x, t^+_n) - e_{ad}^p(x, t^-_n) = 0 \) then it can also be written \( \alpha_{ad}^p(x, t^+_n) - \alpha_{ad}^p(x, t^-_n) = 0 \) because of (3.59). By accounting for (3.27), the dissipation error takes the following expression

\[
\Delta \zeta^2_{ad}(x, t_n) = \lim_{\Delta t \to 0^+} \int_{t_n}^{t_n + \Delta t} R_{ad,\Delta t}(x, \tau) \dot{p}_{ad,\Delta t}(x, \tau) d\tau =
\]

\[= \frac{R_{ad}(x, t^+_n) + R_{ad}(x, t^-_n)}{2} (p_{ad}(x, t^+_n) - p_{ad}(x, t^-_n)), \quad (3.60)\]

with the State Equations being satisfied at \( t^-_n \) and \( t^+_n \).

- If \( e_{ad}^p(x, t^+_n) - e_{ad}^p(x, t^-_n) \neq 0 \), and recalling equation (3.27)1, the dissipation error takes the following expression

\[
\Delta \zeta^2_{ad}(x, t_n) = \lim_{\Delta t \to 0^+} \int_{t_n}^{t_n + \Delta t} \left\{ R_0 \| e_{ad,\Delta t}^p(x, \tau) \| + \frac{c}{2a} \frac{\| e_{ad,\Delta t}^p(x, \tau) - \dot{\alpha}_{ad,\Delta t}(x, \tau) \|^2}{\| e_{ad,\Delta t}^p(x, \tau) \|} - \sigma_{ad,\Delta t}(x, \tau) : e_{ad,\Delta t}^p(x, \tau) + X_{ad,\Delta t}(x, \tau) : \alpha_{ad,\Delta t}(x, \tau) + R_{ad,\Delta t}(x, \tau) \dot{p}_{ad,\Delta t}(x, \tau) \right\} d\tau =
\]

\[= \frac{R_0 \| e_{ad}^p(x, t^+_n) - e_{ad}^p(x, t^-_n) \|}{2a} \frac{\| e_{ad}^p(x, t^+_n) - e_{ad}^p(x, t^-_n) \|^2}{\| e_{ad}^p(x, t^+_n) - e_{ad}^p(x, t^-_n) \|} + \frac{c}{2a} \frac{\| (e_{ad}^p(x, t^+_n) - e_{ad}^p(x, t^-_n)) - (\alpha_{ad}(x, t^+_n) - \alpha_{ad}(x, t^-_n)) \|^2}{\| e_{ad}^p(x, t^+_n) - e_{ad}^p(x, t^-_n) \|} + \frac{\sigma_{ad}(x, t^+_n) + \sigma_{ad}(x, t^-_n)}{2} : (e_{ad}^p(x, t^+_n) - e_{ad}^p(x, t^-_n)) + \frac{X_{ad}(x, t^+_n) + X_{ad}(x, t^-_n)}{2} : (\alpha_{ad}(x, t^+_n) - \alpha_{ad}(x, t^-_n)) + \]

87
\[ + \frac{R_{ad}(x, t^+_n) + R_{ad}(x, t^-_n)}{2} (p_{ad}(x, t^+_n) - p_{ad}(x, t^-_n)), \]

with the State Equations being satisfied at \( t^-_n \) and \( t^+_n \).

**Theorem 3.5.**

Given the admissible solutions \( s_{ad}(t^-_n) \) and \( s_{ad}(t^+_n) \), satisfying the state equations at \( t^-_n \) and \( t^+_n \) respectively, and the finite value error requirements, it follows that

\[ \Delta \zeta^2_d(x, t_n) \geq 0 \]

**Proof.** If \( \Delta \epsilon^p = 0, \Delta \alpha^p = 0 \),

\[ \Delta \zeta^2_d(x, t_n) = \frac{R^+ + R^-}{2} \Delta p \geq 0 \]

because of the finite value error requirements.

If \( \Delta \epsilon^p \neq 0 \),

\[
\Delta \zeta^2_d(x, t_n) = R_0 \| \Delta \epsilon^p \| + \frac{c}{2a} \| \Delta \epsilon^p - \Delta \alpha \|^2 \frac{\sigma^+ + \sigma^-}{2 : \Delta \epsilon^p +} \\
+ \frac{X^+ + X^-}{2} \Delta \alpha + \frac{R^+ + R^-}{2} \Delta p. \tag{3.62}
\]

Let us consider the term

\[
R_0 \| \Delta \epsilon^p \| + \frac{c}{2a} \| \Delta \epsilon^p - \Delta \alpha \|^2 \frac{\sigma^+ + \sigma^-}{2 : \Delta \epsilon^p +} - \sigma : \Delta \epsilon^p + X : \Delta \alpha + R \Delta p, \tag{3.63}
\]

where \( \sigma, X, R \) can all refer either to the time instant \( t^+_n \) or \( t^-_n \). In the following, we will show that (3.63) is non negative, which then determines the non negativity of (3.62).

Since,

\[ \Delta p \geq \| \Delta \epsilon^p \|, \]

it follows

\[ R \Delta p \geq R \| \Delta \epsilon^p \|. \]

Also,

\[
\sigma : \Delta \epsilon^p = \sigma_D : \Delta \epsilon^p = (\sigma_D - X + X) : \Delta \epsilon^p = \\
= (\sigma_D - X) : \Delta \epsilon^p + X : \Delta \epsilon^p \leq \| \sigma_D - X \| \| \Delta \epsilon^p \| + X : \Delta \epsilon^p
\]

Hence,

\[ -\sigma : \Delta \epsilon^p \geq -\| \sigma_D - X \| \| \Delta \epsilon^p \| - X : \Delta \epsilon^p. \]
By accounting for the above inequalities, it results
\[ R_0 \| \Delta e^p \| + \frac{c}{2a} \frac{\| \Delta e^p - \Delta \alpha \|^2}{\| \Delta e^p \|} - \sigma : \Delta e^p + X : \Delta \alpha + R \Delta p \geq 0 \]
\[ \geq \frac{c}{2a} \frac{\| \Delta e^p - \Delta \alpha \|^2}{\| \Delta e^p \|} + (R_0 + R) \| \Delta e^p \| - \| \sigma_D - X \| \| \Delta e^p \| + X : (\Delta e^p - \Delta \alpha). \]

From the yield condition, it follows
\[ (R_0 + R) - \| \sigma_D - X \| \geq \frac{a}{2c} \| X \|^2, \]

hence,
\[ [(R_0 + R) - \| \sigma_D - X \|] \| \Delta e^p \| \geq \frac{a}{2c} \| X \| \| \Delta e^p \| \]

Also, it follows
\[ -X : (\Delta e^p - \Delta \alpha) \geq -\| X \| \| \Delta e^p - \Delta \alpha \| \]

so that finally, we have
\[ R_0 \| \Delta e^p \| + \frac{c}{2a} \frac{\| \Delta e^p - \Delta \alpha \|^2}{\| \Delta e^p \|} - \sigma : \Delta e^p + X : \Delta \alpha + R \Delta p \geq 0 \]
\[ \geq \frac{c}{2a} \frac{\| \Delta e^p - \Delta \alpha \|^2}{\| \Delta e^p \|} + \frac{a}{2c} \| X \| \| \Delta e^p \| - \| X \| \| \Delta e^p - \Delta \alpha \| = \]
\[ = \left( \sqrt{\frac{c}{2a} \| \Delta e^p - \Delta \alpha \|} - \sqrt{\frac{a}{2c} \| X \| \| \Delta e^p \|} \right)^2 \geq 0 \quad \square \] (3.64)

**Theorem 3.6.**

Given the admissible solutions \( s_{ad}(t_n^-) \) and \( s_{ad}(t_n^+) \), satisfying the state equations at \( t_n^- \) and \( t_n^+ \) respectively, and the finite value error requirements, it follows that

\[ \Delta \zeta_{ad}^2(x, t_n) = 0 \Rightarrow \]
\[ \begin{align*}
\sigma_{ad}(x, t_n^+) &= \sigma_{ad}(x, t_n^-) \\
X_{ad}(x, t_n^+) &= X_{ad}(x, t_n^-) \\
R_{ad}(x, t_n^+) &= R_{ad}(x, t_n^-) \\
e_{ad}(x, t_n^+) &= e_{ad}(x, t_n^-) \\
e^p_{ad}(x, t_n^+) &= e^p_{ad}(x, t_n^-) \\
\alpha_{ad}(x, t_n^+) &= \alpha_{ad}(x, t_n^-) \\
p_{ad}(x, t_n^+) &= p_{ad}(x, t_n^-)
\end{align*} \]
Proof. We again have to distinguish the two cases in relation to the expression of the dissipation error as given by equation (3.60) and (3.61). We first consider the case of $\Delta \epsilon_p \neq 0$ and then $\Delta \epsilon_p = 0$, $\Delta \alpha = 0$.

If $\Delta \epsilon_p \neq 0$, given the expression (3.61), and accounting for (3.64), it follows

$$
\left\{ \begin{array}{l}
\sigma^+ : \Delta \epsilon_p = R_0 \| \Delta \epsilon_p \| + \frac{c}{2a} \frac{\| \Delta \epsilon_p - \Delta \alpha \|}{\| \Delta \epsilon_p \|} + \\
+ R^+ \Delta p + X^+ : \Delta \alpha \\
\sigma^{-} : \Delta \epsilon_p = R_0 \| \Delta \epsilon_p \| + \frac{c}{2a} \frac{\| \Delta \epsilon_p - \Delta \alpha \|}{\| \Delta \epsilon_p \|} + \\
+ R^- \Delta p + X^- : \Delta \alpha 
\end{array} \right.
$$

thus,

$$
\Delta \sigma : \Delta \epsilon_p = \Delta R \Delta p + \Delta X : \Delta \alpha
$$

(3.65)

From equilibrium at $t_n^-$ and $t_n^+$ it follows (see equation (3.56))

$$
\int_{\Omega} \Delta \sigma : \Delta \epsilon d\Omega = 0 \Leftrightarrow \int_{\Omega} \Delta \sigma : \Delta \epsilon^e d\Omega + \int_{\Omega} \Delta \sigma : \Delta \epsilon^p d\Omega = 0
$$

(3.66)

In the dissipation error the state laws are satisfied, i.e.

$$
\Delta \sigma = C \Delta \epsilon^e \\
\Delta R = H \Delta p \\
\Delta X = \Lambda \Delta \alpha
$$

hence, it follows

$$
\Delta \sigma : \Delta \epsilon^e = C \Delta \epsilon^e : \Delta \epsilon^e \geq 0
$$

(3.67)

$$
\Delta \sigma : \Delta \epsilon^p = \Delta R \Delta p + \Delta X : \Delta \alpha = H \Delta p^2 + \Lambda \Delta \alpha : \Delta \alpha \geq 0
$$

for $\Lambda$ is a second order positive definite tensor.

By accounting for (3.67), from (3.66) it results

$$
\Delta \sigma : \Delta \epsilon^e = 0 \Rightarrow \Delta \sigma = 0 \Rightarrow \Delta \epsilon^e = 0
$$

$$
\Delta \sigma : \Delta \epsilon^p = \Delta R \Delta p + \Delta X : \Delta \alpha = 0 \Rightarrow
$$

$$
\Rightarrow \left\{ \begin{array}{l}
H \Delta p^2 = 0 \Rightarrow \Delta p = 0 \overset{(a)}{\Rightarrow} \Delta \epsilon^p = 0 \\
\Lambda \Delta \alpha : \Delta \alpha = 0 \Rightarrow \Delta \alpha = 0
\end{array} \right.
$$

where implication (a) follows from the condition imposed on the admissible solution to deliver a finite error.
If $\Delta \epsilon^p = 0, \Delta \alpha = 0$, the dissipation error is given by

$$\Delta \zeta^2_d(x, t_n) = \frac{R^- + R^+}{2} \Delta p = \frac{H}{2}(p^{+2} - p^{-2})$$

where we have taken into account the meeting of the state equations at $t_n^-$ and $t_n^+$. Thus, it follows

$$\Delta \zeta^2_d(x, t_n) = 0 \Rightarrow p^{+2} = p^{-2} \Rightarrow p^+ = p^-, \text{ for } p \geq 0$$

which delivers

$$\Delta p = 0 \Rightarrow \Delta R = 0$$

From equations (3.66) and (3.67) we derive also

$$\Delta \sigma = 0$$

### 3.5.2.2 Augmented Extended Dissipation Error

The extended dissipation error at the time $T$ of the admissible solution with jump across the time $t_n \in ]0, T[$ is given by

$$\Delta \epsilon_{ext}^2(T) = \sup_{t \leq T} \left\{ 2 \int_{\Omega} s_l^T u d \Omega + \theta^2_{\Delta}(t) \right\} + 2 \int_{\Omega} \int_{t_n^-}^{t+n} d \eta^2_{x, \tau} d \Omega + \theta^2_{\Delta}(t) + 2 \int_{\Omega} \int_{t_n^-}^{t+n} d \eta^2_{x, \tau} d \Omega$$

where for $\Delta \zeta^2_d(x, t_n)$ we consider the same expressions as given in Section 3.5.2.1. For the applications, it is convenient to rewrite equation (3.68) as follows,

$$\Delta \epsilon_{ext}^2(T) = \max \left\{ \sup_{t \leq t_n^-} \left[ 2 \int_{\Omega} s_l^T u d \Omega + 2 \int_{\Omega} \int_{t_n^-}^{t+n} d \eta^2_{x, \tau} d \Omega \right], \right\}$$

which highlights the different contributions to the error from the parts of the admissible solution which are continuous in time.

The next result guarantees that definition (3.68) can be assumed as measure of the error in the following sense
Theorem 3.7.
Given the admissible solutions $s_{ad}(t^-)$ and $s_{ad}(t^+)$, meeting the finite value error requirements and not necessarily the state equations at $t^-$ and $t^+$, respectively, it follows that

$$
\Delta e^2_{ext}(T) \geq 0
$$

$$
\Delta e^2_{ext}(T) = 0 \iff s_{ad}(x,t) = s_{ex}(x,t) \quad \forall x \in \Omega, \quad \forall t \leq T.
$$

Proof. The extended dissipation error $\Delta e^2_{ext}(T)$ is defined as the supremum of a function which is sum of non negative terms, thus

$$
\Delta e^2_{ext}(T) = 0 \iff \begin{cases}
\theta^2_{sl}(t) = 0 \forall t \leq T & \iff \Delta e^2_{ad}(x,t_n) = 0 \\
\forall x \in \Omega, \quad \forall t \leq T. & (a)
\end{cases}
$$

$$
\Delta \theta^2_d(T) = 0 \forall t \leq T & \iff \Delta \theta^2_d(T) = 0 \\
\forall x \in \Omega, \quad \forall t \leq T. & (b)
$$

$$
\Delta \delta^2_{d}(t) = 0 \forall t \leq T & \iff d \delta^2_{d}(t) = 0 \\
\forall x \in \Omega, \quad \forall t \leq T. & (c)
$$

(3.70)

Condition (3.70a) means that the admissible solution $s_{ad}(x,t)$ satisfies the state laws, $\forall x \in \Omega, \quad \forall t \leq T$. This condition along with (3.70b) allows one to conclude also that the jump in all the variables is zero as a result of the argument given in Section 3.5.2.1 for the rate independent plasticity models taken into account. Finally, condition (3.70c) allows to conclude that the time continuous admissible solution satisfies also the evolution law.

Even for the extended dissipation error a characterization of only the discontinuity can be given. Hereafter, the condition is proved only for the Prandtl–Reuss model with linear hardening. The argument applies likewise to the standard variant Marquis–Chaboche model with linear hardening. For both the models with more general hardening laws, we believe, however, that similar conclusions can be obtained.

The condition characterizing the discontinuity is next given in a more general format which applies to admissible solutions with jump across time instant $t_n$ in the case of rate-independent plasticity.

Denote by

$$
s_{ad}(x,t_n) = (\sigma_{ad}(x,t_n), R_{ad}(x,t_n); e_{ad}(x,t_n), e^p_{ad}(x,t_n), p_{ad}(x,t_n))
$$

and

$$
s_{ad}(x,t_n + \Delta t) = (\sigma_{ad}(x,t_n + \Delta t), R_{ad}(x,t_n + \Delta t); e_{ad}(x,t_n + \Delta t), e^p_{ad}(x,t_n + \Delta t), p_{ad}(x,t_n + \Delta t))
$$
any admissible solution at \( t_n \) and \( t_n + \Delta t \), respectively, corresponding to the same load level and with \( s_{ad}(t) \) the admissible solution obtained as time linear interpolation over \([t_n, t_n + \Delta t]\) of \( s_{ad}(t_n) \) and \( s_{ad}(t_n + \Delta t) \). The following theorem, then, can be stated

**Theorem 3.8.**

Given the admissible solutions \( s_{ad}(t_n) \) and \( s_{ad}(t_n + \Delta t) \), corresponding to the same load level, meeting the finite value error requirements and not necessarily the state equations at \( t_n \) and \( t_n + \Delta t \), respectively, it follows \( \forall \mathbf{x} \in \Omega \),

\[
\begin{align*}
\text{IF} & \quad \begin{cases}
  s^L_{\mathbf{x}, t_n}^2 = s^L_{\mathbf{x}, t_n + \Delta t} = s^L_{\mathbf{x}, t} \\
  \forall t \in [t_n, t_n + \Delta t]. \\
  \Delta \mathbf{e}_{\mathbf{x}, ad}^2(\mathbf{x}, t_n) = 0
\end{cases} \\
\Rightarrow & \quad \begin{cases}
  \sigma_{ad}(\mathbf{x}, t_n) = \sigma_{ad}(\mathbf{x}, t_n + \Delta t) \\
  R_{ad}(\mathbf{x}, t_n) = R_{ad}(\mathbf{x}, t_n + \Delta t) \\
  \mathbf{e}_{ad}(\mathbf{x}, t_n) = \mathbf{e}_{ad}(\mathbf{x}, t_n + \Delta t) \\
  \mathbf{p}_{ad}(\mathbf{x}, t_n) = \mathbf{p}_{ad}(\mathbf{x}, t_n + \Delta t)
\end{cases}
\end{align*}
\]

**Proof.** For the Prandtl–Reuss model with linear hardening, the hypothesis of the theorem concerning the error in the state law writes as

\[
\begin{align*}
\int_{\Omega} (\sigma_{ad}(t) - C \mathbf{e}_{ad}(t)) : C^{-1} (\sigma_{ad}(t) - C \mathbf{e}_{ad}(t)) d\mathbf{x} + \\
+ \frac{1}{H} \int_{\Omega} (R_{ad}(t) - H \mathbf{p}_{ad}(t))^2 d\mathbf{x} = \text{const} \quad \forall t \in [t_n, t_n + \Delta t].
\end{align*}
\]

Thus, differentiation with respect to time delivers

\[
\begin{align*}
\int_{\Omega} (\sigma_{ad}(t) - C \mathbf{e}_{ad}(t)) : C^{-1} (\dot{\sigma}_{ad}(t) - C \dot{\mathbf{e}}_{ad}(t)) d\mathbf{x} + \\
+ \frac{1}{H} \int_{\Omega} (R_{ad}(t) - H \mathbf{p}_{ad}(t))(\dot{R}_{ad}(t) - H \dot{\mathbf{p}}_{ad}(t)) d\mathbf{x} = 0 \quad \forall t \in [t_n, t_n + \Delta t]
\end{align*}
\]

which, because of the definition of \( s_{ad}(t) \), can be written as

\[
\begin{align*}
\int_{\Omega} (\sigma_{ad}(t) - C \mathbf{e}_{ad}(t)) : C^{-1} (\Delta \sigma_{ad} - C \Delta \mathbf{e}_{ad}) d\mathbf{x} + \\
+ \frac{1}{H} \int_{\Omega} (R_{ad}(t) - H \mathbf{p}_{ad}(t))(\Delta R_{ad} - H \Delta \mathbf{p}_{ad}) d\mathbf{x} = 0 \quad \forall t \in [t_n, t_n + \Delta t]
\end{align*}
\]

(3.71)

where the same notation as in the proof of Theorem 3.3 has been employed provided that \( t_n^* \) is meant as \( t_n \) and \( t_n^* \) as \( t_n + \Delta t \).
After computing equation (3.71) for \( t = t_n \) and \( t = t_n + \Delta t \) and subtracting side to side one obtains
\[
\int_{\Omega} (\Delta \sigma_{ad} - C \Delta \epsilon^e_{ad}) : C^{-1}(\Delta \sigma_{ad} - C \Delta \epsilon^e_{ad}) \, dx + \frac{1}{H} \int_{\Omega} (\Delta R_{ad} - H \Delta p_{ad})(\Delta R_{ad} - H \Delta p_{ad}) \, dx = 0,
\]
that is,
\[
\int_{\Omega} (\Delta \sigma_{ad} - C \Delta \epsilon^e_{ad}) : C^{-1}(\Delta \sigma_{ad} - C \Delta \epsilon^e_{ad}) \, dx = 0
\]
\[
\frac{1}{H} \int_{\Omega} (\Delta R_{ad} - H \Delta p_{ad})^2 \, dx = 0,
\]
so that the following relations hold
\[
\Delta \sigma_{ad} - C \Delta \epsilon^e_{ad} = 0
\]
\[
\Delta R_{ad} - H \Delta p_{ad} = 0,
\]
and the same arguments as in the proof of Theorem 3.3 can now be adopted. □

### 3.5.3 Definition of error in solution

Let
\[
s_{ex}(x,t) = (\sigma_{ex}(x,t), \mathbf{X}_{ex}(x,t), R_{ex}(x,t); \epsilon_{ex}(x,t), \epsilon^p_{ex}(x,t), \alpha_{ex}(x,t), p_{ex}(x,t))
\]
denote the exact solution of the initial boundary value problem defined in section 3.2, that is, \( s_{ex}(x,t) \) is the time continuous function that meets all the equations given in Box 3.1.

Let
\[
s_{ad}^{kin}(x,t) = (\mathbf{u}_{ad}(x,t), \epsilon^p_{ad}(x,t), \alpha_{ad}(x,t), p_{ad}(x,t)), \quad (3.72)
\]
be a kinematically admissible solution with \( \mathbf{u} \) meeting the compatibility conditions, and \( \epsilon^p_{ad}(x,t), \alpha_{ad}(x,t), p_{ad}(x,t) \) meeting the initial conditions. The kinematically admissible solution may also present discontinuity jump across time instants \( t_n \). We assume the error in the constitutive equations produced by
\[
s_{ex,ad} = (\sigma_{ex}, \mathbf{X}_{ex}, R_{ex}; \mathbf{u}_{ad}, \epsilon^p_{ad}, \alpha_{ad}, p_{ad})
\]
as global measure of the exact error in solution associated with \( s_{ad}^{kin} \).

This is defined as
\[
e^2_{ex}(T) = \sup_{t \leq T} \theta^2_{ex}(t), \quad (3.73)
\]

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94
where
\[
\theta_{ex}^2(t) = 2 \int_\Omega s \eta_{ex,t}^2(\sigma_{ex}, X_{ex}, R_{ex}; \epsilon_{ad}, \alpha_{ad}, p_{ad}) d\Omega + \\
+ 2 \int_\Omega \int_0^t d \eta_{ex,t}^2(\sigma_{ex}, X_{ex}, R_{ex}; \epsilon^p_{ad}, \alpha_{ad}, \dot{p}_{ad}) d\tau d\Omega.
\]

The following heuristic argument show that this definition of error is meaningful. First, note that
\[
e_{ex}^2(T) \text{ FINITE } \Rightarrow \left\{ \begin{array}{l}
(\sigma_{ex}, X_{ex}, R_{ex}) \in \mathbb{E} \quad \forall x \in \Omega, \quad \forall t \leq T \\
(\dot{\epsilon}_{ad}, \dot{\alpha}_{ad}, \dot{p}_{ad}) \in \mathbb{C} \quad \forall x \in \Omega, \quad \forall t \leq T
\end{array} \right. \tag{3.74}
\]

where \(\mathbb{E} \times \mathbb{C}\) is the effective domain of the function \(d \eta_{ex,t}^2(\sigma, X, R; \dot{\epsilon}, \dot{\alpha}, \dot{p})\). For the plasticity models under consideration, if the admissible solution is discontinuous across the time instant \(t_n\), conditions (3.74b) imply that
\[
\Delta p_{ad} \geq \|\Delta \epsilon_{ad}^p\|. \tag{3.75}
\]

Now, we can give the following result:

**Theorem 3.9.**

Given a kinematic admissible solution \(s_{ad}^{\text{kin}} = (u_{ad}, \epsilon_{ad}, \alpha_{ad}, p_{ad})\), it follows that
\[
e_{ex}^2(T) = 0 \text{ IF and ONLY IF } \left\{ \begin{array}{l}
\text{s_{ex,ad} is time continuous} \\
\text{s_{ex,ad}(x, t) = s_{ex}(x, t) } \forall x \in \Omega, \forall t \leq T
\end{array} \right. \tag{3.76}
\]

**Proof.** Denote with
\[
\tilde{\sigma} = C[\epsilon_{ad} - \epsilon_{ad}^p], \\
\tilde{X} = \Lambda \alpha_{ad}, \\
\tilde{R} = g(p_{ad})
\]

the forces conjugate to the admissible variables \((\epsilon_{ad}, \epsilon_{ad}^p, p_{ad}, \alpha_{ad})\). Since \(e_{ex}^2(T)\) is defined as the supremum of a function which is sum of non negative terms, it follows
\[
e_{ex}^2(T) = 0 \Rightarrow \left\{ \begin{array}{l}
s_{ex,ad} = 0 \quad \forall x \in \Omega, \quad \forall t \leq T, \quad (a) \\
d \eta_{ex,t}^2 = 0 \quad \forall x \in \Omega, \quad \forall t \leq T. \quad (b)
\end{array} \right. \tag{3.77}
\]

Condition (a) means that
\[
\tilde{\sigma}(x, t) = \sigma_{ex}(x, t) \quad \forall x \in \Omega, \quad \forall t \leq T, \\
\tilde{X}(x, t) = X_{ex}(x, t) \quad \forall x \in \Omega, \quad \forall t \leq T, \\
\tilde{R}(x, t) = R_{ex}(x, t) \quad \forall x \in \Omega, \quad \forall t \leq T.
\]
Because of the time continuity of \((\sigma_{ex}(x, t), X_{ex}(x, t), R_{ex}(x, t))\) and of the continuity of the functional relations that define the state equations, the time continuity of \((\epsilon_{ad}^p, p_{ad}, \alpha_{ad})\) also follows. In turn, the time continuity of \(p_{ad}\), along with the condition (3.75) implies also the continuity of \(\epsilon_{ad}^p\), hence the continuity of \(\epsilon_{ad} = \epsilon_{ad}^e + \epsilon_{ad}^p\) as well. Then, it follows that

\[
\left( \tilde{\sigma}, \tilde{X}, \tilde{R}; u_{ad}, \epsilon_{ad}^p, \alpha_{ad}, p_{ad} \right) = s_{ex, ad}
\]

is time continuous, statically admissible, meets the state laws and the evolution laws. Thus it coincides with the exact solution, \(s_{ex}\).

### 3.5.3.1 Extension of the Prager-Synge theorem to the Dissipation Error

In the case of the dissipation error, for a time continuous admissible solution \(s_{ad}\), Ladevèze (1999) shows that the following equation holds for a normal formulation of the model

\[
\int_0^t \int_\Omega d_\eta^2_{\xi,\tau}(s_{ad}, s_{ex}) d\tau d\Omega =
\]

\[
= \frac{1}{2} \int_\Omega (\sigma_{ex} - \sigma_{ad}) : C^{-1}(\sigma_{ex} - \sigma_{ad}) d\Omega_t +
\]

\[
+ \frac{1}{2} \int_\Omega (X_{ex} - X_{ad}) : \Lambda^{-1}(X_{ex} - X_{ad}) d\Omega_t +
\]

\[
+ \frac{1}{2} \int_\Omega \left[ (R_{ex} - R_{ad}) H^{-1}(R_{ex} - R_{ad}) \right] d\Omega_t + (3.76)
\]

where

\[
d_\eta^2_{\xi,\tau}(s_{ad}, s_{ex}) = d_\eta^2_{\xi,\tau}(s_{ex, ad}) + d_\eta^2_{\xi,\tau}(s_{ad, ex})
\]

with

\[
s_{ex, ad} = \left( \sigma_{ex}, X_{ex}, R_{ex}; u_{ad}, \epsilon_{ad}^p, \alpha_{ad}, p_{ad} \right),
\]

\[
s_{ad, ex} = \left( \sigma_{ad}, X_{ad}, R_{ad}; u_{ex}, \epsilon_{ex}^p, \alpha_{ex}, p_{ex} \right),
\]

which can be easily obtained by elaborating on the expression of \(d_\eta^2_{\xi,\tau}(s_{ad})\) and accounting of the properties of the exact solution \(s_{ex}\).

Equation (3.76) can be considered as an extension of the Prager-Synge theorem to the Dissipation Error and, likewise for the linear elasticity, allows one to show easily that the dissipation error is an upper bound for the error in the solution as defined by equation (3.73). Indeed, we have the following result:

**Theorem 3.10.**

Given an admissible solution, \(s_{ad}\), with respect to the computation of the dissipation error, it follows

\[
e_{dis}^2(T) \geq e_{ex}^2(T) \quad (3.77)
\]
Proof. Let
\[
\theta_{ex}^2(t) = \int_{\Omega} (\sigma_{ex} - \sigma_{ad}) : C^{-1}(\sigma_{ex} - \sigma_{ad}) d\Omega |_t + \\
+ \int_{\Omega} (X_{ex} - X_{ad}) : \Lambda^{-1}(X_{ex} - X_{ad}) d\Omega |_t + \\
+ \int_{\Omega} \left[ (R_{ex} - R_{ad}) H^{-1}(R_{ex} - R_{ad}) \right] d\Omega |_t + \\
+ 2 \int_0^t \int_0^t \eta_{x,\tau}^2(s_{ex,ad}) d\tau d\Omega
\]
and
\[
\theta_{ad}^2(t) = 2 \int_{\Omega} \int_0^t \eta_{x,\tau}^2(s_{ad}) d\tau d\Omega.
\]
Since \(d\eta_{x,\tau}^2(s_{ex,ad}) \geq 0\) and \(d\eta_{x,\tau}^2(s_{ad,ex}) \geq 0\), it follows from equation (3.76),
\[
\theta_{ad}^2(t) \geq \theta_{ex}^2(t) \quad \forall \ t \leq T.
\]
Thus, it is
\[
\sup_{t \leq T} \theta_{ad}^2(t) \geq \sup_{t \leq T} \theta_{ex}^2(t) = e_{ex}^2(T)
\]
where
\[
\sup_{t \leq T} \theta_{ad}^2(t) = \theta_{ad}^2(T) \equiv e_{dis}^2(T),
\]
and \(e_{dis}^2(T)\) is an increasing function of time. \(\Box\)

3.6 Concluding Remarks

This Chapter represents the theoretical core of the thesis. Here, the theory of the error in the constitutive equations for material models with internal variables and associative flow rule has been presented following the works of Ladevèze (1989) and Ladevèze et al. (1999).

The fundamental notion of admissible solution has been given. The error in the constitutive equations developed by Ladevèze et al. (1999) has been extended to admissible solutions with discontinuity jump at the time instant \(t_n\) in the case of rate-independent plasticity models. Theorem 3.8 represents the main proposition of the Chapter. The theorem provides a characterization of the discontinuity jump in the admissible solution as a function of the augmented term \(\Delta \zeta_{x,d}^2(x, t_n)\) and of the behaviour of the error component associated with the residual in the state law.

As a result, the augmented extended dissipation error can be employed as a basis of methodology for the assessment of the global accuracy in time of finite element solutions on evolving meshes.
With this regard, in the next Chapter we first recall the governing finite element equations and subsequently, we continue discussing the several sources of discretization errors which are introduced, in particular those arising from the change of finite element mesh from one time increment to the other.
Part II

Application to the Finite Element Solution of the IBVP in Elasto-Plasticity
Chapter 4

The Finite Element Solution of the IBVP in Elasto-Plasticity

4.1 Introduction

In this chapter we report on the displacement finite element method for the solution of the initial boundary value problem of an elastoplastic model with internal variables. After reformulating the general problem in a way which presents the displacement field \( u = u(x, t) \in \mathcal{V} \) as the sole principal unknown, we continue by discussing the corresponding discrete schemes so that the nature of the ensuing discretization errors can be understood.

With this regard and the aim to set a general framework for handling evolving finite element meshes, we need to invert the usual sequence of first spatial and then temporal discretization by considering first the semidiscrete scheme in time, which is here obtained by a backward Euler integration in time. The initial boundary value problem, continuous in time and space, is transformed into the recursive solution of nonlinear problems, continuous with respect to the space, which are referred to as incremental boundary value problems, henceforth abbreviated as InBVP. In these problems, the state of the system at the time instant \( t_n \) is a data of the problem, whereas the principal unknown is the displacement field at the time instant \( t_{n+1} \).

The fully discrete scheme of the initial boundary value problem is therefore obtained by a finite element discretization of these incremental boundary value problems. This is obtained by replacing the general infinite dimensional affine spaces where the principal unknown and test functions belong to, with finite dimensional affine spaces. Here, we observe that change of data and/or of finite element mesh from one time interval to the other can be both related to a discontinuity jump of the approximate solution across the time instant \( t_n \). As a result of the observations expressed in Section 3.4, in the developments of reliable \textit{a posteriori} error estimators, one needs, therefore, to account also for the jump. With such \textit{a posteriori} error estimator at hand, indication on how to change the finite element space and define the corresponding data can be given and the assessment of the several transfer operations proposed in literature can be framed in the context of the ensuing error.
We, therefore, will give a brief review of some of the current techniques to transfer data from one mesh to the other, and recall the numerical techniques adopted for the solution of the nonlinear algebraic system of equations before concluding the chapter.

**4.2 The displacement formulation of the IBVP**

In section 3.2 the properties and equations that define the behaviour of a standard generalised material model have been given, which for the reader’s convenience have been summarized in Box 3.1. There, the constitutive equations were presented in the alternative scalar equivalent formulations for the purpose of the theory of the error in the constitutive equations. The latter are now given in a more general format, not necessarily restricted to the class of standard models, which is used as basis of the numerical discretization. The general formulation of the problem, therefore, reads as in Box 4.1 where $\mathfrak{F}$ and $\mathfrak{G}$ denote two multivalued tensorial functions which assure the thermodynamic admissibility of the model and enjoy the necessary regularity properties to guarantee a solution of the constitutive initial value problem for given total strain. Furthermore, as for the specific choice of the functional spaces, that is, of the functional setting in which the initial boundary value problem is posed, in the following we assume, if not stated otherwise, that the spaces are endowed with those minimum regularity properties that make the operations involved meaningful and guarantee at least the existence of a solution (Brezis, 1986). However, given the generality of the formulation in Box 4.1, in the current literature, there are no results on its well–posedness. These are available only for some special classes of material models and for some formulations of the initial boundary value problem, as it can be found, among others, in the works of Moreau (1974); Duvaut & Lions (1976); Johnson (1976a, 1978); Suquet (1981); Temam (1985, 1986); Han & Reddy (1999); Alberty & Carstensen (2000) and Fuchs & Seregin (2000).

**Remark 4.1.** We would like to point out the two main techniques which have been adopted for the proof of existence of solutions and which differentiate mainly in the first part of the proof. One makes a systematic use of Rothe’s method (Kacur, 1985) in which first approximate solutions are constructed by semidiscretization in time, and then one passes to the limit using compactness arguments. The other technique applies the methods of the constructive theory of partial differential equations in which a family of regularized problems is examined before passing to the limit using likewise compactness arguments (Evans, 1999). For both the techniques it is fundamental that uniform *a priori* estimates of the approximate solutions are available.
Box 4.1. General formulation of the Initial Boundary Value Problem for a model with internal variables

Given \( b(x, t) \) on \( \Omega \), \( t(x, t) \) on \( \partial \Omega \), and the initial state (here assumed as free)

Find \( \forall t \in [0, T] \)

\[
\begin{align*}
\mathbf{u} &= u(\mathbf{x}, t) \in \mathcal{V} \\
\mathbf{e}^p &= e^p(\mathbf{x}, t) \in \mathcal{E} \\
\mathbf{\alpha} &= \alpha(\mathbf{x}, t) \in \Lambda \\
\mathbf{\sigma} &= \sigma(\mathbf{x}, t) \in \mathcal{S} \\
\mathbf{A} &= A(\mathbf{x}, t) \in \mathcal{A}
\end{align*}
\]

such that the following equations are satisfied:

\[
\langle \mathbf{\sigma}(\mathbf{x}, t), \nabla \mathbf{n}(\mathbf{x}) \rangle = \langle \mathbf{b}(\mathbf{x}, t), \mathbf{n}(\mathbf{x}) \rangle + \langle \mathbf{t}(\mathbf{x}, t), \mathbf{n}(\mathbf{x}) \rangle_{\partial \Omega},
\]

\( \forall \mathbf{n} \in \mathcal{V}_0, \ \forall t \in [0, T], \)

and, \( \forall \mathbf{x} \in \Omega, \ \forall t \in [0, T], \)

\[
\mathbf{e}(\mathbf{x}, t) = \nabla_s u(\mathbf{x}, t)
\]

Constitutive Initial Value Problem (CIVP)

\[
\begin{align*}
\mathbf{e}(\mathbf{x}, t) &= \mathbf{e}^c(\mathbf{x}, t) + \mathbf{e}^p(\mathbf{x}, t) \\
\mathbf{\sigma}(\mathbf{x}, t) &= \frac{\partial \psi_e}{\partial \mathbf{e}}(\mathbf{e}^c(\mathbf{x}, t)) \\
\mathbf{A}(\mathbf{x}, t) &= \frac{\partial \psi_p}{\partial \mathbf{\alpha}}(\mathbf{\alpha}(\mathbf{x}, t)) \\
\partial_t \mathbf{e}^p(\mathbf{x}, t) &\in \mathcal{F}(\mathbf{\sigma}(\mathbf{x}, t), \mathbf{A}(\mathbf{x}, t)) \\
\partial_t \mathbf{\alpha}(\mathbf{x}, t) &\in \mathcal{G}(\mathbf{\sigma}(\mathbf{x}, t), \mathbf{A}(\mathbf{x}, t)) \\
\mathbf{e}^p(\mathbf{x}, t = 0) &= \mathbf{0} \\
\mathbf{\alpha}(\mathbf{x}, t = 0) &= \mathbf{0}
\end{align*}
\]
4.2.1 Statement of the problem

The displacement formulation of the initial boundary value problem defined in Box 4.1 is obtained further the observation that the constitutive initial value problem can be solved at any point \( x \in \Omega \) with respect to the stress tensor once the displacement field is given.

**Box 4.2.** Displacement formulation of the initial boundary value problem for a model with internal variables

Given \( b(x, t) \) on \( \Omega \), \( t(x, t) \) on \( \partial \Omega \) and the initial state (here assumed as free)

Find \( \forall t \in [0, T] \)

\[
\mathbf{u} = \mathbf{u}(\mathbf{x}, t) \in \mathcal{V}
\]

such that the following equation is satisfied:

\[
\langle \mathbf{\sigma}(x, t), \nabla \eta(x) \rangle = \langle b(x, t), \eta(x) \rangle + \langle t(x, t), \eta(x) \rangle_{\partial \Omega},
\]

\( \forall \eta \in \mathcal{V}_0, \ \forall t \in [0, T], \)

where \( \mathbf{\sigma} = \mathbf{\sigma}(x, t) \) is the stress tensor field obtained by solving at any point \( x \in \Omega \) the following constitutive initial value problem with prescribed strain \( \mathbf{e}(x, t) = \nabla\mathbf{u}(x, t), \ \forall t \in [0, T] \)

**Constitutive Initial Value Problem (CIVP)**

\[
\begin{align*}
\mathbf{e}(x, t) &= \mathbf{e}^e(x, t) + \mathbf{e}^p(x, t) \\
\mathbf{\sigma}(x, t) &= \frac{\partial \psi_e}{\partial \mathbf{e}^e} (\mathbf{e}^e(x, t)) \\
\mathbf{A}(x, t) &= \frac{\partial \psi_p}{\partial \mathbf{\alpha}} (\mathbf{\alpha}(x, t)) \\
\partial_t \mathbf{e}^p(x, t) &\in \mathfrak{F}(\mathbf{\sigma}(x, t), \mathbf{A}(x, t)) \\
\partial_t \mathbf{\alpha}(x, t) &\in \mathfrak{G}(\mathbf{\sigma}(x, t), \mathbf{A}(x, t)) \\
\mathbf{e}^p(x, t = 0) &= 0 \\
\mathbf{\alpha}(x, t = 0) &= 0
\end{align*}
\]

We assume that given the displacement field, \( \mathbf{u} = \mathbf{u}(x, t) \), the constitutive initial value problem given in Box 4.1 has solution.

By replacing then the stress tensor into the equilibrium equation, we obtain an equation with respect to only the displacement field. The displacement formulation of the initial boundary value problem reads, therefore, as in Box 4.2 (de Souza Neto et al., 2002).
Remark 4.2. The solution of the constitutive model with respect to the stress tensor in terms of the displacement field is implied, on a conceptual level and under general smoothness assumptions, by the equivalence of the two type of formulations of a constitutive model, the one in terms of the history of the mechanical variables and the other in terms of the internal variables (Bataille & Kestin, 1979; Ladevèze, 1999). Also, it is worth mentioning that the solution of the constitutive initial value problem (henceforth abbreviated as CIVP) with prescribed strain field \( \epsilon(x, t) = \nabla_s u(x, t) \) delivers the field of all the variables which describe the state of the system (Laborde & Nguyen, 1990).

Remark 4.3. A formal expression of the problem given in Box 4.2 in the case of perfect plasticity with the equation expressed only in terms of the displacement field can be found, for instance, in Rannacher & Suttmeier (1998). This formulation is, in fact, not different from the one given in Box 4.2 since the authors let \( \sigma = \Pi(\nabla u) \) where \( \Pi \) denotes a very general operator obtained by solving the CIVP with respect to \( u \).

4.3 The time discrete problem

The displacement formulation of the initial boundary value problem given in Box 4.2 is discretized in time by the backward Euler method. This is the time discrete scheme which will be the focus of our considerations in the following because of its stability and accuracy properties for finite time step (Ortiz & Popov, 1985).

Let \( 0 = t_1 < \ldots < t_n < \ldots < t_{N+1} = T \) be a partition of the time interval of interest \([0, T]\) and set \( k = \max_{1 \leq n \leq N} \{k_n = t_{n+1} - t_n\} \). A family of fully implicit approximations of the problem in Box 4.2 is obtained as recursive solution of the non linear spatially continuous variational problems defined in Box 4.4. These are obtained by replacing the rate quantities with backward difference quotients and by sampling all the other functions at \( t_{n+1} \). Thus, the constitutive initial value problem is transformed into the constitutive incremental nonlinear problem (henceforth, abbreviated as CInNP), and the resulting global problem appears, therefore, in the form of a system of variational equations, which expresses the equilibrium, and unilateral constraints in the presence of the inclusions which define the stress tensor field. This format is much clearer if, for instance, we refer to the model of linear elasticity and associative plasticity with linear hardening (Simo & Hughes, 1998; Rannacher & Suttmeier, 1998; Han & Reddy, 1999), which is given in Box 4.3. Here, the CInNP is obtained by backward Euler discretization of the principle of maximum plastic dissipation (3.17).

The effect of replacing the time derivative with backward difference quotients produces an error which is referred to as the time discretization error. The main effect of this error is visualized in Figure 4.1 for a model of perfect plasticity with regard to the error on the direction of the plastic flow under the assumption that the point \( x \) experiences plastic loading passing from \( t_n \) to \( t_{n+1} \). However, also an error in the intensity of the plastic flow must be noted, in general. The magnitude
Box 4.3. Fully implicit scheme of the IBVP for the model of linear elasticity and associative plasticity with linear hardening

For: \( n = 1, 2, \ldots, N \)

Given: External Loading

\[
b_{n+1}(x) = b(\bullet, t_{n+1}), \quad \text{on } \Omega
\]

\[
t_{n+1}(x) = t(\bullet, t_{n+1}), \quad \text{on } \partial \Omega_t
\]

State of the system at \( t_n \)

\[
\mathbf{e}_n(x) = \mathbf{e}^p(\bullet, t_n) \in \mathcal{E}
\]

\[
\mathbf{\alpha}_n(x) = \mathbf{\alpha}(\bullet, t_n) \in \Lambda
\]

Find: \( u_{n+1}(x) = u(\bullet, t_{n+1}) \in \mathcal{V} \)

Such That the following equation is satisfied:

\[
\langle \mathbf{\sigma}_{n+1}(x), \nabla \mathbf{\eta}(x) \rangle = \langle b_{n+1}(x), \mathbf{\eta}(x) \rangle + \langle t_{n+1}(x), \mathbf{\eta}(x) \rangle_{\partial \Omega_t}
\]

\[\forall \mathbf{\eta} \in \mathcal{V}_0.\]

with \( \mathbf{\sigma}_{n+1}(x) = \mathbf{\sigma}(x, t_{n+1}) \in \Sigma_{n+1}(x) \), where \( \Sigma_{n+1}(x) \) is obtained by solving at any point \( x \in \Omega \) the following variational inequality defined by the prescribed strain \( \mathbf{e}(x, t_{n+1}) = \nabla_s u_{n+1}(x) \),

\[
\begin{align*}
\langle \mathbf{\sigma}_{n+1}^{\text{trial}}(x) - \mathbf{\sigma}_{n+1}(x) \rangle : & C^{-1}(\mathbf{\tau}(x) - \mathbf{\sigma}_{n+1}(x)) + \\
+ & (\mathbf{H}\mathbf{\alpha}_n(x) - \mathbf{A}_{n+1}(x)) : H^{-1}(\mathbf{B}(x) - \mathbf{A}_{n+1}(x)) \leq 0 \\
\forall (\mathbf{\tau}, \mathbf{B}) \in \mathcal{E}
\end{align*}
\]

where

\[
\mathbf{\sigma}_{n+1}^{\text{trial}}(x) = C(\nabla_s u_{n+1}(x) - \mathbf{e}_n^p(x))
\]

of this error depends on the accuracy for finite time step of the time integration scheme. This accuracy is, usually, assessed numerically with the so-called isoerror maps (Krieg & Krieg, 1977; Schreyer et al., 1979; Ortiz & Popov, 1985).

In proposing the above time discrete scheme, we are faced with two questions: one refers to the existence and uniqueness of the solution for the single nonlinear variational problem while the second regards the convergence of the family of the approximations with their respective rate. Unfortunately, for the formulation given in Box 4.4, likewise the continuous formulation, there are no general results of well posedness and, a fortiori, of convergence of the approximation. However, some considerations can be done in merit, though of heuristic character, as extension of those holding for formulations for which a complete analysis has been provided by Han & Reddy (1999).

In stating the single one step variational problem relative to \([t_n, t_{n+1}]\), the state of the system at the time instant \( t_n \) defined by \( \mathbf{e}_n^p(x) \), \( \mathbf{\alpha}_n(x) \) is a data of the
problem whereas the unknown is given by the function $u_{n+1}(x)$. Therefore, we assume that the single nonlinear variational problem has solution, as long as very general regularity properties are met by the data. This can be easily shown, for example, for the time discrete scheme of the dual variational formulation for the model of linear elasticity and associative plasticity with linear hardening, given in Han & Reddy (1999), where $(u, \sigma, A)$ are assumed as primary variables and the change of data corresponds to the projection onto a closed convex set at a different point.

Also, we require that $\epsilon^p_n(x), \alpha_n(x)$ are obtained from the pointwise solution of the CInNP relative to the previous time interval $[t_{n-1}, t_n]$ and to the solution $u_n(x)$. This condition, which can be expressed as continuity of the piecewise linear interpolant of the discrete solutions $\{\epsilon^p_n(x)\}_{n=1}^{N+1}, \{\alpha_n(x)\}_{n=1}^{N+1}$, is invoked in order to obtain an a priori estimate for the family of solutions which is independent on $k$. This is a basic result which is used for a compactness argument to prove finally that as $k \to 0$, the limit of the interpolants is in fact a solution of the continuous problem. Part of the analysis of the semidiscrete scheme would be also an a priori error estimate of the approximate solutions which would have in general the following format

$$\max_{1 \leq n \leq N} \| u_{ex}(x, t_{n+1}) - u_{n+1}(x) \| \leq O(k^q), \quad q > 0,$$

which describes the time discretization error with its dependence on the discretization parameter $k$ along with its rate of convergence and $\| \cdot \|$ is an appropriate norm.

**Remark 4.4.** In relation to the particular constitutive model, the solution of CInNP can result in more or less complex algorithm. The procedures for its solution are generally called constitutive update algorithms. In rate independent plasticity models, in particular, they are also referred to as the stress return algorithms, because the stresses must be returned to the yield surface (Ortiz & Stainier, 1999). For an overview of these procedures, we refer to Simo (1998); Armero & Pérez-Foguet.
Box 4.4. Fully implicit scheme of the problem given in Box 4.2

For: \( n = 1, 2, \ldots, N \)

Given: External Loading

\[ b_{n+1}(x) = b(\bullet, t_{n+1}), \text{ on } \Omega \]
\[ t_{n+1}(x) = t(\bullet, t_{n+1}), \text{ on } \partial \Omega \]

State of the system at \( t_n \)

\[ e_p^n(x) = e_p^n(\bullet, t_n) \in \mathcal{E} \]
\[ \alpha_n(x) = \alpha(\bullet, t_n) \in \Lambda \]

Find: \[ u_{n+1}(x) = u(\bullet, t_{n+1}) \in \mathcal{V} \]

Such That the following equation is satisfied:

\[ \langle \sigma_{n+1}(x), \nabla \eta(x) \rangle = \langle b_{n+1}(x), \eta(x) \rangle + \langle t_{n+1}(x), \eta(x) \rangle_{\partial \Omega}, \]
\[ \forall \eta \in \mathcal{V}_0, \]

where \( \sigma_{n+1}(x) = \sigma(x, t_{n+1}) \) is the stress tensor field obtained by solving, at any point \( x \in \Omega \), the following constitutive nonlinear problem with prescribed strain \( \epsilon(x, t_{n+1}) = \nabla u_{n+1}(x) \),

Constitutive Incremental Nonlinear Problem (CInNP)

\[
\begin{align*}
\epsilon(x, t_{n+1}) &= \epsilon^e(x, t_{n+1}) + \epsilon^p(x, t_{n+1}) \\
\sigma(x, t_{n+1}) &= \frac{\partial \psi_e}{\partial \epsilon_e}(\epsilon^e(x, t_{n+1})) \\
A(x, t_{n+1}) &= \frac{\partial \psi_p}{\partial \alpha}(\alpha(x, t_{n+1})) \\
\frac{\epsilon^p(x, t_{n+1}) - \epsilon^p(x, t_n)}{k_n} &\in \mathfrak{F}(\sigma(x, t_{n+1}), A(x, t_{n+1})) \\
\frac{\alpha(x, t_{n+1}) - \alpha(x, t_n)}{k_n} &\in \mathfrak{G}(\sigma(x, t_{n+1}), A(x, t_{n+1}))
\end{align*}
\]

(2002) and de Souza Neto et al. (2002).

We conclude this section by observing first that the nonlinear variational problem of Box 4.4 can be given the following compact form of a nonlinear variational
Find: \( u_{n+1}(x) \in V \)

Such that:

\[
L(\nabla_s u_{n+1}, \eta) \overset{\text{def}}{=} \langle \sigma_{n+1}(\nabla_s u_{n+1}(x)), \nabla \eta(x) \rangle - \langle b_{n+1}(x), \eta(x) \rangle - \langle t_{n+1}(x), \eta(x) \rangle_{\partial \Omega} = 0
\]

\[
\forall \eta \in V_0,
\]

where the stress tensor

\[
\sigma_{n+1}\left(\epsilon^p_n(x), \alpha_n(x); \epsilon_{n+1}(x) = \nabla_s u_{n+1}(x)\right)
\]

is obtained as part of the solution of the CInNP with data \( \epsilon^p_n(x) \) and \( \alpha_n(x) \). It is understood that, in (4.3), \( \epsilon_{n+1}(x) \) must be considered as variable. Secondly, the function \( L = L(\nabla_s u_{n+1}, \eta) \) is defined over \( V \times V_0 \), where we recall \( V \) to be the space of the kinematically admissible displacement fields \( u_{n+1} = u_{n+1}(x) \), principal unknown of the problem. In the time discrete problem, the displacement field \( u_{n+1} = u_{n+1}(x) \) may be any element of \( V \).

### 4.4 The fully discrete problem: Constant finite element mesh

The single one step problem (4.2) arising from the time discretization of the initial boundary value problem has the same structure as an elliptic problem apart from substitution into the equilibrium equation of the stress tensor with a nonlinear function of the displacement, \( u_{n+1} = u_{n+1}(x) \), primary unknown of the problem. The latter is therefore amenable to the discretization methods for this class of problems (Glowinski et al., 1981) in particular the finite element method. In this way the complete discretization of the initial boundary value problem is achieved, which is referred to as the fully discrete scheme.

More precisely, approximations to the single nonlinear incremental boundary value problem (4.2) with standard displacement finite element procedure are obtained by simply replacing the infinite dimensional affine spaces \( V \) and \( V_0 \) of the trial functions, \( u_{n+1} = u_{n+1}(x) \), and test functions, \( \eta = \eta(x) \), with finite dimensional affine subspaces, \( V^h \) and \( V_0^h \), respectively, which are intended to be finite element spaces. For the construction of such spaces with respective terminology, we refer to standard textbooks on finite element methods (Ciarlet, 1978; Zienkiewicz & Taylor, 2000).

The weak enforcement of the equilibrium with respect to only some test functions produces an error which is referred to as the space discretization error. As it
has been already noted for linear problems in Section 2.2, the main effect of this error is the lack of the pointwise equilibrium over the domain $\Omega$, along the boundary traction $\partial \Omega_t$ and the element boundaries. The latter, in turn, means lack of continuity of the stress tensor field therein.

The discrete problem relative to the time interval $[t_n, t_{n+1}]$ reads as follows

| Given: | External Loading $b_{n+1}(x)$, on $\Omega$
|        | $t_{n+1}(x)$, on $\partial \Omega_t$
| State of the system at $t_n$ | $\epsilon_p^n(x) \in \mathcal{E}$
|        | $\alpha_n(x) \in \Lambda$
| Find: | $u_{n+1}^h(x) \in \mathcal{V}^h$
| Such That | $\langle h\sigma_{n+1}^h(x), \nabla \eta^h(x) \rangle = \langle b_{n+1}(x), \eta^h(x) \rangle + \langle t_{n+1}(x), \eta^h(x) \rangle_{\partial \Omega_t}$
|        | $\forall \eta^h \in \mathcal{V}^h_0$ | (4.4)

where the stress tensor field $h\sigma_{n+1}^h(x)^\dagger$ is the function defined by solving at any point $x \in \Omega$ the CInNP for any given strain $\epsilon_{n+1}^h(x) = \nabla_s u_{n+1}^h(x)$ and fixed data $\epsilon_p^n(x), \alpha_n(x)$.

Since a complete theory for the fully discrete approximations to the displacement formulation of the initial boundary value problem is still to be developed, the considerations that follow will have, therefore, heuristic character and will assume minimum regularity requirements. In rate independent plasticity with positive hardening, this leads, in our case, to the assumption that $\epsilon_{p}(x,t), \alpha(x,t)$ are continuous in time and in space, whereas $\dot{\epsilon}_{p}(x,t), \dot{\alpha}(x,t)$ can experience discontinuity in time.

**Remark 4.5.** The following observations take their motivation primarily from the analysis of fully discrete approximations to other formulations of plasticity, basically the dual variational formulation of the model of linear elasticity and associative plasticity with linear hardening (Johnson, 1976b, 1977; Hlaváček, 1980; Han & Reddy, 1999) and of other evolutive processes, in general, (Kacur, 1985; Evans, 1999) and in particular, parabolic equations (Raviart & Thomas, 1983), and degenerate parabolic equations (Nochetto et al., 1997; Chen et al., 2000a). Instrumental is also the work by Dorfler & Wilderotter (2000), though for elliptic problems, on the development of *a posteriori* error estimates which account for data error.

Likewise the Galerkin finite element approximation of the primal formulation of an elliptic problem, we can, therefore, assume that the results of well posedness

---

\dagger Hereafter, the symbol $h(\bullet)$ will refer to secondary variables which are obtained from the solution of the CInNP for the prescribed strain $\epsilon^n_{n+1}(x)$. They do not have to be confused with the respective finite dimensional discretizations, which are here not considered. Only the interpolation of the primary variable, that is, the displacement field, has been assumed which is remarked with the superscript $h$ on the right.
for the continuous problem (4.2) carry over also to its finite element counterparts represented by problem (4.4); in particular, we emphasize the existence of solution as long as the data $e_n(x), \alpha_n(x)$ satisfy general regularity properties.

The result of the approximation which involves only the unknown function $u_{n+1}(x)$ is an error whose value depends on the approximation properties of the finite element subspaces. This is usually mirrored by an a priori error estimate of the discretization error which is of the following type

$$||u_{n+1}(x) - u_{n+1}^h(x)|| \leq O(h^p)$$

where $h$ denotes the mesh size of the triangulation $T_h$ of the domain $\Omega$ associated with the finite element space $V_h$, and $u_{n+1}(x)$ is the solution of the continuous problem (4.2). If we combine the above result with (4.1) and use the triangular inequality, finally, we can obtain

$$\max_{1 \leq n \leq N} ||u_{ex}(x, t_{n+1}) - u_{n+1}^h(x)|| \leq O(k^q) + O(h^p)$$ (4.5)

which shows the effects of time and space discretization on the full discrete approximation along with their respective order of convergence.

In posing problem (4.4), however, the data $e_n(x), \alpha_n(x)$ are unknowns, for being solution at $t_n$ of the time discrete scheme. Therefore, a fully discrete approximation to the problem (4.2), actually, calls also for an approximation to the data of each nonlinear incremental boundary value problem.

If the finite element space $V^h$ does not change from one time interval to the other, a fully discrete scheme is formulated by assuming the data $e_n^h(x), \alpha_n^h(x)$, which denote the pointwise solution at $t_n$ of the CInNP with $e_{n-1}^h(x), \alpha_{n-1}^h(x)$ and prescribed strain $e_n^h(x) = \nabla_s u_n^h(x)$ corresponding to the finite element solution at $t_n$. As a result of this choice, the piecewise linear interpolant of the discrete solutions $\{e_n^h(x)\}_{n=1}^{N+1}, \{\alpha_n(x)\}_{n=1}^{N+1}$ is continuous. By exploiting then properties of the equations, one can, therefore, envisage to get a uniform a priori estimate for the family of solutions which proves to be crucial for the error analysis of the fully discrete scheme as it results from the convergence studies carried out in Johnson (1976b, 1977); Hlaváček (1980) and Han & Reddy (1999) which finally deliver an error estimate in the form of relation (4.5).

The above considerations suggest that also other approximations to the data of (4.2) can be imagined, provided that then it is possible to prove the convergence of the resulting fully discrete problems. However, the effects of different approximations to the data from the one proposed above and of change of finite element space from one time step to the other, necessary in the adaptive finite element solution of the given initial boundary value problem, are similar and related somehow to each other. Both can be related, in fact, to the introduction of a discontinuity in the solution as it will be shown in the next Section.
4.4.1 Change of finite element mesh

In this Section we consider change of finite element mesh from one time interval to the other and assume that we are able to solve the CInNP at any point $x \in \Omega$. More precisely, let $\mathcal{V}^h_n$ and $\mathcal{V}^{h_{n+1}}$ be the finite element spaces adopted for the discretization of the problem (4.2) relative to the time interval $[t_{n-1}, t_n]$ and $[t_n, t_{n+1}]$, respectively. Set $h = \max_{1 \leq n \leq N} h_n$ where $h_n$ is the meshsize of the triangulation $T_{h_n}$ of the domain $\Omega$ associated with the finite element space $\mathcal{V}^h_n$ and denote with $h_n \epsilon_n(x)$, $h_n \alpha_n(x)$, $h_n \sigma_n(x)$, the pointwise solution of the CInNP corresponding to the finite element solution $u_{h_n}(x)$ at $t_n$.

Assume $h_n \epsilon_n(x)$, $h_n \alpha_n(x)$ as data of the problem (4.2) relative to $[t_n, t_{n+1}]$ and the finite element space $\mathcal{V}^{h_{n+1}}$ for its discretization.

If we envision to solve first for $\Delta t \to 0$, that is, at $t_n + \Delta t$ for $\Delta t$ very small (henceforth, this time instant will be denoted as $t_n^+$), one expects in general

$$h_n \epsilon_n(x) \neq h_{n+1} \epsilon_n(x), \quad h_n \alpha_n(x) \neq h_{n+1} \alpha_n(x), \quad h_n \sigma_n(x) \neq h_{n+1} \sigma_n(x),$$

where

$$h_{n+1} \epsilon_n(x), h_{n+1} \alpha_n(x), h_{n+1} \sigma_n(x)$$

are obtained as usual from the solution of the CInNP with data $h_n \epsilon_n(x)$, $h_n \alpha_n(x)$, and prescribed strain $\epsilon^{h_{n+1}}(x) = \nabla_h u_{h_{n+1}}(x)$, where $u_{h_{n+1}}(x) \in \mathcal{V}^{h_{n+1}}$ is the finite element solution at $t_n^+$. Indeed, $h_{n+1} \sigma_n(x)$ is in equilibrium with respect to $\mathcal{V}^{h_{n+1}}$, whereas $h_n \sigma_n(x)$ is not if $\mathcal{V}^{h_{n+1}} \neq \mathcal{V}^h$, even though they correspond to the same external load level at $t_n$. Figure 4.2 sketches the different quantities in a finite element solution with change of finite element mesh $T_{h_n} \to T_{h_{n+1}}$ at the time instant $t_n$.

![Figure 4.2: External loads and Finite element solutions at $t_n$ and $t_{n+1}$ with equilibration of the initial state for change of finite element mesh $T_{h_n} \to T_{h_{n+1}}$ at the time instant $t_n$.](image)

The solution at $t_n^+$, sometimes, is said to be obtained further to the equilibration of the initial state (Cirak & Ramm, 2000).
Remark 4.6. If the finite element space does not change, i.e. $\mathcal{V}^{n} = \mathcal{V}^{n+1}$, then it is

\[ h_{n} \epsilon_{n}^{p}(x) = h_{n+1} \epsilon_{n+1}^{p}(x), \quad h_{n} \alpha_{n}(x) = h_{n+1} \alpha_{n}(x), \quad h_{n} \sigma_{n}(x) = h_{n+1} \sigma_{n}(x), \]

that is, the interpolant will be continuous at $t_{n}$.

Consider now also the solution at $t_{n+1}$ obtained with data $h_{n+1} \epsilon_{n}^{p}(x)$, $h_{n+1} \alpha_{n}(x)$ and corresponding to the finite element solution $u^{n+1}(x) \in \mathcal{V}^{n+1}$ of (4.4). It is easy to realize that the piecewise linear interpolant of the discrete solutions

\[ \ldots, h_{n} \epsilon_{n}(x), h_{n+1} \epsilon_{n}^{p}(x), h_{n+1} \epsilon_{n+1}^{p}(x), \ldots \] 

\[ \ldots, h_{n} \alpha_{n}(x), h_{n+1} \alpha_{n}(x), h_{n+1} \alpha_{n+1}(x), \ldots \]

will be discontinuous across the time node $t_{n}$ as is depicted in Figure 4.3 at a generic point $x \in \Omega$. As a result, the considerations of Section 4.4 holding for static finite element meshes can no more be extended and, consequently, nothing can be said, in general, on the convergence of the interpolants as $k, h \to 0$. In this case, in fact, by accounting for the observations in Section 3.4, the residual error, which is obtained by substituting the approximation into the general equations given in Box 4.2 and representing the forcing in the problem that defines the global error, has two components: One component is regular, which is present also with static finite element mesh and depends essentially on the time step and mesh size. The other component is singular for the presence of the rate quantities $\dot{\epsilon}_{p}(x, t)$, $\dot{\alpha}(x, t)$ and the discontinuity jump in the time interpolant of $\epsilon_{p}$ and $\alpha$. The singular components, therefore, depend on the value of the discontinuities which, for the way the fully discrete schemes have been formulated in this section, can be arbitrary. Consequently, in principle, they can have an important influence on the global error. It is, therefore, clear that reliable a posteriori estimates of the error of such approximations will have to depend also on these singular components of the residual.

Figure 4.3: The time interpolant of the plastic strain at a generic point $x \in \Omega$ is discontinuous at $t_{n}$ further to the change of finite element mesh $T_{h_{n}} \to T_{h_{n+1}}$. 
Remark 4.7. Convergence studies of fully discrete approximations in presence of changing mesh are completely missing from the current literature, even for those variational formulations of plasticity models which have been investigated in detail in the case of finite element mesh constant in time. Furthermore, it must be noted that the issue of convergence of fully discrete approximations in presence of evolving finite element meshes is shared also by other evolutive processes, in general, as it has been pointed out since by Dupont (1982). This problem, actually, calls for constraints on the type of change of mesh as shown in Nochetto et al. (1997) and Dawson & Kirby (1999) in order to deliver family of discrete schemes that are stable and convergent. Also, it is interesting to note in the a posteriori error estimates of these approximations the presence of terms that are due to change of mesh (Eriksson & Johnson, 1991; Estep et al., 2000; Chen et al., 2000a).

In conclusion, if no change of mesh occurs, $k$ and $h$ are the only parameters that control the accuracy of the approximation, thus, both a priori and a posteriori error estimates will depend somehow on them. On the contrary, if change of mesh occurs, we introduce a discontinuity in the solution which can be arbitrary. The residual will have singular components depending on the jump. Since the error depends on the residual, its estimates, both a priori and a posteriori, will have to account for the discontinuity.

Also, the considerations expressed in this section point out an important issue in presence of change of mesh. This is the development of a posteriori error estimates which by including terms that depend on the jump allow one to assess the effects of change of mesh and data for the solution of (4.4). We will see in Chapter 6 how the error in the constitutive equations will account for the discontinuity in a natural and consistent way.

Remark 4.8. A similar outcome, though starting from a different point of view, can be retrieved from the analysis carried out in Rannacher & Suttmeier (1999), which has been highlighted in section 2.3.2. The authors refer to a dual variational formulation of plasticity and propose a splitting of the error which distinguishes the component due to time discretization, to the space discretization and to the effect, through the stability of the nonlinear incremental boundary value problem, of the error for using different data in posing this problem.

4.5 Overview on the different definitions of the initial state. Transfer procedures

In the previous section, we have assumed the data $h^ne^p_n(x)$, $h^n\alpha_n(x)$ to be known at any point $x \in \Omega$ and equal to the solution at $t_n$ relative to the previous time interval $[t_{n-1}, t_n]$. However, in the actual computation, we do not need in general to know such fields but only their value at discrete points, namely the integration points of each element. In fact, the integrals that appear in the variational formulation (4.4) are seldom computed exactly. Instead, they are approximated through a process of numerical integration, such as, for instance, Gaussian quadrature formula.
Let $\Omega_{l}^{h_{n+1}} \in \mathcal{T}_{h_{n+1}}$ be a generic element of the triangulation $\mathcal{T}_{h_{n+1}}$ and denote with $x_{l,j}^{h_{n+1}} \in \Omega_{l}^{h_{n+1}}$ the $j^{th}$ Gauss point of the element $\Omega_{l}^{h_{n+1}}$ and $\text{ngp}$ their number. Problem (4.4), formulated next with generic data $\tilde{e}_{n}^{p}(x), \tilde{\alpha}_{n}(x)$, reads as

\begin{align*}
\text{Given:} \quad & \forall \Omega_{l}^{h_{n+1}} \in \mathcal{T}_{h_{n+1}} \text{ and for } j = 1, \ldots, \text{ngp}, \quad \text{External Loading} \\
& b_{n+1}(x_{l,j}^{h_{n+1}}) \\
& \tilde{e}_{n}^{p}(x_{l,j}^{h_{n+1}}) \\
& \tilde{\alpha}_{n}(x_{l,j}^{h_{n+1}}) \\
\text{State of the system at } t_{n} \quad & \text{Find:} \quad u_{n+1}^{h}(x) \in \mathcal{V}_{n+1}^{h} \\
\text{Such that:} \quad & \sum_{\Omega_{l}^{h_{n+1}} \in \mathcal{T}_{h_{n+1}}} \sum_{j=1}^{\text{ngp}} j_{i,j} w_{i,j} \left\{ h_{n+1} \sigma_{n+1}(\nabla_{s} u_{n+1}^{h}(x_{l,j}^{h_{n+1}})) : \nabla \eta_{n+1}^{h}(x_{l,j}^{h_{n+1}}) + b_{n+1}(x_{l,j}^{h_{n+1}}) : \eta_{n+1}^{h}(x_{l,j}^{h_{n+1}}) \right\} = 0, \quad \forall \eta_{n+1}^{h} \in \mathcal{V}_{0}^{h}, \quad (4.6)
\end{align*}

where the stress tensor $\sigma_{n+1}(h_{n+1}(x_{l,j}^{h_{n+1}}) = \nabla_{s} u_{n+1}^{h}(x_{l,j}^{h_{n+1}}))$ is obtained by solving at $x_{l,j}^{h_{n+1}}$ the CInNP with data $\tilde{e}_{n}^{p}(x_{l,j}^{h_{n+1}}), \tilde{\alpha}_{n}(x_{l,j}^{h_{n+1}})$ and prescribed strain $\epsilon_{n+1}(x_{l,j}^{h_{n+1}}) = \nabla_{s} u_{n+1}^{h}(x_{l,j}^{h_{n+1}})$. The symbols $w_{i,j}, j_{i,j}$ denote the weight and the value at the Gauss points of the Jacobian of the transformation of the master element onto the current element in the Gaussian quadrature scheme (Ciarlet, 1978; Zienkiewicz & Taylor, 2000; de Souza Neto et al., 2002). For the sake of notation, the work of the traction forces in (4.6) has been dropped.

The choice of the quantities in rate form as secondary variables, for which no a priori interpolation assumptions have been made, and the use of backward Euler as time discrete scheme poses, however, the question on how to define the data $\tilde{e}_{n}^{p}(x_{l,j}^{h_{n+1}})$ and $\tilde{\alpha}_{n}(x_{l,j}^{h_{n+1}})$ for the InBVP in the case the finite element mesh adopted for its discretization is different from the one used in the previous time interval. In this case, inasmuch as the mesh changes across the time node $t_{n}$, the Gauss points change as well and it is no more possible to define the history of the secondary variables at these points if they have not been considered from the initial time $t = 0$.

The procedures currently in use for the definition of these data, in general, try to compute the value of the unknowns fields $\tilde{e}_{n}^{p}(x_{l,j}^{h_{n+1}}), \tilde{\alpha}_{n}(x_{l,j}^{h_{n+1}})$ at the new integration points, $x_{l,j}^{h_{n+1}} \in \Omega_{l}^{h_{n+1}}$ with $\Omega_{l}^{h_{n+1}} \in \mathcal{T}_{h_{n+1}}$, in terms of the values $\epsilon_{n}^{h}(x_{e,j}^{h}), \alpha(x_{e,j}^{h})$, solution relative to the previous time interval $[t_{n-1}, t_{n}]$ at the old integration points, $x_{e,j}^{h} \in \Omega_{e}^{h}$ with $\Omega_{e}^{h} \in \mathcal{T}_{h_{n}}$.

These procedures are usually known by the name of Transfer of Data and represent a very delicate issue for the global accuracy of a finite element adaptive
solution. With this regard, in fact, several vaguely defined properties are usually invoked for a mapping scheme in order to prevent corrupting the quality of the resulting finite element solution. These are, for instance, listed in Perić et al. (1996) and Rashid (2002), among others, and are referred to as self-consistency, locality, consistency with the constitutive equations, equilibrium, compatibility of the state transfer with the displacement field on the new mesh, minimisation of the numerical diffusion of these variables, just to mention a few. The remapping schemes proposed in literature attempt somehow to meet these properties and are different from each other according to the main aspect they address. As a result, it appears difficult trying to draw a classification. However, the fundamental approaches and ideas can be referred by some means to the following procedures:

- Variationally Consistent Transfer
- Weak Enforcement Continuity Transfer
- Smoothing Transfer

These transfer processes are in the following described succinctly with reference to the specific problem at hand. This means that only transfer of $\epsilon^p$ and $\alpha$ will be analysed. It is also worth noting that all the following operations share the same underlying idea of defining first a field for the state variables which depends on the old mesh with its relative distribution of the elemental Gauss points. This field is then transformed, according to the specific procedure, into a new field on the new mesh which allows the sampling at the new Gauss points. In Chapter 6, in comparing the effects of the above transfer procedures, we will consider the state variables at the new Gauss points after equilibration of the initial state and along with an assumption of prolongation over each element. The resulting function is denoted by $h^{n+1}(\bullet)_n$. The difference between the two fields, $h_n(\bullet)_n - h^{n+1}(\bullet)_n$, defines the discontinuity. Finally, it must be observed that it does not have to be of concern that the incremental form of the constitutive equations may be satisfied only at the Gauss points whereas it may be violated in other points of the domain $\Omega$ because of the prolongation operation. The error in the constitutive equations will quantify this discrepancy and in this sense, it must be considered as the error associated with the given assumption for the variables distribution.

### 4.5.1 Variationally consistent transfer

We define variationally consistent transfers as those remapping procedures where the initial data at $t_n$ for the solution of the nonlinear incremental boundary value problem relative to the time step $[t_n, t_{n+1}]$ is obtained from sampling at new Gauss points the solution of the variational formulation of the nonlinear incremental boundary value problem relative to the time step $[t_{n-1}, t_n]$. For this to happen, the equations that define the secondary variables, and appearing as data of the problem, must be expressed in a variational form and consequently an interpolation for those variables
must be prescribed. It is this variational formulation that provides the data for the fully discrete problem in case of change of mesh. With the aim of shedding light on this class of transfers, which have been analysed by Ortiz and coworkers in Ortiz & Quigley (1991); Camacho & Ortiz (1997) and Radovitzky & Ortiz (1999), it appears appropriate first to consider for motivation the simpler model problem of heat conduction and discuss the several definitions of data in posing the fully discrete scheme obtained by a backward Euler integration in time and finite element interpolation in space. This analysis will serve as a motivation to introduce the so-called variational consistent transfers for the problem at hand.

### 4.5.1.1 Heat Conduction

We consider the classical heat conduction problem in an isotropic body \( \Omega \subset \mathbb{R}^d \) with heat capacity \( \lambda = 1 \) and conductivity \( \mu = 1 \) described by the following equations

\[
\text{Find } u = u(x, t) \text{ with } x \in \Omega, \ t \in I = [0, T] \\
\frac{\partial u}{\partial t} - \Delta u = f \quad \text{in } \Omega \times I \\
u(x, t) = 0 \quad \text{in } \partial \Omega \times I \quad \text{(boundary condition)}
\]

\[
u(x, t = 0) = u_0 \quad \text{in } \Omega \text{ at } t = 0 \quad \text{(initial condition)}
\]

In (4.7), \( u(x, t) \) is the temperature at \( x \in \Omega \) at time \( t \in I \), \( u_0 \) is a given initial temperature and \( f \) is a given heat production. The symbol \( \Delta \) denotes the Laplacian operator.

In order to define the functional setting in which the weak form of (4.7) is posed, we need first to introduce the following notation. Let \( H^1_0(\Omega) \) denote the Sobolev space of functions of \( L^2(\Omega) \) with the first derivatives in the sense of distributions belonging to \( L^2(\Omega) \) and trace vanishing on \( \partial \Omega \) (Raviart & Thomas, 1983) and with \((\cdot, \cdot)\) the inner product in \( L^2(\Omega) \). Also, denote with \( C^0(I; V) \) and \( L^2(I; V) \) the space of the vector valued functions defined over the time interval \( I \) and with values in the space of functions \( V \) which are continuous and square integrable over \( I \), respectively.

By assuming \( u_0 \in H^1_0(\Omega) \) and \( f \in L^2(I; L^2(\Omega)) \), the weak form of (4.7) reads as follows (Raviart & Thomas, 1983)

\[
\text{Data } u_0 \in H^1_0(\Omega), \ f \in L^2(I; L^2(\Omega)) \\
\text{Find } u \in L^2(I; H^1_0(\Omega)) \cap C^0(I; H^1_0(\Omega)) \\
\frac{d}{dt}(u(t), v) + a(u(t), v) = (f(t), v) \quad \forall \ v \in H^1_0(\Omega),
\]

\[
u(0) = u_0
\]

where the time derivative is meant in the sense of distributions over \( I \) and the
bilinear form \( a(u(t), v) \) is defined as follows

\[
a(u(t), v) = \sum_{i=1}^{d} \left( \frac{\partial u}{\partial x_i}, \frac{\partial v}{\partial x_i} \right).
\]

In (4.8) we have identified the real function \( u(x, t) \) defined over \( \Omega \times I \) with the vector valued function \( u(t) \) defined over \( I \) and values in the space of functions of \( \Omega \) into \( \mathbb{R} \) (Raviart & Thomas, 1983).

We consider fully discrete schemes for the problem (4.8) obtained by a backward Euler discretization in time and a Galerkin approximation in space. We refer, therefore, to the same notation as introduced in Section 4.3 and 4.4.1; that is, \( I_{n+1} = [t_n, t_{n+1}] \) denotes the generic time interval of the partition of the time interval \( I \) of interest, and \( V^{h_{n+1}} \) is the conforming finite element space associated with the mesh \( T_{h_{n+1}} \) for the discretization of the one single step problem relative to \( I_{n+1} \).

The fully discrete scheme analysed in Dupont (1982) is

\[
\text{For } n = 1, \ldots, N
\]

\[
\text{Data: } u_{n}^{h_{n}} \in V^{h_{n}}
\]

\[
\text{Find: } u_{n+1}^{h_{n+1}} \in V^{h_{n+1}}
\]

\[
\left\|rac{1}{k_{n}} (u_{n+1}^{h_{n+1}}, v_{n+1}^{h_{n+1}}) + a(u_{n+1}^{h_{n+1}}, v_{n+1}^{h_{n+1}}) = (f_{n+1}, v_{n+1}^{h_{n+1}}) + \frac{1}{k_{n}} (u_{n}^{h_{n}}, v_{n+1}^{h_{n+1}})
\right|_{\forall v_{n+1}^{h_{n+1}} \in V^{h_{n+1}}}
\]

where for \( n = 1, \) \( u_{1}^{h_{1}} \) denotes any element of the space \( H_{0}^{1}(\Omega) \), whose choice is relevant for the error on the initial state \( u_{0} \).

**Remark 4.9.** Problem (4.9) is the finite element formulation of an elliptic equation of the form \((I - k_{n} \Delta)u = k_{n} f + u_{n}\) (Thomée, 1997).

In Dupont (1982) an asymptotic a priori error estimate is obtained under fairly general assumptions on the change of mesh. These are: Each mesh \( T_{n+1} \) is a refinement of some given coarse partition \( T \) of \( \Omega \). Moreover, \( T_{n+1} \) is obtained by at most one level of refinement or coarsening of the mesh \( T_{n} \).

An improvement of the rate of convergence, and with similar constraints on the change of mesh, has been obtained by Eriksson & Johnson (1991) who consider, however, the discontinuous Galerkin discretization in time with order zero, which reduces to the backward Euler.

We note that, in (4.9), we must compute \((u_{n}^{h_{n}}, v_{n+1}^{h_{n+1}})\). That is, we have to compute the \( L^{2} \) projection of the solution \( u_{n}^{h_{n}} \in V^{h_{n}} \) relative to the previous time step, onto \( V^{h_{n+1}} \). If numerical quadrature is used, we get

\[
(u_{n}^{h_{n}}, v_{n+1}^{h_{n+1}}) = \sum_{\Omega_{l}^{h_{n+1}} \in T_{h_{n+1}}} \sum_{j=1}^{n_{gp}} j_{l,j} w_{l,j} u_{n}^{h_{n}}(x_{l,j}^{h_{n+1}}) v_{n+1}^{h_{n+1}}(x_{l,j}^{h_{n+1}}).
\]
Thus, the data of the one step problem relative to the new mesh $V_{h_{n+1}}$ are obtained by sampling the field $u_{h_n}$, solution of the variational problem relative to the previous time step, at the new Gauss points $x_{h_{n+1}}$. For this reason, the term $(u_{h_n}, v_{h_{n+1}})$ is referred to as the variationally consistent data.

**Remark 4.10.** The previous sampling does not involve any complication, for the variable $u$ is a primary variable of the formulation, therefore, it is defined over all the domain $\Omega$ as finite element interpolation.

Nevertheless, also other fully discrete schemes have been proposed in relation to the way that the term $(u_{h_n}, v_{h_{n+1}})$ is treated. For example, in the analysis of the two–phase Stefan problem carried out by Nochetto et al. (1997) the aforementioned term is replaced in (4.9) with $(I_{V_{h_{n+1}}} u_{h_n}, v_{h_{n+1}})$, where $I_{V_{h_{n+1}}}$ is the nodal Lagrangian operator with respect to $V_{h_{n+1}}$ (Ciarlet, 1978). The resulting fully discrete scheme is stable and convergent under similar constraints on the change of the finite element mesh.

Another interesting fully discrete scheme is proposed by Dawson & Kirby (1999) for the analysis of 1D linear parabolic problems discretized in space with mixed finite element method. A piecewise constant approximation is assumed for the variable $u$ appearing in rate form. The term $(u_{h_n}, v_{h_{n+1}})$ is replaced with $\left(\bar{u}_n, v_{h_{n+1}}\right)$ where now $\bar{u}_n$ is a piecewise linear function obtained by ad hoc postprocessing of the piecewise constant function $u_{h_n}$. In this case, the authors prove convergence of the resulting discrete scheme which preserves the optimal convergence rate under very general changes in the mesh.

In conclusion, the definition of data as given in Nochetto et al. (1997) and in Dawson & Kirby (1999) are not variationally consistent, for the data of the one single fully discrete problem is not obtained from sampling at the new Gauss points the solution of the problem relative to the previous time step. Nevertheless, the resulting fully discrete schemes are convergent and stable.

**Remark 4.11.** For the definition of an *a posteriori* error estimate, it is not important to have a convergence result of the fully discrete scheme. This result is important, however, in the context of effectivity of the estimate and convergence of the adaptive process. This information is, in turn, implied by bounding above the *a posteriori* estimate with an *a priori* error estimate (Eriksson & Johnson, 1991).

### 4.5.1.2 Weak enforcement of the constitutive equations

The unambiguous definition of the initial state for the fully discrete scheme has been made possible by the assumption of the state variables appearing as data of the problem as primary variables of the formulation. As a result, their values could then be computed at any point of the domain by means of the respective interpolation functions. In a displacement formulation, on contrary, the primary unknown is only the displacement field whereas the secondary variables are obtained from
the pointwise solution of the equations used for the reduction of the global problem with respect to only the displacement, that is, the ClnNP. If these equations are also imposed in weak form along with an interpolation assumption for the secondary variables involved, it will be the same variational formulation to indicate how to state the data for the fully discrete problem in case of change of mesh. This observation, therefore, suggests enforcement of the ClnNP in a weak form and not in a pointwise manner, as it is implied by the standard displacement formulation. However, inasmuch as we are interested to perform transfer of the variables obtained from the displacement formulation, we must ensure that the solution obtained from this more general variational formulation conforms to the one obtained from the displacement formulation.

This can be easily achieved, as it is asserted in Ortiz & Quigley (1991), by an appropriate choice of the interpolation functions for the secondary variables and assuming the same element–base quadrature scheme. Since the fields \( \sigma(x), e_p(x), \alpha(x) \) are not involved in spatial derivative in the general variational formulation, unlike the displacement field \( u(x) \), the respective finite element interpolation functions are not required to be continuous over the element and across the element boundaries (Ortiz & Quigley, 1991) but only to meet general regularity properties. For example, the interpolated fields \( \sigma(x), e_p(x), \alpha(x) \) must be bounded (Radovitzky & Ortiz, 1999) or at most square integrable over \( \Omega \). This allows the variational equations of the constitutive equations to be imposed element by element.

Furthermore, if we assume the values of the field at the quadrature points as degrees of freedom for the element interpolant of the state variable, then the Galerkin finite element approximation becomes equivalent to the set of equations that enforce the constitutive equations at each Gauss point of the element. This equivalence of the displacement formulation based on element quadrature with underlying more general variational formulations will be shown next for the \( u - p \) formulation of linear elasticity and for a mixed formulation of plasticity which enforces in weak form some equations of the incremental form of the evolution laws.

In the \( u - p \) formulation of linear elasticity, the displacement \( u \) and the pressure \( p \) are assumed as independent variables. The additional variational equation other than the weak form of the equilibrium equation, is, therefore, given by (Bathe, 1996)

\[
\int_{\Omega} \left( \frac{p}{\kappa} + \epsilon_v \right) \delta p \, d\Omega = 0, \quad \forall \delta p \in L^2(\Omega)
\]  \tag{4.10}

where \( \delta p \) denotes the weighting function, \( \kappa \) is the bulk modulus,

\[
\kappa = \frac{E}{3(1 - 2\nu)}
\]

and \( \epsilon_v \) is the volumetric strain,

\[
\epsilon_v = \epsilon_{xx} + \epsilon_{yy} + \epsilon_{zz}.
\]
If we replace the space of the test functions with a finite dimensional space $\mathcal{P}^{h_{n+1}}$ built from discontinuous functions, and the degree of freedom of the element interpolant are the values of the function at the Gauss points of the element, equation (4.10) can be read element by element

$$\int_{\Omega}^{} \left( \frac{P}{\kappa} + \epsilon_v \right) \delta p^{h_{n+1}} \, d\Omega = 0, \quad \forall \delta p^{h_{n+1}} \in \mathcal{P}^{h_{n+1}}. \quad (4.11)$$

If element basis quadrature is adopted, one gets

$$p(x_{l,j}^{h_{n+1}}) = 0$$

which is the constitutive equation for the pressure enforced at the Gauss point $x_{l,j}^{h_{n+1}}$. This is an example of limitation principle for mixed formulations introduced by Fraeijs de Veubeke (1965). This principle shows that no particular advantage is gained by the use of the mixed formulation against the displacement one with the above choice of the interpolation functions (Malkus & Hughes, 1978). However, this is of no concern, for here we are interested that such equivalence with an underlying mixed formulation does exist.

In the mixed formulation of plasticity given in Simo et al. (1989) $\mathbf{u}, \sigma, \alpha$ and the plastic multiplier $\lambda$ are the independent variables. If we introduce the following set

$$K^p = \{ \delta \lambda \in L^2(\Omega) \mid \delta \lambda \geq 0 \}$$

the additional variational equations, other than the weak form of the equilibrium equation, are obtained by enforcing in a weak sense the following equations of the incremental form of the evolution laws

$$\int_{\Omega}^{} \left[ \nabla_{w} \mathbf{u}_{n+1} - e_{n}^{p} - \frac{\partial \psi_{p}^{*}}{\partial \sigma}(\sigma_{n+1}) - \lambda_{n+1} \frac{\partial f}{\partial \sigma}(\sigma_{n+1} A_{n+1}) \right] : \delta \sigma \, d\Omega = 0, \forall \delta \sigma \in (L^2(\Omega))^6$$

$$\int_{\Omega}^{} \left[ - \frac{\partial \psi_{p}^{*}}{\partial A}(A_{n+1}) + \frac{\partial \psi_{p}^{*}}{\partial A}(A_{n}) - \lambda_{n+1} \frac{\partial f}{\partial A}(\sigma_{n+1} A_{n+1}) \right] : \delta \alpha \, d\Omega = 0, \forall \delta \alpha \in (L^2(\Omega))^{n_{dim}}$$

$$\int_{\Omega}^{} f(\sigma_{n+1}, A_{n+1}) \delta \lambda \, d\Omega = 0, \forall \delta \lambda \in K^p$$

where $\delta \sigma$, $\delta \alpha$, $\delta \lambda$ denote the weighting functions and $n_{dim}$ equals the number of components of the tensor $\alpha$ with respect to a given basis. All the other equations of the constitutive model, on the other hand, are enforced pointwise.

For illustrative purposes, likewise the previous example, only the affine space of the test functions is replaced with conforming finite element spaces. If these spaces are built from discontinuous functions and the element shape interpolation functions of $\delta \sigma$, $\delta \alpha$, $\delta \lambda$ have the value at the Gauss points as degrees of freedom,
the discrete equations can be posed element by element

\[ \forall \Omega^i_{h^{n+1}} \subset \mathcal{T}_{h^{n+1}} \]
\[ \int_{\Omega^i_{h^{n+1}}} \left[ \nabla_s u_{n+1} - e^p_n - \frac{\partial \psi^s_\alpha}{\partial \sigma} (\sigma_{n+1}) - \lambda_{n+1} \frac{\partial f}{\partial \sigma} (\sigma_{n+1} A_{n+1}) \right] : \delta \sigma_{i,h^{n+1}}^\alpha \ d\Omega = 0 \]
\[ \int_{\Omega^i_{h^{n+1}}} \left[ - \frac{\partial \psi^s_\alpha}{\partial A} (A_{n+1}) + \frac{\partial \psi^s_\alpha}{\partial A} (A_{n}) - \lambda_{n+1} \frac{\partial f}{\partial A} (\sigma_{n+1} A_{n+1}) \right] : \delta \alpha_{i,h^{n+1}}^\alpha \ d\Omega = 0 \]
\[ \int_{\Omega^i_{h^{n+1}}} f(\sigma_{n+1}, A_{n+1}) \delta \lambda_{i,h^{n+1}}^\alpha \ d\Omega = 0 \]

and after using element based quadrature, one gets

\[ \forall \Omega^i_{h^{n+1}} \subset \mathcal{T}_{h^{n+1}} \]
for \( j = 1, \ldots, ngp \)

\[ e^p_{n+1}(x_{i,j}^{h^{n+1}}) - e^p_n(x_{i,j}^{h^{n+1}}) = \lambda_{n+1}(x_{i,j}^{h^{n+1}}) \frac{\partial f}{\partial \sigma} (\sigma_{n+1}(x_{i,j}^{h^{n+1}}) A_{n+1}(x_{i,j}^{h^{n+1}})) \]
\[ -\alpha_{n+1}(x_{i,j}^{h^{n+1}}) + \alpha_n(x_{i,j}^{h^{n+1}}) = \lambda_{n+1}(x_{i,j}^{h^{n+1}}) \frac{\partial f}{\partial A} (\sigma_{n+1}(x_{i,j}^{h^{n+1}}) A_{n+1}(x_{i,j}^{h^{n+1}})) \]
\[ f(\sigma_{n+1}(x_{i,j}^{h^{n+1}}), A_{n+1}(x_{i,j}^{h^{n+1}})) = 0 \]

which enforce at the Gauss points \( x_{i,j}^{h^{n+1}} \) of each element \( \Omega^i_{h^{n+1}} \) the equations of the incremental form of the evolution laws which have been considered in weak form.

The previous examples indicate that, in general, the displacement formulation of the incremental boundary value problem for the time step \([t_{n-1}, t_n]\) can be obtained from an underlying more general variational formulation which has also \( e^p \) and \( \alpha \) as independent variables. In this case, the following interpolation assumptions must hold over each element \( \Omega^h_{c,n} \subset \mathcal{T}_{h_{n}} \) for the components \( (e^p)_{a,b} \) and \( (\alpha)_{a,b} \),

\[ (h_n e^p)_{a,b} (x) = \sum_{i=1}^{ngp} e_{i,n} \bar{N}^{h_n} (e^p)_{a,b} (x) \]
\[ (h_n \alpha)_{a,b} (x) = \sum_{i=1}^{ngp} e_{i,n} \bar{N}^{h_n} (\alpha)_{a,b} (x) \]

(4.12)

with the elemental shape functions being piecewise continuous and meeting the following requirements

\[ e_{i,n} \bar{N}^{h_n} (e^p)_{a,b} (x_{i,k}) = \delta_{i,k} \]
\[ e_{i,n} \bar{N}^{h_n} (\alpha)_{a,b} (x_{i,k}) = \delta_{i,k} \]

(4.13)

In equation (4.13) \( \delta_{i,k} \) is the Kronecker symbol whereas in equation (4.12) the coefficients \( (h_n \bar{N} (\bullet)_{a,b}) \) identify with the value of the component of the respective field.
at the Gauss points $\mathbf{x}_{e,n}^{h_n}$. The latter results from the displacement finite element solution at $t_n$.

Figure 4.4 depicts some possible choices for $\epsilon^{iN}_{e_{ab}}$, for example, where $\epsilon^p_{ab}$ denotes a component of the second order tensor $\epsilon^p$.

![Figure 4.4: Possible choices for the interpolation functions of the internal variables which comply with the requirements of equation (4.13).](image)

Finally, the state of the system at $t_{n+1}$, that is, $\tilde{\epsilon}_{n}^{p}(x_{i,j}^{h_{n+1}})$ and $\tilde{\alpha}_{n}(x_{i,j}^{h_{n+1}})$, is obtained by sampling the fields (4.12) at the new Gauss points $\mathbf{x}_{i,j}^{h_{n+1}} \in \Omega_{e}^{h_{n+1}}$.

**Remark 4.12.** The equivalence of the displacement formulation with mixed variational formulations for models of associative plasticity has been object of analysis also in Comi & Perego (1995) and in Alfano et al. (1998). In these studies, the relevant variational equations are obtained as stationary conditions of a suitable functional. Hence, with an appropriate choice of the interpolation functions for the secondary variables, the equations of the displacement formulation using element basis quadrature are retrieved. These interpolations for the state variables may, therefore, be assumed as definition of other variationally consistent transfers in the sense of Ortiz & Quigley (1991).

In summary, variationally consistent transfer is obtained by making an interpolation assumption for $\epsilon^p$ and $\alpha$ which complies with the requirements of equation (4.13) and by sampling these fields at the Gauss points of the new mesh. Given the generality of the conditions (4.13), the specific choice of the interpolation functions comes from considerations of the accuracy of the solution. For example, Ortiz & Quigley (1991) refer to quadratic triangular elements with three Gauss points, thus assuming a linear interpolation for the secondary state variables. However, it was then noted in Camacho & Ortiz (1997), that the local properties of the solution at the Gauss points, in particular the isochoricity of the plastic strain, was not preserved by the transfer because of the linear interpolation. Thus, a first cure was to apply the transfer procedure to the logarithmic measures of the plastic strain rather than to the plastic strain. However, in areas undergoing rapid transients, the piecewise linear interpolation was observed to contain considerable noise away from the Gauss points.
from the Gauss points. This effect was therefore mitigated by smoothing the state variable fields prior to effecting their transfer. In this sense, however, the technique resembles the smoothing technique which is to be described next. In Radovitzky & Ortiz (1999), on the other hand, the authors consider a piecewise constant distribution of the state variables over the Voronoi cells (Frey & George, 2000), of all the Gauss points with respect to the whole domain. This results in defining the value at the new Gauss point equal to the one relative to the nearest Gauss point of the old mesh. With this choice for the interpolation of the state variables, the local properties of the solution are preserved, as well.

**Remark 4.13.** The definition of a variationally consistent transfer has the only advantage for providing naturally a variational setting where an error analysis could be carried out more easily. The outcome of this analysis would then be an error estimate which accounts for the weak enforcement, against the pointwise, of the equilibrium and of the incremental constitutive equations. However, this analysis seems to have been invoked but never actually performed by the cited authors. We will discuss in the following Chapters that the error in the constitutive equations will be able to account for the effects of the above approximations, in addition to the time discretization error in replacing the initial value constitutive problem with the incremental constitutive problem. In conclusion, it is worth noting that the definition of a variationally consistent transfer, by itself, does not guarantee that the most accurate adaptive finite element solution is delivered, as has been pointed out in the previous Section in the analysis of the heat conduction problem.

4.5.2 Weak enforcement of the continuity

This transfer procedure is obtained from an approximation *a la* Galerkin of the variational equation which imposes in weak form the continuity across the time node \( t_n \) of the variables which appear as data of the InBVP.

Let us assume, in the following, as general regularity property that \( \tilde{\mathbf{v}}_n \in (L^2(\Omega))^{n_{\text{dim}}} \) and also that \( \mathbf{v}_n \in (L^2(\Omega))^{n_{\text{dim}}} \), with \( n_{\text{dim}} \) equal to the number of components of the respective tensor field with respect to a given basis. The field \( \tilde{\mathbf{v}}_n \) is the data for the InBVP whereas \( \mathbf{v}_n \) is relative to the solution at the previous time interval.

The following condition

\[
\langle (\tilde{\mathbf{v}}_n) - (\mathbf{v}_n), \theta \rangle = 0, \quad \forall \theta \in (L^2(\Omega))^{n_{\text{dim}}}
\]  

(4.14)

enforces, in the weak form, the continuity of the field \( \mathbf{v}_n \) across the time node \( t_n \) with \( \langle \cdot, \cdot \rangle \) being an inner product for the space \( (L^2(\Omega))^{n_{\text{dim}}} \).

In Rashid (2002), condition (4.14) is enforced in a Galerkin sense by replacing the infinite dimensional space \( (L^2(\Omega))^{n_{\text{dim}}} \) with finite dimensional spaces defined by piecewise constant functions representing the distribution assumption for the variables \( e^p_n \) and \( \alpha^p_n \). These spaces are defined by constant functions over the Voronoi tessellation of each element with respect to its integration points and are denoted
next with $C_{hn}$ and $C_{hn+1}$ relative to $T_{hn}$ and $T_{hn+1}$, respectively. If $\omega_{c,i}^{hn}$ is the Voronoi cell relative to the point $x_{c,i}^{hn} \in \Omega_{c,i}^{hn}$, the field $h_{n}^{c,i} = h_{n}^{c,i}(x)$, for instance, is defined as follows

$$h_{n}^{c,i} = \sum_{\Omega_{c,i}^{hn} \in T_{hn}}^{n_{gp}} \int_{\omega_{c,i}^{hn}} h_{n}^{c,i}(x) \frac{\partial}{\partial x_{c,i}^{hn}}$$

where $\int_{\omega_{c,i}^{hn}}$ is the following function

$$\int_{\omega_{c,i}^{hn}} = \begin{cases} 1 & \text{if } x \in \omega_{c,i}^{hn} \\ 0 & \text{if } x \in \Omega - \omega_{c,i}^{hn} \end{cases}$$

and $h_{n}^{c,i}(x)_{c,i}^{hn}$ are the computed values from the solution at the previous time interval $[t_{n-1}, t_{n}]$.

Likewise, the data field will be represented by

$$\tilde{\epsilon}_{n}^{p}(x) = \sum_{T_{hn+1}, n_{gp}} \int_{\omega_{c,i}^{hn+1}} \tilde{\epsilon}_{n}^{p}(x)_{c,i}^{hn+1}$$

where $\tilde{\epsilon}_{n}^{p}(x)_{c,i}^{hn+1}$ are the unknowns to be determined.

Thus, equation (4.14) can be used to formulate the following problem,

Given: $h_{n}^{c,i} = C_{hn}$

Find: $\tilde{\epsilon}_{n}^{p}(x) \in C_{hn+1}$

Such that:

$$\langle h_{n}^{c,i} - \tilde{\epsilon}_{n}^{p}, \theta \rangle = 0, \quad \forall \theta \in C_{hn+1}$$

which finally delivers $\tilde{\epsilon}_{n}^{p}(x) \in C_{hn+1}$ as projection of $h_{n}^{c,i} = h_{n}^{c,i}(x) \in C_{hn}$ onto $C_{hn+1}$ with respect to the inner product $\langle \cdot, \cdot \rangle$ of $L^{2}(\Omega)^{n_{dim}}$.

Same procedure can be applied to compute $\tilde{\alpha}_{n}(x) \in C_{hn+1}$.

By definition, this transfer is, therefore, self-consistent, inasmuch as it is a projection operator. Furthermore, because of the prolongation assumption into piecewise constant functions, it preserves the local character of the state variables at the relative Gauss point, as well.

4.5.3 Smoothing transfer

This procedure represents perhaps the most widely used remapping algorithm in solid mechanics applications for its relatively simple implementation. Details on the transfer operation can be found, among others, in Lee & Bathe (1994) and Perić et al. (1996). Next, we just sketch the main steps which are summarized in Figure 4.5. The values of the state variables $(\bullet)$ at the old Gauss points, $(\bullet)(x_{N}^{hn})$, are first transferred to the nodes of the old mesh, $e,i(\bullet)(x_{N}^{hn})$, possibly also with a weight.
A weighted average is then carried out at each node, \((\bullet)(\mathbf{x}^{h_n})\), and a smooth field, \((\bullet)(\mathbf{x}^{h_n})\), is consequently defined by interpolation of the nodal values by means of the basis functions of the finite element space, \(\mathcal{V}^{h_n}\), associated with the old mesh. The nodal interpolant of this field with respect to the new finite element space, \((\bullet)(\mathbf{x}^{h_{n+1}}) = \mathcal{I}^{h_{n+1}}(\bullet)(\mathbf{x}^{h_n})\), is constructed and the resulting field is sampled at the new Gauss points delivering therein the transferred values of the state variables, \((\tilde{\bullet})(\mathbf{x}^{h_{n+1}}_{l,j})\).

Some of the above steps can be by-passed and each of them can be tackled in different ways, delivering a fairly large spectrum of transfer procedures. On one end, there are those that obtain the values at the new Gauss points directly in terms of the values at the old Gauss points by means of interpolations which can be global or local over predefined neighbourhoods of the new Gauss points, such as in Tabbara et al. (1994); Boroomand & Zienkiewicz (1998) and Villon et al. (2000). On the other end, there are the procedures described in Figure 4.5, such as in Lee & Bathe (1994) and Perić et al. (1996). However, when the extrapolation to the nodes is considered, this represents a delicate point of the whole transfer process. Many studies have been carried out in the field of the so-called recovery procedures and various schemes such as least square fitting (Hinton & Campbell, 1974), superconvergence–patch recovery (SPR) (Zienkiewicz & Zhu, 1992a), moving least square (Tabbara et al., 1994), etc. have been proposed. Despite its simplicity, the procedure however is not self-consistent and if frequent remeshing takes place diffusion of plastic strain can spread all over the domain because of the smoothing operation. Also, special care must be paid in accepting the transferred values. In fact, some properties cannot be inherited by the values at the new Gauss points, namely the incompressibility of the plastic strain, for this constraint does not commute in general with the extrapolation operation.

Remark 4.14. The previous classification is by no means exhaustive. A fairly general up–date account of current adaptive strategies in elastoplasticity can be found, however, in Ladevèze & Oden (1998).
4.6 Numerical techniques. Newton-Raphson method

Without loss of generality and only for notational convenience, in the following we refer to the formulations of the discrete problems given in Section 4.4 rather than to problem (4.6), which is the one to be solved in an actual computation. Nevertheless, the considerations are general and can be easily adapted to (4.6).

The discrete problem (4.4), which here we formulate with generic data \( \bar{\epsilon}_n(x) \), \( \bar{\alpha}_n(x) \), represents an algebraic system of nonlinear equations that is convenient to write in the following form,

\[
\begin{align*}
\text{Data:} & \quad \text{External Loading} \quad & b_{n+1}(x), \text{ on } \Omega \\
& \quad \text{t}_{n+1}(x), \text{ on } \partial \Omega_t \\
& \quad \text{State of the system at } t_n \\
\text{Find:} & \quad \bar{\epsilon}_{n+1} \in \mathcal{E} \\
& \quad \bar{\alpha}_n(x) \in \mathcal{L} \\
\text{Such that:} & \quad \mathcal{L}(\nabla_x \bar{u}_{n+1}, \bar{\eta}_{n+1}) \defeq \\
& \quad \begin{aligned}
& \left\langle \sigma_{n+1}(\nabla_x \bar{u}_{n+1}(x)), \nabla \eta_{n+1}(x) \right\rangle + \\
& - \left\langle b_{n+1}(x), \eta_{n+1}(x) \right\rangle - \left\langle t_{n+1}(x), \eta_{n+1}(x) \right\rangle_{\partial \Omega_t} = 0 \\
& \forall \eta_{n+1} \in \mathcal{V}_{n+1},
\end{aligned}
\end{align*}
\]

where the stress tensor \( \sigma_{n+1}(\epsilon_{n+1}(x) = \nabla_x \bar{u}_{n+1}(x)) \), we recall, is the function of \( \epsilon_{n+1} \) defined implicitly at any point \( x \in \Omega \) by the solution of the following problem with generic data \( \bar{\epsilon}_n(x), \bar{\alpha}_n(x) \),

\[
\begin{align*}
\text{Data:} & \quad \bar{\epsilon}_n, \bar{\alpha}_n \\
\text{Given:} & \quad \epsilon_{n+1} = \nabla_x \bar{u}_{n+1}(x) \\
\text{Find:} & \quad h_{n+1} \epsilon_{n+1}, h_{n+1} \epsilon_n, h_{n+1} \alpha_{n+1}; \quad h_{n+1} \sigma_{n+1}, h_{n+1} A_{n+1} \\
\text{Such that:} & \quad \epsilon_{n+1} = \epsilon_{n+1} \epsilon_{n+1} \epsilon_{n+1} \epsilon_{n+1} \\
& \quad \begin{aligned}
& h_{n+1} \sigma_{n+1} = \frac{\partial \psi}{\partial \epsilon}(h_{n+1} \epsilon_{n+1}); \quad h_{n+1} A(x, t_{n+1}) = \frac{\partial \psi}{\partial \alpha}(h_{n+1} \alpha_{n+1}) \\
& \frac{h_{n+1} \epsilon_{n+1} - \bar{\epsilon}_n}{k_n} \in \mathcal{F}(h_{n+1} \sigma_{n+1}, h_{n+1} A_{n+1}) \\
& \frac{h_{n+1} \alpha_{n+1} - \bar{\alpha}_n}{k_n} \in \mathcal{G}(h_{n+1} \sigma_{n+1}, h_{n+1} A_{n+1})
\end{aligned}
\end{align*}
\]
For the considerations of Section 4.4 we assume that problem (4.16) has solution for generic data $\tilde{\epsilon}_n(x), \tilde{\alpha}_n(x)$ as long as minimum regularity requirements are satisfied.

**Remark 4.15.** The functional $L = L(\nabla_s u_n^{h_{n+1}}, \eta_n^{h_{n+1}})$ is defined over $\gamma^{h_{n+1}} \times V_0^{h_{n+1}}$. However, next, whenever necessary, we will refer only to the dependence on $u_n^{h_{n+1}} \in \gamma^{h_{n+1}}$.

There exist several techniques to solve the system (4.16) (Crisfield, 1991; Dennis & Schnabel, 1996) which are mostly of iterative type. They, fundamentally, differentiate each other for their rate of convergence and for being locally or globally convergent according to whether or not, they require a starting value which is close to the exact solution.

A technique particularly attractive is the fully consistent Newton’s method which replaces iteratively in the equation $L(\nabla_s u_n^{h_{n+1}}, \eta_n^{h_{n+1}}) = 0$ the function $L(\nabla_s u_n^{h_{n+1}}, \eta_n^{h_{n+1}})$ by its first order expansion around the point $u_n^{h_{n+1}(i)} \in \gamma^{h_{n+1}}$ (Le Tallec, 1994; Bonet & Wood, 1997). Further to the assumption of small displacements, this is obtained simply by replacing the stress function in the weak form of the equilibrium with its first order Taylor series, consequently the generic iterate $u_n^{h_{n+1}(i+1)}$ is obtained by solving the following linear system:

**Data:** $u_n^{h_{n+1}(i)} \in \gamma^{h_{n+1}}$

**Find:** $u_n^{h_{n+1}(i+1)} \in \gamma^{h_{n+1}}$

$$
\begin{align*}
\langle D^{h_{n+1}} \sigma_n^{h_{n+1}} \left( \nabla_s u_n^{h_{n+1}(i)} \right), \nabla_s u_n^{h_{n+1}(i+1)} - \nabla_s u_n^{h_{n+1}(i)} \rangle & = \langle b_n^{h_{n+1}}, \eta_n^{h_{n+1}} \rangle + \langle t_{n+1}, \eta_n^{h_{n+1}} \rangle \partial \Omega - \langle \sigma_n^{h_{n+1}} \left( \nabla_s u_n^{h_{n+1}(i)} \right), \nabla_s \eta_n^{h_{n+1}} \rangle \\
\forall \eta_n^{h_{n+1}} & \in \gamma^{h_{n+1}}
\end{align*}
$$

(4.18)

where $h_{n+1} \sigma_n^{h_{n+1}} \left( \nabla_s u_n^{h_{n+1}(i)} \right)$ is obtained from the solution, at any point $x \in \Omega$, of the problem (4.17) with $\epsilon_{n+1}^{h_{n+1}(i)}(x) = \nabla_s u_n^{h_{n+1}(i)}(x)$ and data $\tilde{\epsilon}(x), \tilde{\alpha}(x)$, whereas

$$
D^{h_{n+1}} \sigma_n^{h_{n+1}} \left( \nabla_s u_n^{h_{n+1}(i)} \right) \left[ \nabla_s u_n^{h_{n+1}(i+1)} - \nabla_s u_n^{h_{n+1}(i)} \right]
$$

(4.19)

is the directional derivative of the function $h_{n+1} \sigma_n^{h_{n+1}} = h_{n+1} \sigma_n^{h_{n+1}} \left( \epsilon_{n+1}^{h_{n+1}(i)}(x) \right)$ at $\epsilon_{n+1}^{h_{n+1}(i)} = \epsilon_{n+1}^{h_{n+1}(i)}$ along the direction $\Delta \epsilon_{n+1}^{h_{n+1}} = (\epsilon_{n+1}^{h_{n+1}(i+1)} - \epsilon_{n+1}^{h_{n+1}(i)})$, and is referred to as the algorithmic stiffness (Simo & Taylor, 1985).

The iterative process is stopped when a norm of the residual

$$
h_{n+1} R_n^{h_{n+1}(i+1)}(\eta_n^{h_{n+1}}) = \langle b_n^{h_{n+1}}, \eta_n^{h_{n+1}} \rangle + \langle t_{n+1}, \eta_n^{h_{n+1}} \rangle \partial \Omega - \langle h_{n+1} \sigma_n^{h_{n+1}} \left( \nabla_s u_n^{h_{n+1}(i+1)} \right), \nabla_s \eta_n^{h_{n+1}} \rangle
$$

and/or of the displacement increment

$$
u_n^{h_{n+1}(i+1)} - u_n^{h_{n+1}(i)}
$$
is sufficiently small.

The algorithm (4.18) for given starting value \( u^{h_{n+1},(0)}_{n+1} \) generates a sequence of iterates which under some conditions is shown to be convergent. More precisely, if \( u^{h_{n+1}}_{n+1} \) is the exact solution of (4.16) and \( u^{h_{n+1},(0)}_{n+1} \) is sufficiently close to \( u^{h_{n+1}}_{n+1} \), the following is valid (Ortega & Rheinboldt, 2000)

\[
\|u^{h_{n+1},(i+1)}_{n+1} - u^{h_{n+1}}_{n+1}\| \leq \beta \|u^{h_{n+1},(i)}_{n+1} - u^{h_{n+1}}_{n+1}\|^2.
\]

Remark 4.16. In applying Newton’s method for the solution (4.16), the starting point \( u^{h_{n+1},(0)}_{n+1} \) has no physical meaning. It is only the solution of (4.16) to have physical meaning. The starting point is only required to be close to the exact solution and to belong to the domain of the function \( \mathcal{L}(\nabla_s u^{h}_{n+1}, \eta^{h_{n+1}}) \).

The initialization strategy represents, therefore, an important issue for the success of the method. If mesh does not change from one time step to the other, the functional \( \mathcal{L}(\nabla_s u^{h}_{n+1}, \eta^{h}) \) is defined over \( \mathcal{V}^h \times \mathcal{V}^h \). Therefore, we can assume \( u^{h,(0)}_{n+1} = u^h_n \), solution at the previous time step. As a result, the initial residual is given by

\[
hR^{(0)}_{n+1}(\eta^h) = \langle b_{n+1}, \eta^h \rangle + \langle t_{n+1}, \eta^h \rangle_{\partial \Omega_t} - \langle \sigma_{n+1}(\nabla_s u^{h,(0)}_{n+1}), \nabla_s \eta^h \rangle,
\]

and for the choice of the starting value, it is also

\[
\langle \sigma_{n+1}(\nabla_s u^{h,(0)}_{n+1}), \nabla_s \eta^h \rangle = \langle b_n, \eta^h \rangle + \langle t_n, \eta^h \rangle_{\partial \Omega_t}.
\]

Thus, in this case, if \( u^{h,(0)}_{n+1} \) is not close to \( u^h_{n+1} \), that is, the method fails to converge, then by reducing the external load level we reduce the initial residual, which eventually approaches to zero, that is,

\[
\lim_{t_{n+1} \to t_n} hR^{(0)}_{n+1}(\eta^h) = 0.
\]

Thus, one may expect that with the above choice of the starting value the algorithm finally will converge.

If the mesh changes from one time step to the other, the functional \( \mathcal{L}(\nabla_s u^{h_{n+1}}, \eta^{h_{n+1}}) \) is defined over \( \mathcal{V}^{h_{n+1}} \times \mathcal{V}^{h_{n+1}} \), therefore, \( u^{h_n} \in \mathcal{V}^{h_n} \) cannot be used as starting value, for \( u^{h_{n+1},(0)}_{n+1} \) must belong to \( \mathcal{V}^{h_{n+1}} \). In this case, the definition of the starting value becomes an important issue, for the residual does not vanish by reducing the external load level, that is,

\[
\lim_{t_{n+1} \to t_n} hR^{(0)}_{n+1}(\eta^{h_{n+1}}) \neq 0.
\]

Thus, even though we adopt the incremental load procedure within the step as initialization procedure, the algorithm may still fail to converge. This is to be related, finally, to the value of the residual

\[
h_{n+1}R^{(0)}_n(\eta^{h_{n+1}}) = \langle b_n, \eta^{h_{n+1}} \rangle + \langle t_n, \eta^{h_{n+1}} \rangle_{\partial \Omega_t} - \langle \sigma_{n+1}(\nabla_s u^{h_{n+1},(0)}_{n+1}), \nabla_s \eta^{h_{n+1}} \rangle.
\]

128
As a result, other solution techniques must be envisaged to solve the system (4.16) or to build a starting value which is good for the success of Newton’s method, such as, for instance, the line search method whose general principles are recalled in the next section.

4.6.1 Line search method

Lack of convergence of the Newton–Raphson’s method is usually shown either by an increase or by an oscillation of the norm of the residual of the iterates. The line search method avoids these behaviours by imposing a decrease in the residual at each iteration.

The principle of the method is quite simple (Dennis & Schnabel, 1996): If the residual at the proposed new iterate \( u_{n+1}^{(i+1)} = u_{n+1}^{(i)} + \Delta u_{n+1}^{(i)} \), one tries to find along a given direction \( d \) an acceptable point \( u_{n+1}^{(i+1)} = u_{n+1}^{(i)} + \eta d \) whose residual is lower.

As direction \( d \), one usually takes the solution \( \Delta u_{n+1}^{(i)} \) obtained from the Newton–Raphson of the current step which has resulted unsatisfactory, whereas for the choice of the scalar \( \eta \), we refer to the criterion given in Bonet & Wood (1997) which approximates the norm of the residual dependent on \( \eta \) with a quadratic function. The resulting algorithm is described in Box 4.5 where the method,

**Box 4.5. Newton–Raphson with line search method**

1. Given \( u_{n+1}^{(0)} \), and the residual functional \( h_{n+1} R_{n+1}^{(0)}(\eta) \) with \( \eta \in \mathcal{V}_{n+1}^0 \)

2. Compute \( u_{n+1}^{(1)} \) with Newton–Raphson

3. Set \( d = (u_{n+1}^{(1)} - u_{n+1}^{(0)}) \in \mathcal{V}_{n+1}^0 \)

4. IF \( h_{n+1} R_{n+1}^{(1)}(d) \leq \rho h_{n+1} R_{n+1}^{(0)}(d) \) use Newton–Raphson

   ELSE \( \alpha = \frac{h_{n+1} R_{n+1}^{(0)}(d)}{h_{n+1} R_{n+1}^{(1)}(d)} \)

   IF \( \alpha > 0 \), \( \eta = \frac{\alpha}{2} \)

   ELSE \( \eta = \frac{\alpha}{2} + \sqrt{\left(\frac{\alpha}{2}\right)^2 - \alpha} \)

   END IF

   END IF

5. Compute \( u_{n+1}^{(1)} = u_{n+1}^{(0)} + \eta d \) and GOTO 3

where, typically, a value of \( \rho = 0.5 \) is used (Bonet & Wood, 1997).
method, which is a fast local convergent strategy, obtaining a strategy which inherit the benefits of both.

4.7 Concluding Remarks

In this Chapter we have described the displacement finite element solution of an elastoplastic model with internal variables discretized in time with the backward Euler method. A critical analysis of the nature of the discretization errors introduced in the formulation of the fully discrete scheme has been provided. In particular, the fundamental observation has been made that change of data and/or finite element mesh from one time interval to the other can be both related to a discontinuity jump of the approximate solution across the time instant $t_n$. Also, a critical review of the current techniques to transfer of the data from one mesh to the other has been given.

However, before proceeding to the study of the transfer procedures in terms of the error produced, in the next Chapter we will first consider how to use the extended dissipation error to assess the quality of finite element solutions with meshes constant throughout the loading process. The objective is to show that this error measure is able to account for effects of time and space discretization error, and also to provide the relative importance of the error components associated with the residual in the state law and in the evolution law.
Chapter 5

The error in the constitutive equations to assess the quality of the finite element solution with mesh constant in time

5.1 Introduction

Objective of this chapter is the application of the general theory of the error in the constitutive equations developed in Chapter 3 to the assessment of the quality of finite element solutions of elastoplasticity problems.

The general properties of an admissible solution of the problem under consideration have been given and discussed in Section 3.5. For instance, with respect to the computation of the extended dissipation error, an admissible solution is the set of the time dependent fields of the state variables which satisfy the compatibility and equilibrium equations along with the initial conditions. As a result, a finite element solution obtained for instance by a displacement formulation, is not in general an admissible solution. The values are discrete in time and the finite element stresses do not satisfy the equilibrium equations in a pointwise manner.

In order to apply the theory of the error in the constitutive equations, given the finite element solution, a corresponding admissible solution must be therefore defined. However, if the admissible solution must reflect the approximations associated with the finite element solution, it is understood that some conditions are necessary for its definition (Ladevèze & Pelle, 2001). A rather general one can be expressed in the following terms

Given a family of finite element solutions which converges to the exact solution as the discretization parameters approach their corresponding limit values, then also the family of corresponding admissible solutions converges to the exact solution and possibly with the same rate of convergence.
In linear elasticity, this condition refers to the definition of the so called prolongation conditions, that is, to the conditions posed on the definition of the statically admissible stress fields in terms of the computed finite element stresses, so that the error in the constitutive equation can be bounded above by the exact error in solution. A thorough analysis of the meaning of this condition can be found in Ladevèze & Leguillon (1983) and Ladevèze & Pelle (2001) for the constant triangular element and is referred to as strong prolongation condition.

In this chapter, the finite element mesh is assumed to be constant throughout the time evolution of the continuum. As a result, the admissible solution corresponding to the finite element solution will be time continuous. Criteria to build a such admissible solution for the computation of the extended dissipation error are first given in general and then detailed for the Prandtl-Reuss model. Numerical analysis of its performance compared to classical measures of the error are illustrated on a 1D model problem. In the second part of the chapter, we recall also the dissipation error and compare its performance to the extended dissipation error.

5.2 Extended Dissipation Error

In the definition of an admissible solution for the computation of the extended dissipation error, the statically admissible variables are not constrained to their conjugate variables by means of the state laws as it happens in the dissipation error. This allows more information from the finite element solution to be included in building the corresponding admissible solution and strengthen the link between the two solutions. For example, in the case of a \( J_2 \)-plasticity model and of finite element solution which delivers plastic strains meeting the incompressibility condition, the computed plastic strain field can be assumed, in some circumstances, as part of the admissible solution. In particular, in a displacement finite element formulation the plastic strain, known only at the Gauss points \( \mathbf{x}_{e,i}^h \) of the element used for the numerical integration of the constitutive equations, can be extended over the element in a field which continues to meet the incompressibility condition. This can be realized, for example, by assuming each element partitioned by the Voronoi cells associated with each Gauss point and assuming a constant distribution of the plastic strain over the cell equal to the value of the strain at the respective Gauss point. In this way we define a plastic strain field whose incompressibility is guaranteed at almost every point of the domain. For elements with only one Gauss point, the field will be clearly constant over the element. Figure 5.1 depicts, for instance, the definition of the Voronoi cells for the triangular element when one or three Gauss points are used for the numerical integration of the constitutive equations, respectively, along with the assumed distribution of internal variables. Clearly, one can envisage also other partitions of the element along with relative definition of the state variables distribution. It will follow that the error in the constitutive equations will have to be considered as the error associated with the given assumption for the state variables distribution.
Figure 5.1: Partition of a triangular element with Voronoi cells relative to its Gauss points along with respective assumed distribution for the internal variables

Likewise, conforming finite element displacements can be used as part of the admissible solution and do not need to be modified, unlike for the definition of the admissible solution to compute the dissipation error, as it will be discussed in the following (Moës, 1996).

A key feature of the analysis and implementation of the error in the constitutive equations, however, is in general the definition of an equilibrated stress field $\sigma_{ad}(x, t_{n+1})$ linked to the finite element solution $^h\sigma_{n+1}(x^h_{e,i})$. A substantial body of research has been devoted to recover more or less equilibrated stress field on the basis of the computed finite element stresses such as in Stein & Ahmand (1977), Zienkiewicz & Zhu (1992c), Ainsworth & Oden (1993), Wiberg et al. (1994) and de Miranda & Umbertini (2002), among others. In these works different conditions are given to relate the recovered stress field to the finite element stresses. In the following we refer to the techniques initiated by Ladevèze (1975) where the so-called prolongation condition depending in general on the regularity of the mesh establishes the aforementioned link (see, e.g., Ladevèze, 1994; Ladevèze & Pelle, 2001). In particular, hereafter, we apply the strong prolongation condition as introduced and analysed in Ladevèze (1975), Ladevèze & Leguillon (1983) and Ladevèze & Pelle (2001). This condition distinguishes the statically admissible stress fields $\sigma_{ad}(x, t_{n+1})$ which satisfy the following equation for every shape function $N_i(x)$ and for all the elements $\Omega_e$,

$$\int_{\Omega_e} \left( \sigma_{ad}(x, t_{n+1}) - ^h\sigma_{n+1}(x^h_e) \right) : \nabla N_i(x) \, d\Omega = 0. \quad (5.1)$$

For anisotropic meshes, the condition (5.1) is required to hold for any element $\Omega_e$ and for any shape function of higher order which is associated with a non-vertex node. This condition has been termed as weak prolongation condition in Ladevèze (1994) and Ladevèze & Rougeot (1997).

The only unknowns left apart and necessary to determine a complete admis-
sible solution are, therefore,
\[ p_{ad}(x, t_{n+1}), \alpha_{ad}(x, t_{n+1}); R_{ad}(x, t_{n+1}), X_{ad}(x, t_{n+1}). \]

For their computation, the general method of minimization of the error introduced in Ladevèze et al. (1999) can be adopted. However, in general, it may be much more convenient to resort to the simpler criterion given in Ladevèze & Moës (1997) which resembles the integration of the evolution law for the constitutive model which is used. Next, we detail this construction for the Prandtl-Reuss model.

### 5.2.1 Construction of the admissible solution

The Prandtl-Reuss plasticity model with Linear Hardening

The essential equations for this model are hereafter recalled, cf. Section 3.2.4.5,

\begin{align*}
\text{Yield Condition:} & \quad \| \sigma_{ad}^D \| - (R_{ad} + R_0) \leq 0, \quad R_{ad} \geq 0 \\
^s h^2 y_{xt}(\sigma_{ad}, R_{ad}, \epsilon_{ad}^e, p_{ad}) = & \quad \left( \sigma_{ad} - C \epsilon_{ad}^e \right) : C^{-1} \left( \sigma_{ad} - C \epsilon_{ad}^e \right) + \\
& \quad + \left( R_{ad} - H p_{ad} \right) H^{-1} \left( R_{ad} - H p_{ad} \right); \\
^d h^2 y_{xt}(\sigma_{ad}, R_{ad}, \epsilon_{ad}^p, \dot{p}_{ad}) = & \quad R_0 \| \epsilon_{ad}^p \| - \sigma_{ad} : \dot{\epsilon}_{ad}^p + R_{ad} \dot{p}_{ad},
\end{align*}

with Tr[\epsilon_{ad}^p] = 0 and \( \dot{p}_{ad} \geq \| \dot{\epsilon}_{ad}^p \| \), where \( \| q \| = \sqrt{q : q} \) is the norm of the second order tensor \( q \).

In the following, we refer to the solution of a fully implicit conforming finite element displacement formulation of the initial boundary value problem of the plasticity model under consideration. The primary variable is given at the discrete time instants \( t_{n+1} \) in terms of the finite element displacement field \( u_{n+1}(x) \), whereas the secondary variables, such as \( h \sigma_{n+1}, h \epsilon_{n+1}, h p_{n+1} \), are issued only at the Gauss points \( x_{e,i}^h \) used for the numerical quadrature of the internal virtual work. As a result of the considerations of Section 4.2, we recall that on conceptual level, all the secondary variables can be obtained at any point \( x \in \Omega \) provided that at this point and for all the previous discrete time instants \( t_n \), the incremental constitutive value problem has been solved. This would require to store the computed finite element displacement field at each discrete time instant \( t_n \) and consequently huge memory capacities would be necessary. By contrast, in an incremental solution procedure, only the accepted solution at the previous discrete time instant is stored at most. The stress field which would be so obtained would not satisfy anyway the equilibrium equations in a pointwise manner, thus the finite element solution cannot be used to compute the extended dissipation error.

Objective of this section is to propose a procedure to build an admissible solution corresponding as close as possible to the computed finite element solution and make use of minimum memory requirements. This is feasible because equation (3.47) shows that the extended dissipation error at the time \( t_{n+1} \) can be expressed
in terms of only the error at the time \( t_n \) and the admissible solution over \([t_n, t_{n+1}]\). As a result, the admissible solution can be defined in an incremental manner, as well. Consistently with an assumed linear variation of the external load over each time interval, and the convexity of the equilibrium and compatibility conditions, the admissible solution is taken to vary linearly over \([t_n, t_{n+1}]\). Therefore, for its complete definition we need to solve the following problem

**Given:** the admissible solution at \( t_n \),
the finite element solution at \( t_{n+1} \),

**Find:** a corresponding admissible solution at \( t_{n+1} \).

As far as the definition of the admissible displacement field and the equilibrated stress field are concerned, the general considerations given in the previous section apply. In particular, the equilibrated stress field is obtained with a two-stage procedure. The first consists, by means of the prolongation condition, in defining an equilibrated traction forces along the boundary of each element and the second in solving the equilibrium equations over each element, usually, with higher order elements. For technical details we refer to Rougeot (1989), Ladevèze et al. (1991), Ladevèze & Rougeot (1997) and Ladevèze & Pelle (2001). The outcome is the definition of a statically admissible stress field \( \sigma_{ad}(x, t_{n+1}) \), which is known at any point \( x \in \Omega \) and is continuous over \( \Omega \).

Also, we consider hereafter the computed finite element plastic strain to be extended over the domain with the criterion given in the previous section so that it makes sense to write \( ^h e^{p}_{n+1}(x) \) for each \( x \in \Omega \). For the definition of other admissible state variables \( e^{p}_{ad}(x, t_{n+1}), p_{ad}(x, t_{n+1}), R_{ad}(x, t_{n+1}) \) we start from the general procedure indicated by Ladevèze et al. (1999), which considers the minimization of the extended dissipation error over the set of the admissible values for the remaining state variables. The minimization can be carried out at each point of the domain since there are no spatial derivative involved in the constitutive equations, in particular it will be done at the Gauss points used to compute numerically the space integrals that define the error (cf. Ladevèze & Pelle, 2001). These quadrature points do not have to be confused with those where the constitutive equations are integrated numerically. These points, in turn, are the one used to compute numerically the integral that appear in the internal virtual power.

The minimization problem in its general terms is described in Box 5.1. We propose an approximate solution of this problem built as follows. The admissible thermodynamic force \( R_{ad}(x, t_{n+1}) \) is defined as

\[
R_{ad}(x, t_{n+1}) = \max \{ R_1, R_2 \}
\]

where

\[
R_1 = \| \sigma^{p}_{ad}(x, t_{n+1}) \| - R_0
\]

\[
R_2 = R_{ad}(x, t_{n}).
\]
Box 5.1. Definition of the state variables as minimization of the extended dissipation error

For each \( x \in \Omega \)

Given:

\[ \sigma_{ad}(x, t_n), R_{ad}(x, t_n); \]

\[ \epsilon_{ad}(x, t_n), \epsilon_{ad}^p(x, t_n), p_{ad}(x, t_n). \]

\[ \sigma_{ad}(x, t_{n+1}), \epsilon_{ad}(x, t_{n+1}) \]

Find:

\[ R_{ad}(x, t_{n+1}); \]

\[ \epsilon_{ad}^p(x, t_{n+1}), p_{ad}(x, t_{n+1}) \]

such that by assuming a time linear variation over \([t_n, t_{n+1}]\) of the variables

\[ \sigma_{ad}(x, t), R_{ad}(x, t); \]

\[ \epsilon_{ad}(x, t), \epsilon_{ad}^p(x, t), p_{ad}(x, t), \]

we realize the minimum of the following function

\[
F(\sigma_{ad}, R_{ad}; \epsilon_{ad}, \epsilon_{ad}^p, p_{ad}) =
\sup_{t_n \leq t \leq t_{n+1}} \left\{ \left[ \sigma_{ad}(x, t) - C \epsilon_{ad}^p(x, t) \right] : C^{-1} \left[ \sigma_{ad}(x, t) - C \epsilon_{ad}^p(x, t) \right] + \right.
\]

\[ + \left[ R_{ad}(x, t) - H p_{ad}(x, t) \right] H^{-1} \left[ R_{ad}(x, t) - H p_{ad}(x, t) \right] + \]

\[ + \int_{t_n}^{t} \left[ R_0 \| \epsilon_{ad}^p(x, \tau) \| - \sigma_{ad}(x, \tau) : \epsilon_{ad}^p(x, \tau) + R_{ad}(x, \tau) p_{ad}(x, \tau) \right] d\tau \}
\]

under the following constraints

\[ \forall t \in [t_n, t_{n+1}] \]

\[ ||\sigma_{ad}(x, t)|| - (R_{ad}(x, t) + R_0) \leq 0, \]

\[ \dot{p}_{ad}(x, t) \geq \| \dot{\epsilon}_{ad}^p(x, t) \|, \]

\[ \text{Tr} [\epsilon_{ad}^p(x, t)] = 0 \]

\[ \epsilon_{ad}(x, t) = \epsilon_{ad}^e(x, t) + \epsilon_{ad}^p(x, t) \]

\[ R_{ad}(x, t) \text{ non–negative and non–decreasing} \]
The admissible plastic strain, on the other hand, will be given by
\[ \epsilon_{ad}^{p}(x, t_{n+1}) = h\epsilon_{n+1}^{p}(x) \]
if the following condition is satisfied,
\[ \sigma_{ad}(x, t_{n+1}): [h\epsilon_{n+1}^{p}(x) - \epsilon_{ad}^{p}(x, t_{n})] \geq 0. \] (5.2)
Otherwise we choose
\[ \epsilon_{ad}^{p}(x, t_{n+1}) = \epsilon_{ad}^{p}(x, t_{n}). \]

This definition of \(\epsilon_{ad}^{p}(x, t_{n+1})\) will guarantee the nonnegativity of the implicit expression of the plastic work.

Finally, with regard to the admissible accumulated plastic strain \(p_{ad}(x, t_{n+1})\) it is
\[ p_{ad}(x, t_{n+1}) = p_{ad}(x, t_{n}) + \|\epsilon_{ad}^{p}(x, t_{n+1}) - \epsilon_{ad}^{p}(x, t_{n})\| \]
which corresponds to the integration of the equation \(\dot{p} = \|\epsilon_{ad}^{p}\|\) that occurs for the model under consideration by assuming linear variation of the variables over \([t_{n}, t_{n+1}].\)

Once all the admissible state variables have been computed at \(t_{n+1}\), the use of a time linear interpolation over \([t_{n}, t_{n+1}]\) guarantees the admissibility of the solution for the convexity of the equilibrium and compatibility conditions. Also, the above procedure delivers an admissible solution which produces a finite value of the error for the convexity of the domains \(E\) and \(C\) introduced in Section 3.2.4.5.

**Remark 5.1.** The use of a time linear interpolation of the computed nodal values has the implied assumption that the accuracy of this time dependent function is the same as the computed nodal values. This is not the case in general but it holds for small values of the time step \(\Delta t\).

Figure 5.2 reports schematically the notation relative to the finite element solutions and corresponding admissible solutions in the case of finite element mesh constant in time.

### 5.2.2 Error Expressions

In the section Error Analysis which follows, comparisons of the extended dissipation error with classical measures of the exact error in solution will be reported. The aim is to illustrate that the extended dissipation error reflects quite well the evolution of the admissible solution with respect to the exact one as described by more classical measures of the error. This section therefore presents the expressions of the classical measures of the error used for the subsequent numerical comparison and recalls, for the reader’s convenience, the expressions of the extended dissipation error and the error in solution.

**Classical measures of the exact error in solution**

As shown in the previous section, the extended dissipation error applied to the finite
Finite element solution and admissible solution for finite element mesh $\mathcal{T}_h$ constant in time.

element solution measures actually the accuracy of an admissible solution corresponding to the finite element solution. The admissible solution is a time dependent function which is obtained as time linear interpolation of the discrete values at the time instants used for the time discretization of the initial boundary value problem under consideration. Thus, it is this time dependent solution that will be assumed in the definition of the exact error. Furthermore, as generalization of the error in elasticity, it has been observed in Section 3.5.3 that the extended dissipation error can be interpreted as a global measure of the error of the kinematically admissible solution $s_{ad}^{kin} = (\epsilon_{ad}, p_{ad})$. Thus, it appears quite natural to consider the $L^\infty L^2$ norm of the exact error of the admissible total strain,

$$\|e_{ad}\|_{L^\infty([0,T];L^2(\Omega)^d \times d)} = \text{SUP}_{t \leq T} \|e_{ad}(x, t)\|_{(L^2(\Omega)^d \times d)} =$$

$$= \text{MAX}_{1 \leq n \leq N} \text{SUP}_{t \in [t_n, t_{n+1}]} \left\{ \int_{\Omega} (\epsilon_{ex}(x, t) - \epsilon_{ad}(x, t)) : (\epsilon_{ex}(x, t) - \epsilon_{ad}(x, t)) \, d\Omega \right\}^{\frac{1}{2}},$$

along with the $L^\infty$ control in time of the free complementary energy norm of the exact error of the generalised stress field conjugate of the kinematically admissible solution. That is, if we let

$$\|e_\sigma(x, t)\|_V^2 = \int_{\Omega} (\sigma_{ex}(x, t) - C\epsilon_{ad}(x, t)) : C^{-1}(\sigma_{ex}(x, t) - C\epsilon_{ad}(x, t)) \, d\Omega$$

$$\|e_R(x, t)\|_M^2 = \int_{\Omega} [R_{ex}(x, t) - H p_{ad}(x, t)] H^{-1} [R_{ex}(x, t) - H p_{ad}(x, t)] \, d\Omega$$

then, it follows

$$\|e_{GSF}\|_{L^\infty([0,T];V \times M)} = \text{SUP}_{t \leq T} \left\{ \|e_\sigma(x, t)\|_V^2 + \|e_R(x, t)\|_M^2 \right\}^{\frac{1}{2}}.$$
This choice is motivated by the following result

**Theorem 5.1.**

Given a kinematically admissible solution \( s_{ad}^{\text{kin}} = (e_{ad}, e_{ad}^p, p_{ad}) \),

\[
\|e\|_{L^\infty([0,T];L^2(\Omega)^{d \times d})} = 0 \quad \Rightarrow \quad \forall x \in \Omega \quad \forall t \leq T
\]

\[
\|e_{GSF}\|_{L^\infty([0,T];V \times M)} = 0 \quad \Rightarrow \quad \forall x \in \Omega \quad \forall t \leq T
\]

The proof follows by the same arguments as in the proof of Theorem 3.9, and will not be repeated here.

**Error in the constitutive equations**

This error measure is given by equation (3.46) which is here rewritten in a form that fits better the incremental origin of the admissible solution \( s_{ad} = (\sigma_{ad}, R_{ad}; e_{ad}, e_{ad}^p, p_{ad}) \).

\[
e_{ext}^2(T) =
\]

\[
= \max_{1 \leq n \leq N} \sup_{t \in [t_n, t_{n+1}]} \left\{ \int_{\Omega} (\sigma_{ad}(x, t) - C e_{ad}(x, t)) : C^{-1} (\sigma_{ad}(x, t) - C e_{ad}(x, t)) dx + \right\}
\]

\[
+ \int_{\Omega} \left[ R_{ad}(x, t) - H p_{ad}(x, t) \right] H^{-1} \left[ R_{ad}(x, t) - H p_{ad}(x, t) \right] dx + \right\}
\]

\[
+ \theta_{d}^2(t_n) + 2 \int_{t_n}^{t} \int_{\Omega} \left[ R_0 \| e_{ad}(x, \tau) \| - \sigma_{ad}(x, \tau) : \dot{e}_{ad}(x, \tau) + R_{ad}(x, \tau) \dot{p}_{ad}(x, \tau) \right] d\tau \right\},
\]

(5.3)

where

\[
\theta_{d}^2(t_n) = 2 \int_{0}^{t_n} \int_{\Omega} \left[ R_0 \| e_{ad}(x, \tau) \| - \sigma_{ad}(x, \tau) : \dot{e}_{ad}(x, \tau) + R_{ad}(x, \tau) \dot{p}_{ad}(x, \tau) \right] dx \right dt.
\]

Equation (5.3) basically expresses the global error at the time \( T \) in terms of the error at time \( t_n \) and of the admissible solution over \([t_n, T]\).

Since the admissible solution \( s_{ad}(x, t) \) is continuous piecewise linear over each time step, the time integral in the error expression can be computed easily. Hence,
we have for $t \in [t_n, t_{n+1}]$,

$$
\int_\Omega \int_{t_n}^t \left\{ R_0 \| e_{ad}^p(x, \tau) \| - \sigma_{ad}(x, \tau) : e_{ad}^p(x, \tau) + R_{ad}(x, \tau)p_{ad}(x, \tau) \right\} d\tau \, dx =
$$

$$
= \frac{t - t_n}{t_{n+1} - t_n} \int_\Omega \left\{ R_0 \| \Delta e_{ad}^p(x) \| - \frac{\sigma_{ad}(x, t_n) + \sigma_{ad}(x, t_{n+1})}{2} : \Delta e_{ad}^p(x) + \frac{R_{ad}(x, t_n) + R_{ad}(x, t_{n+1})}{2} \Delta p_{ad}(x) \right\} d\,dx
$$

where

$$
\Delta e_{ad}^p(x) = e_{ad}^p(x, t_{n+1}) - e_{ad}^p(x, t_n)
$$

$$
\Delta p_{ad}(x) = p_{ad}(x, t_{n+1}) - p_{ad}(x, t_n),
$$

whereas $\sigma_{ad}(x, t)$ and $R_{ad}(x, t)$ are the time linear interpolation over $[t_n, t_{n+1}]$ of the respective values at $t_n$ and $t_{n+1}$. Also, it follows

$$
\theta_d^2(t_n) = \sum_{i=1}^{n-1} 2 \int_\Omega \left\{ R_0 \| \Delta e_{ad,i}^p(x) \| - \frac{\sigma_{ad}(x, t_{i+1}) + \sigma_{ad}(x, t_i)}{2} : \Delta e_{ad,i}^p(x) + \frac{R_{ad}(x, t_{i+1}) + R_{ad}(x, t_i)}{2} \Delta p_{ad,i}(x) \right\} d\,dx
$$

with

$$
\Delta e_{ad,i}^p(x) = e_{ad}^p(x, t_{i+1}) - e_{ad}^p(x, t_i)
$$

$$
\Delta p_{ad,i}(x) = p_{ad}(x, t_{i+1}) - p_{ad}(x, t_i).
$$

The value of the dissipation error at $t_1 = 0$, $\theta_d^2(t_1)$, is assumed equal to zero.

**Error in solution**

This error measure is given by equation (3.73) which is the extended dissipation error associated with the admissible solution $s_{ex, ad} = (\sigma_{ex}, R_{ex}; e_{ad}, e_{ad}^p, p_{ad})$.

$$
e_{ex}^2(T) =
$$

$$
\text{MAX} \sum_{1 \leq n \leq N} \text{SUP} \left\{ \int_\Omega (\sigma_{ex}(x, t) - C e_{ad}^e(x, t)) : C^{-1} (\sigma_{ex}(x, t) - C e_{ad}^e(x, t)) \right\} d\,dx +
$$

$$
+ \int_\Omega [R_{ex}(x, t) - H p_{ad}(x, t)] H^{-1} [R_{ex}(x, t) - H p_{ad}(x, t)] \, dx + \theta_d^2 \, (t_n) + \tag{5.4}
$$

$$
+ 2 \int_\Omega \int_{t_n}^t \left[ R_0 \| e_{ad}^p(x, \tau) \| - \sigma_{ex}(x, \tau) : e_{ad}^p(x, \tau) + R_{ex}(x, \tau) p_{ad}(x, \tau) \right] d\tau \, dx
$$

140
where, as before, it is
\[ \theta^2_{d,ex}(t_n) = 2 \int_0^{t_n} \int_\Omega \left[ R_0 \| \hat{e}^p_{ad}(x, t) \| - \sigma_{ex}(x, t) : \hat{e}^p_{ad}(x, t) + R_{ex}(x, t) \hat{p}_{ad}(x, t) \right] dx \, dt. \]

Unlike the analogous expression (5.3), \( \sigma_{ex}(x, t) \) and \( R_{ex}(x, t) \), as functions of time, are not in general linear over each time step, therefore the time integral must be computed by accounting for the actual time variation of the functions.

For the ease of implementation, it may be useful rewriting the error in the evolution law as follows:

\[
\int_{t_n}^{t} \left[ R_0 \| \hat{e}^p_{ad}(x, \tau) \| - \sigma_{ex}(x, \tau) : \hat{e}^p_{ad}(x, \tau) + R_{ex}(x, \tau) \hat{p}_{ad}(x, \tau) \right] d\tau = \\
= \frac{t - t_n}{t_{n+1} - t_n} R_0 \| \Delta e^p_{ad} \| - \frac{1}{t_{n+1} - t_n} \left( \int_{t_n}^{t} \sigma_{ex}(x, \tau) d\tau \right) : \Delta e^p_{ad} + \\
+ \frac{1}{t_{n+1} - t_n} \left( \int_{t_n}^{t} R_{ex}(x, \tau) d\tau \right) \Delta p_{ad}.
\]

5.2.3 Numerical example

The general theory developed in the previous Sections of this Chapter is here applied to assess the quality of the finite element solution of a one dimensional elastoplastic bar under distributed axial loads. Despite the simplicity of the model, it allows one to emphasize the physical concepts of the theory and illustrate all the ingredients which characterize an error estimator of a finite element solution of elastoplastic problems (Orlando & Peric, 2000).

The model problem is shown in Figure 5.3 along with the variation of the external load multiplier. The bar is assumed to be composed of an elastoplastic material which obeys the Prandtl-Reuss plasticity law with linear hardening. The hardening law and elastic domain in the space of the generalized stresses are also depicted in Figure 5.3. For the problem at hand a closed form solution was not available, hence an ”overkill” procedure has been adopted, that is, the backward Euler finite element solution of a uniform mesh of 2000 elements with linear interpolation and time step \( dt = 0.025 \) has been used as an ”exact” solution.

The evolution of the state variables is reported in Figure 5.4 and Figure 5.5. Here, a steep gradient of the total strain is observed over the subdomain \( \Omega' = [0.725, 0.7275] \) starting at \( t = 16.375 \) which spreads over \([0.628, 0.73]\) as the load is increased (in absolute value). This is due to the fact that plastic strains of opposite sign are therein produced further to the sign reversing of the load. For the associative model under consideration, the plastic strain rate is, indeed, given by \( \dot{\epsilon}^p = \lambda \text{sign}(\sigma) \) where the plastic multiplier is nonnegative, i.e. \( \lambda \geq 0 \), thus if plastic loading occurs, the sign of \( \dot{\epsilon}^p \) is the same as \( \sigma \).
Table 5.1: Parts of the domain which experience plastic loading for the given loading history. "Exact" solution is obtained as finite element solution with 2000 elements and $dt = 0.025$
Figure 5.4: Exact Stress and Total Strain Distribution at each \( dt = 0.75 \) for \( t \in [0.0, 30.0] \)
Figure 5.5: Exact Plastic Strain and Accumulated Plastic Strain Distribution at each $dt = 0.75$ for $t \in [0.0, 30.0]$.
The Table 5.1 reports the parts $\Omega'$ of the domain $\Omega$ which experience plastic loading for the given loading history.

Before proceeding to the error analysis of several fully discrete schemes of the problem given in Figure 5.3, some considerations are due on the particularity of time and finite element discretization of 1D associative plasticity problems which will help to gain critical insight into the error behaviour.

As a result of finite element interpolation for the displacement field, the initial boundary value problem which governs the evolution of the elastoplastic continuum is transformed into a system of ordinary differential equations given in implicit form and algebraic constraints on the variables, which are usually called differential–algebraic equations (Brenan et al., 1996). The differential problem is stated over the time interval of interest and its unknowns are given by the nodal displacements.

The exact solution of this system of equations would allow one to build a displacement field which is affected by only space discretization error. Unfortunately, the resulting system is complex, for the main difficulty comes from the exact integration of the initial value constitutive problem which has been obtained only for very special conditions, such as in Krieg & Krieg (1977) and Ristinmaa & Tryding (1993). Consequently, as we have discussed in Section 4.3, one must generally resort to numerical integration algorithms, such as, the implicit backward Euler difference scheme. For an autonomous ordinary differential equation, for example,

$$\begin{align*}
    \dot{y} &= F(y) \\
    y(t_0) &= y_0
\end{align*}$$

the backward Euler would be displayed as in Figure 5.6(a) where the slope $\frac{y_{n+1} - y_n}{\Delta t}$

![Figure 5.6](image)

Figure 5.6: Backward Euler method graphically displayed: (a) For a general ordinary differential equation (b) For the initial value constitutive problem of 1D associative plasticity. The solution is exact at the time instants of the discretization

is given by the value of the function $F(y)$ at $y_{n+1}$. Also, note that the approximate solution does not lie in general on the exact curve.
In 1D associative plasticity, update stresses equations obtained from backward Euler integration scheme applied to the evolution equations are solved by a predictor-corrector algorithm which reads as in Box 5.2 (see, e.g., de Souza Neto et al., 2002), and has the geometrical interpretation given in Figure 5.7.

By inspection of the same Figure, it follows at once that the use of backward Euler will deliver the exact solution at the load levels that are considered. Under the assumption that the projection direction on the yield surface does not depend on the imposed total strain, which occurs surely in 1D plasticity and isotropic hardening, the application of the total strain increment $\Delta \epsilon$ or, for instance, of two increments $\Delta \epsilon^{(i)}$ such that $\Delta \epsilon = \Delta \epsilon^{(1)} + \Delta \epsilon^{(2)}$ delivers the same final state. This can also be read on the isoerror maps given in Krieg & Krieg (1977) and Schreyer et al. (1979). Finally, this means that time discretization effects for different values of the time step depend upon the difference in the time linear interpolations of the corresponding discrete values, Figure 5.6(b).

![Figure 5.7](image1)

Figure 5.7: Return mapping in 1D associative plasticity for $\Delta \epsilon$ and $\Delta \epsilon = \Delta \epsilon^{(1)} + \Delta \epsilon^{(2)}$

![Figure 5.8](image2)

Figure 5.8: Exact and finite element solutions for an elastic bar with constant properties. The finite element solutions are exact at the nodes of the mesh.

The one dimensionality of the problem has also particularities in terms of space discretization. For elastic behaviour, the orthogonality of the residual with respect to the finite element space and the local properties of the shape functions allows
one to show quite easily that the finite element solution is exact at the nodes if, for example, the material properties of the bar are uniform and the elements are linear. This result holds, however, also for more general conditions, for which we refer to Babuska & Strouboulis (2001). The approximation, therefore, arises in the difference between the displacement finite element interpolation within each element and the exact displacement field, Figure 5.8.

For an elastoplastic material, this is no more true. Nevertheless one can expect that the finite element solution at the nodes is close to the exact one.

Box 5.2. Fully implicit Elastic predictor/Return mapping algorithm for numerical integration of 1D associative plasticity constitutive equations. Isotropic hardening

<table>
<thead>
<tr>
<th>Data:</th>
<th>$e_p^n$, $p_n$</th>
</tr>
</thead>
<tbody>
<tr>
<td>Given:</td>
<td>$\epsilon_{n+1}$</td>
</tr>
<tr>
<td>Evaluate:</td>
<td>$Elastic\ Trial\ State$</td>
</tr>
<tr>
<td></td>
<td>$\epsilon_{n+1}^{p, tr} = \epsilon_p^n, \epsilon_{n+1}^{e, tr} = \epsilon_{n+1} - \epsilon_{n+1}^{p, tr}, p_{n+1}^{tr} = p_n$</td>
</tr>
<tr>
<td></td>
<td>$\sigma_{n+1}^{tr} = C \epsilon_{n+1}^{e, tr}, R_{n+1}^{tr} = g(p_{n+1}^{tr})$</td>
</tr>
<tr>
<td>Check:</td>
<td>IF $f_{n+1}^{tr} \overset{\text{def}}{=} f(\sigma_{n+1}^{tr}, R_{n+1}^{tr}) \leq 0$ THEN</td>
</tr>
<tr>
<td></td>
<td>set $(\bullet)<em>{n+1}^{tr} = (\bullet)</em>{n+1}^{tr}$</td>
</tr>
<tr>
<td></td>
<td>ELSE $\Rightarrow$ Return mapping</td>
</tr>
<tr>
<td>Return mapping:</td>
<td>Solve the system</td>
</tr>
<tr>
<td></td>
<td>$\epsilon_{n+1} - \epsilon_{n+1}^{e, tr} + \lambda_{n+1} \frac{\partial f}{\partial \sigma}(\sigma_{n+1}, R_{n+1}) = 0$</td>
</tr>
<tr>
<td></td>
<td>$p_{n+1} - p_{n+1}^{tr} - \lambda_{n+1} \frac{\partial f}{\partial R}(\sigma_{n+1}, R_{n+1}) = 0$</td>
</tr>
<tr>
<td></td>
<td>$\lambda_{n+1} &gt; 0, f_{n+1} = 0$</td>
</tr>
<tr>
<td></td>
<td>for $\lambda_{n+1}, \epsilon_{n+1}^{e}, p_{n+1}$, with</td>
</tr>
<tr>
<td></td>
<td>$\sigma_{n+1} = C \epsilon_{n+1}^{e}$, $R_{n+1} = g(p_{n+1})$</td>
</tr>
</tbody>
</table>

In the light of the aforementioned observations, we will consider fully discrete schemes where time steps and meshes are tailored ad hoc so that effects of space and time discretization can be relevant. This has been realized by requiring that there are different finite elements to yield in the model once the load level varies from 3.0 to 6.0 with step equal to 1.5. In this way, the response of the finite element model departs from the time linear interpolation of the solutions at $t = 3.0$ and $t = 6.0$. Therefore, an underlying non uniform mesh $me0$ has been first constructed with this criterion whereas the other meshes have been obtained by halving each element.
of the corresponding parent mesh. Also, uniform partition of the time interval of interest with time steps \( k = \Delta t = 6.0, 3.0, 1.5, 0.75 \) have been assumed. The fully discrete schemes are shown in Figure 5.9. These have been analysed using an incremental solution based on the backward Euler stepping scheme and the Newton-Raphson procedure. Only one Gauss point has been used for the integration of the constitutive equations, because linear finite elements are used. Plastic strain \( h\epsilon_{p,n+1} \) and accumulated plastic strain \( h\sigma_{p,n+1} \), obtained at the single quadrature point of each element, are prolongated into uniform field over the respective element.

For the definition of the corresponding admissible solution necessary to compute the extended dissipation error the criteria described in the previous Section apply. In particular, if the 1D system is statically indeterminate, the statically admissible stress field \( \sigma_{ad}(x,t_{n+1}) \) is obtained by solving over each element \( \Omega^h \in T_h \) the following equilibrium problem,

\[
\forall \Omega^h = \mid x^h, x^{h+1} \mid \in T_h
\]

\[
- \frac{d}{dx} \sigma_{ad}^e(x,t_{n+1}) + f(x,t_{n+1}) = 0
\]

\[
\sigma_{ad}^e(x^h,t_{n+1}) = - \int_{x^h}^{x^{h+1}} h\sigma(x) \frac{dN^h}{dx}(x) \, dx + \int_{x^h}^{x^{h+1}} f(x,t_{n+1})N^h(x) \, dx
\]

where the boundary condition is obtained from the prolongation condition (cf. Ladevèze & Leguillon, 1983)

\[
\int_{x^h}^{x^{h+1}} \left( \sigma_{ad}^e(x,t_{n+1}) - h\sigma(x) \right) v(x) \, dx \quad \forall v \in V^h(\Omega^h)
\]

with \( V^h(\Omega^h) \) being the finite dimensional space generated by the element Lagrangian shape functions \( N^h(x) \) and \( N_{e+1}^h(x) \) depicted in Figure 5.10.
Remark 5.2. It is an easy matter to check that condition (5.6), albeit local, is consistent with the continuity of $\sigma_{\text{ad}}(x, t_{n+1})$ across the nodes, i.e.,

$$\sigma^{-1}_{\text{ad}}(x_e, t_{n+1}) = \sigma_{\text{ad}}(x_e, t_{n+1})$$

and also, that the following equilibration condition

$$\sigma_{\text{ad}}(x_e, t_{n+1}) - \sigma_{\text{ad}}(x_{e+1}, t_{n+1}) + \int_{x_h^e}^{x_{h_{e+1}}} f(x, t_{n+1}) dx = 0$$

is satisfied. Furthermore, it is possible to show that condition (5.6) corresponds to a splitting assumption for the singular component of the residual in the equilibrium equation associated with the finite element stress, $^h\sigma$, and given by $^h\sigma^{e}(x_{e+1}, t_{n+1}) - ^h\sigma^{e-1}(x_{e}, t_{n+1})$, if a distribution assumption, over the element, of the finite element stresses computed at the quadrature points has also been respected.

The admissible thermodynamic force $R_{\text{ad}}(x, t_{n+1})$ is assumed as

$$R_{\text{ad}}(x, t_{n+1}) = \text{Max} \{ R_1, R_2 \}$$

where

$$R_1 = \| \sigma_{\text{ad}}^D(x, t_{n+1}) \| - R_0$$

and $R_2 = R_{\text{ad}}(x, t_n)$. The admissible total strain, on the other hand, will be given by

$$\epsilon_{\text{ad}}(x, t_{n+1}) = \epsilon^h(x, t_{n+1}),$$

whereas the admissible plastic strain is obtained as

$$\epsilon^p_{\text{ad}}(x, t_{n+1}) = ^h\epsilon^p_{n+1}(x),$$

if the following condition is satisfied

$$\sigma_{\text{ad}}(x, t_{n+1}) : [^h\epsilon^p_{n+1}(x) - \epsilon^p_{\text{ad}}(x, t_{n})] \geq 0, \quad (5.7)$$

otherwise we choose

$$\epsilon^p_{\text{ad}}(x, t_{n+1}) = \epsilon^p_{\text{ad}}(x, t_n).$$
Finally, with regard to the admissible accumulated plastic strain \( p_{ad}(x, t_{n+1}) \) it is
\[
p_{ad}(x, t_{n+1}) = p_{ad}(x, t_n) + \| \epsilon_{p}^n(x, t_{n+1}) - \epsilon_{p}^n(x, t_n) \|
\]
which corresponds to the integration of the equation \( \dot{p} = \| \epsilon_{p}^n \| \) that occurs for the model under consideration by assuming linear variation of the variables over \([t_n, t_{n+1}]\).

For the computation of the error expressions given in Section 5.2.2, the supremum over the generic time interval \([t_n, t_{n+1}]\) is computed as maximum of a discrete set given by the value of the functions sampled at the time \( t_i \in [t_n, t_{n+1}] \) where the "exact" solution is known, whereas the space integrals have been computed with Gauss quadrature by using 12 quadrature points. This does not incur a supplementary computational effort, for only sampling of functions is involved.

The extended dissipation error computed at each time \( t_i \), as given by equation (5.3), and the error in solution, as given by equation (5.4), have been used to define the effectivity index
\[
\xi(t) = \frac{\epsilon_{ext}(t)}{\epsilon_{ex}(t)},
\]
likewise for elliptic problems (Babuska & Rheinboldt, 1978b). The time evolution of \( \xi(t) \) is shown in Figure 5.11 for the fully discrete schemes analysed. For all the computations, the effectivity index was ranging between 1.00 to 1.45. As a result,

![Figure 5.11: Evolution in time of the effectivity index for different fully discrete schemes](image-url)
the extended dissipation error $e_{\text{ext}}(t)$ can be used as a reliable estimate of the error in solution $e_{\text{ext}}(t)$. Furthermore, it is noted that $\xi(t)$ does not approach to 1 as the discretization becomes finer. The condition $\xi \to 1$ defines the asymptotic exactness of the error estimator. This is a desirable property, though not strictly necessary for the reliability and efficiency of the estimator which results from the boundness of the effectivity index from above and from below ($\geq 1$), respectively. However, the asymptotic properties of the extended dissipation error will be analysed numerically in the next section. There, it will be shown that $e_{\text{ext}}(t) \to 0$ as $h, \Delta t \to 0$ and, consequently, the approximate solution will converge to the exact solution. Therefore, at this stage, the default of the extended dissipation error of being an asymptotically exact estimator of the error in solution is not of concern and deserves certainly major investigation in the future. Also, one must be aware that, for instance, for elliptic problems, the asymptotic exactness of an error estimator is a very fragile property (Verfürth, 1996) that depends on or requires, amongst other things, very regular meshes and a fairly smooth solution. In fact, proofs of asymptotic exactness of some estimators given in literature are all based on the validity of superconvergence results (Babuska & Rheinboldt, 1979a, 1981; Duran & Rodríguez, 1992; Babuska & Rodríguez, 1993; Verfürth, 1996). Therefore, the importance of this property does not have to be over-emphasized (Ainsworth & Oden, 1997) and certainly it is not expected to hold for the general types of meshes used in the practical engineering computations (Babuska et al., 1994a) and a fortiori for non linear problems.

A typical time evolution of the extended dissipation error $e_{\text{ext}}(t) = \sup_{\tau \leq t} \theta(\tau)$ for a fully discrete finite element model is given in Figure 5.12. In this picture, also the current value of the components, $\theta(t) = \sqrt{\theta_{\text{al}}^2(t) + \theta_{\text{ad}}^2(t)}$ with $\theta_{\text{al}}(t) = \sqrt{\theta_{\text{al}}^2(t) + \theta_{\text{ad}}^2(t)}$, defined in equation (5.3), are reported. By definition, $e_{\text{ext}}(t)$ is a nondecreasing function of time, which allows one to assess the quality of the approximate solution over the whole interval of interest. The same monotone character is presented also by the current value of the error component in the evolution law, $\theta_{\text{al}}(t)$, which reflects the irreversible phenomena associated with the admissible solution up to the current time $t$. On the contrary, the error in the state law, $\theta_{\text{sl}}(t)$, which is defined in terms of the free Helmholtz energy and its conjugate, will depend on the energy associated with the current approximate solution. Thus its time variation will depend on the behaviour of the current solution. Figure 5.13(a) and Figure 5.13(b) display the sources of the error in the elastic law at $t = 28.5$.

In particular, because of the linearity of the elastic law and definition of the statically admissible stress field as in (5.5), for the problem at hand, it follows that

$$\theta_{\text{al}}^2(t) = \sum_e \frac{1}{C} \int_{x^e_h}^{x^e_{h+1}} \left| \sigma_{\text{ad}}^e(x,t) - C \epsilon_{\text{ad}}^e(x,t) \right|^2 \, dx =$$

$$= \mu^2(t) \sum_e \frac{1}{C} \int_{x^e_h}^{x^e_{h+1}} \left[ \int_{x^e_h}^{x^e_{h+1}} f(\xi) \nabla_{\xi} N^h_e(\xi) d\xi - \int_{x^e_h}^{x^e_{h+1}} f(\xi) d\xi \right]^2 \, dx,$$

which shows that the time variation of $\theta_{\text{al}}^2(t)$ is the same as the variation of the exterior...
nal load, even in presence of plastic loading. The previous result is obtained further to the assumption that $\epsilon^{p,e}_{ad}(x, t) = h \epsilon^{p}(x_{e,GP}, t)$, thus $C_{ad}^{p,e}(x, t) = h \sigma(x_{e,GP}, t)$, where only one Gauss point per element is used for the numerical integration of the constitutive equations.

With regard, then, to the time variation of $\theta_{sl}^p(t)$, changes of $\theta_{sl}^p$ are noted if plastic loading occurs otherwise $\theta_{sl}^p$ remains constant if the admissible thermodynamic force $R_{ad}$, and the thermodynamic force conjugate to the admissible accumulated plastic strain, $H_{P_{ad}}$, do not vary, such as during elastic unloading. Also, when plastic deformations should occur in the model, as identified by value different from zero of $R_{ad}$, but they are not detected by the admissible kinematic solution, $p_{ad}$, a contribution different from zero to $\theta_{sl}^p$ comes from these parts of the domain $\Omega$. This circumstance is shown in Figure 5.13(c) and Figure 5.13(d) which report, for instance, the pointwise contribution to the error in the hardening law at $t = 28.5$.

Even though the admissible accumulated plastic strain is zero all over the element $\epsilon l = 4$, a contribution different from zero to the error associated with the plastic energy is obtained from this element for being, therein, the thermodynamically admissible force different from zero. This may be interpreted as the plastic energy that we have somehow to supply to this part of the domain, since the kinematically admissible solution was not able to describe it.

Major critical insight in the causes that produce variation of the error in the evolution law, $\theta_d$, is obtained by analysing the pointwise contribution to $\theta_d$ within a given time interval $[t_n, t_{n+1}]$,

$$\left[ t_n, t_{n+1} \right] \Theta_{d}^2(\mathbf{x}) \stackrel{\text{def}}{=} \int_{t_n}^{t_{n+1}} \left[ R_0 \| \dot{\epsilon}_{ad}^p(\mathbf{x}, t) \| - \sigma_{ad}(\mathbf{x}, t) \cdot \dot{\epsilon}_{ad}^p(\mathbf{x}, t) + R_{ad}(\mathbf{x}, t)p_{ad}(\mathbf{x}, t) \right] dt$$

Figure 5.12: Time variation of the extended dissipation error and its components
Figure 5.13: The error in the state law at \( t=28.5 \) (a) Admissible stress \( \sigma_{ad} \) versus stress conjugate to the admissible elastic strain \( C\epsilon_{ad} \). (b) Pointwise contribution to the error in the elastic law, \( \frac{1}{C}(\sigma_{ad} - C\epsilon_{ad})^2 \), at \( t=28.5 \) (c) Admissible thermodynamic force \( R_{ad} \) versus force conjugate of the admissible accumulated plastic strain \( H_{pa} \). (d) Pointwise contribution to the error in the hardening law, \( \frac{1}{H}(R_{ad} - H_{pa})^2 \), at \( t=28.5 \)

which is next considered in its general tensorial notation.

By accounting for the definition of the admissible plastic strain and accumulated plastic strain, the assumption of time linear variation for the admissible solution over the time interval \([t_n, t_{n+1}]\), it follows that

\[
[t_n, t_{n+1}] \zeta_{D}^{2}(x) = \frac{\|\Delta \epsilon_{ad}^p\|}{2} \left[ R_0 + R_{ad}(x, t_{n+1}) - \|\sigma_{ad}(x, t_{n+1})\| n_{\sigma_{ad}}(x, t_{n+1}) : n_{\Delta \epsilon_{ad}^p}(x) \right] + \\
+ \frac{\|\Delta \epsilon_{ad}^p\|}{2} \left[ R_0 + R_{ad}(x, t_{n}) - \|\sigma_{ad}(x, t_{n})\| n_{\sigma_{ad}}(x, t_{n}) : n_{\Delta \epsilon_{ad}^p}(x) \right],
\]

where we have let

\[
\sigma_{ad}(x, t) = \|\sigma_{ad}(x, t)\| n_{\sigma_{ad}}(x, t), \quad \Delta \epsilon_{ad}^p(x) = \|\Delta \epsilon_{ad}^p(x)\| n_{\Delta \epsilon_{ad}^p}(x),
\]

and it is, in general, \( R_0 + R_{ad}(x, t) - \|\sigma_{ad}(x, t)\| \geq 0 \).
As a result, if at the point \( x \in \Omega \) the following expressions are valid

\[
\begin{align*}
n_{\sigma_{ad}^D}(x, t_n) \cdot n_{\Delta e^p_{ad}}(x) &= +1 \\
R_0 + R_{ad}(x, t_n) - \|\sigma_{ad}^D(x, t_n)\| &= 0, \\
n_{\sigma_{ad}^D}(x, t_{n+1}) \cdot n_{\Delta e^p_{ad}}(x) &= +1 \\
R_0 + R_{ad}(x, t_{n+1}) - \|\sigma_{ad}^D(x, t_{n+1})\| &= 0,
\end{align*}
\]

then there is no local contribution to the error in the evolution law from the current time interval. This, in turn, means that given

\[
\begin{align*}
\sigma_{ad}^D(x, t) &= \frac{t - t_n}{t_{n+1} - t_n} \sigma_{ad}^D(x, t_{n+1}) + \frac{t_{n+1} - t}{t_{n+1} - t_n} \sigma_{ad}^D(x, t_n) \\
R_{ad}(x, t) &= \frac{t - t_n}{t_{n+1} - t_n} R_{ad}(x, t_{n+1}) + \frac{t_{n+1} - t}{t_{n+1} - t_n} R_{ad}(x, t_n) \\
e^p_{ad}(x, t) &= \frac{t - t_n}{t_{n+1} - t_n} e^p_{ad}(x, t_{n+1}) + \frac{t_{n+1} - t}{t_{n+1} - t_n} e^p_{ad}(x, t_n) \\
p_{ad}(x, t) &= \frac{t - t_n}{t_{n+1} - t_n} p_{ad}(x, t_{n+1}) + \frac{t_{n+1} - t}{t_{n+1} - t_n} p_{ad}(x, t_n)
\end{align*}
\] (5.11)

with

\[
\begin{align*}
\|\sigma_{ad}^D(x, t_n)\| - (R_0 + R_{ad}(x, t_n)) &\leq 0 \\
\|\sigma_{ad}^D(x, t_{n+1})\| - (R_0 + R_{ad}(x, t_{n+1})) &\leq 0 \\
\text{Tr}[\Delta e^p_{ad}] &= 0 \quad \text{(5.12)} \\
\Delta p_{ad} &= \|\Delta e^p_{ad}\| \\
n_{\sigma_{ad}^D}(x, t) \cdot n_{\Delta e^p_{ad}}(x) &= +1, \quad \forall t \in [t_n, t_{n+1}]
\end{align*}
\]

then, it follows that

\[
\forall t \in [t_n, t_{n+1}],
\begin{align*}
\|\sigma_{ad}^D(x, t)\| - \left[ R_0 + R_{ad}(x, t) \right] &\leq 0 \\
\|e^p_{ad}(x, t)\| - \hat{p}_{ad}(x, t) &\leq 0 \\
\text{Tr}[\hat{e}^p_{ad}(x, t)] &= 0 \\
R_0 \|\hat{e}^p_{ad}(x, t)\| - \sigma_{ad}(x, t) : \hat{e}^p_{ad}(x, t) + R_{ad}(x, t) \hat{p}_{ad}(x, t) &= 0.
\end{align*}
\]

That is, the function \((\sigma_{ad}(x, t), R_{ad}(x, t), e^p_{ad}(x, t), p_{ad}(x, t))\) defined as in (5.11) upon the conditions (5.12) is an integral of the evolution law over the time interval \([t_n, t_{n+1}]\) (see Section 3.2.4.5). The difference from the exact solution, however, lies in the diversity of initial condition met at \(t_n\). These observations are finally visualized in Figure 5.14 which show the pointwise contribution to \(\theta_d\) within the
Figure 5.14: (a) Variation of admissible plastic strain between $t = 18.0$ and $t = 19.5$ (b) Space distribution of $f(\sigma_{ad}, R_{ad})$ at $t = 18.0$ and $t = 19.5$ (c) Pointwise contribution to the increment of the error in the evolution law.
time interval [18.0, 19.5]. In conclusion, \([t_n,t_{n+1}]\) \(\zeta_{\text{sl}}(x)\) can be equal to zero without implying that the state variables do not change between \(t_n\) and \(t_{n+1}\). Their variation is, in fact, detected by the variation of the current error in the state law.

### 5.2.4 Analysis of the error

In this Section the behaviour of the extended dissipation error with respect to the parameters that control the approximation, namely time step size and mesh size, is investigated numerically. We start by analysing the relative importance of the error components \(\theta_{\text{sl}}\) and \(\theta_d\) with respect to the global error \(e_{\text{ext}}\).

Figure 5.15 describes the influence of the time discretization for a given finite
Figure 5.16: Extended dissipation error along with its components for different discretizations.

Evolution of the error in the state law and in the evolution law with respect to the global error as the finite element mesh is refined.

In general, one notes a reduction of the absolute error $e_{ext}$ along with the variation of the relative importance of its components. In particular, Figure 5.15 shows that by reducing the time step, the variation of $\theta_{sl}(t)$ remains almost unchanged whereas $\theta_d(t)$ reduces, so that $\theta_{sl}$ represents the main error component. This behaviour is easier to comprehend if we consider the time step contribution to the error in the evolution law given by

$$\int_{\Omega} |\tilde{u}_{n+1} - \tilde{u}_n| \, dx.$$
Figure 5.17: Contribution to the error in the evolution law for given finite element mesh, \( me1 \), and different time discretizations, time step \( \Delta t = 1.5 \) and \( \Delta t = 0.75 \) (a) Time step contribution to the error in the evolution law (b) Time variation for the extended dissipation error along with its components

Figure 5.18: Effect of the time step on the error in the evolution law for different time discretizations \( \Delta t, \Delta t', \Delta t'' \), with \( \Delta t = 2\Delta t' \) and \( \Delta t' = 2\Delta t'' \)
respectively. For the problem at hand, at a given point \( x \in \Omega \), the error in the evolution law results from assuming a linear variation for the variables \( \epsilon^p, p^a \) over the whole time interval \([t_n, t_{n+1}]\), whereas the time \( t_{c} \), which is the time when the exact plastic loading occurs, belongs to \([t_n, t_{n+1}]\). As a result, the exact linear variation of \( \epsilon^p \) and \( p \) occurs only over \([t_{c}, t_{n+1}]\).

The reduction of the time step reduces the time interval during which the plastic flow contributes to the error because of the linear interpolation. This situation is sketched in Figure 5.18.

Figure 5.16, as well, shows that by refining the finite element mesh size the error component \( \theta_{sl}(t) \) reduces drastically so that \( \theta_{d} \) constitutes the main error component. The dependence of \( \theta_{sl} \) on the mesh size \( h \), and more specifically of \( \theta'_{sl} \) on \( h \), can also be revealed by a direct analysis of the equation (5.9). Figure 5.16 also shows that the dependence of \( \theta_{d} \) on the mesh size is not well defined as it can be inferred from comparing the variation of \( \theta_{d} \) for the meshes \( me0 \) and \( me1 \). Here, it is noted that \( \theta_{d} \) increases by refining the mesh size. This behaviour is visualized in Figure 5.19 which compares the time step contribution to the global error in the evolution law for the previous schemes. This circumstance occurs as a result of a larger area experiencing plastic loading detected by the mesh \( me1 \) with the elements \( el = 7 \) and \( el = 8 \), as shown in Figure 5.20.

Another aspect taken into account in this section is to motivate the use of the extended dissipation error by showing that this measure of the error does indeed reflect the global quality of an admissible solution. This is proved numerically on the model problem under consideration by investigating the behaviour of the family of finite element approximations corresponding to values of the discretization parameters approaching their limit values, that is,

\[
\begin{align*}
e_{ext} & \to 0 \quad \text{as } h, \Delta t \to 0 \\
e_{ext} & \to e_{ext,\Delta t} \quad \text{as } h \to 0 \\
e_{ext} & \to e_{ext,h} \quad \text{as } \Delta t \to 0
\end{align*}
\]

(5.13)

where \( e_{ext,\Delta t} \) and \( e_{ext,h} \) denote the error due to only time and space discretization, respectively. That is, \( e_{ext,\Delta t} \) is defined as the error associated with the exact solution of the nonlinear incremental boundary value problem obtained by performing only the time discretization of the initial boundary value problem; whereas, \( e_{ext,h} \) is the error associated with the exact solution of the system of differential algebraic equations obtained by performing only the space finite element discretization of the initial boundary value problem.

We also compare the time evolution of the extended dissipation error (which can be thought of as an estimate of the error of the kinematically admissible variables) with the classical measures of the exact error defined as difference between exact and approximate solution and introduced in Section 5.2.2. The aim of this comparison is to show that the extended dissipation error describes quite well the evolution of the approximate solution compared to the exact one. That is, the occurrence of (5.13) corresponds effectively to have the approximate solution approaching
Figure 5.19: Contribution to the error in the evolution law for given time discretization, time step $\Delta t = 1.5$, and different finite element mesh, $me0$ and $me1$ (a) Time step contribution to the error in the evolution law (b) Time variation for the extended dissipation error along with its components
Figure 5.20: (a) Admissible plastic strain distribution for mesh me0 and time step $\Delta t = 1.5$ (b) Space distribution of $f(\sigma_{ad}, R_{ad})$ for mesh me0 and time step $\Delta t = 1.5$ (c) Admissible plastic strain distribution for mesh me1 and time step $\Delta t = 1.5$ (d) Space distribution of $f(\sigma_{ad}, R_{ad})$ for mesh me1 and time step $\Delta t = 1.5$
to the exact one, to the solution of the time discrete scheme and of the space discrete scheme, respectively.

The results of these studies are delivered in Figures 5.21 and 5.22. In particular, Figure 5.21 shows the effects of the space discretization error by comparing fully discrete schemes which present the same uniform time discretization defined by $\Delta t = 1.5$ and the finite element meshes depicted in Figure 5.9. The asymptotic behaviour of the error measures, however, is better appreciated by considering the variation for fixed values of the time. These diagrams are reported in the same figure and describe the variation of the error at $t = 1.5$, $t = 6$, $t = 24.0$ and $t = 30.0$.

![Extended Dissipation Error](image1)

**Figure 5.21:** Extended dissipation error and classical measures of the exact error. Effect of space discretization for given time discretization. Time step $\Delta t = 0.75$.

At $t = 1.5$, the behaviour of the bar is elastic, consequently the extended dissipation error along with the other measures of the error, which involve gradient of the
displacement, exhibit a linear rate of convergence which is the type of convergence
of the error in energy norm for linear elements, (see, e.g., Babuska & Rheinboldt,
1978b) or (Ciarlet, 1978). In fact, for elastic behaviour it is
\[ \sigma_{ad}(x, t) = \mu(t) \sigma_{ad}(x); \quad \epsilon_{ad}(x, t) = \epsilon_{ad}(x, t) = \mu(t) \epsilon_{ad}(x) \]
whereas the other state variables vanish. Thus equation (5.3) reduces to the same
expression obtained in linear elasticity
\[ e^2_{ext}(T) = \sup_{t \leq T} \int_{\Omega} \left( \sigma_{ad}(x) - C \epsilon_{ad}(x) \right) : C^{-1} \left( \sigma_{ad}(x) - C \epsilon_{ad}(x) \right) \, dx \quad (5.14) \]
which is the norm of the error induced by the elastic energy once one normalizes the
error with respect to the load multiplier.

At the other time instants \( t_n \) plastic deformations occur in the bar. Hence, in
this case, it is interesting to note that the functions \( e_{ext}(t_n, h), \| e_{ad} \|_{L^\infty([0, t_n]; L^2(\Omega))}(h) \) and \( \| e_{GSF} \|_{L^\infty([0, t_n]; V \times M)}(h) \) take values different from zero for \( h \to 0 \). This is due
the presence of the time discretization error. Hence, only enrichment of the finite
dimensional space, without also refining the discretization in time, may not improve
the accuracy of the numerical solution.

Finally, the diagrams given in Figure 5.22 aim to highlight the effect of the
space discretization error. To this end, numerical simulations have been carried out
on given finite element mesh whereas the time step size \( \Delta t \) was changed. In general
a reduction of the extended dissipation error is observed as the time step size \( \Delta t \)
is reduced, though the reduction is not as pronounced as the one obtained by the
enrichment of the mesh. Also here, the error presents a value different from zero as
\( \Delta t \to 0 \), due to the influence of the space discretization error.

As for the time variation of the classical measures of the exact error, as a
result of the little influence of the time discretization, it is interesting to note that,
for example, the diagram of the \( L^2 \) of the exact error of the total strain shows
that starting from \( t \geq 21 \) we have the same time evolution of the error using different
time discretizations. This behaviour is to be related to the one dimensionality of the
model problem under consideration. Even considering other time discretizations, the
elements of the discrete model experiencing plastic loading are the same for \( t \geq 21 \).
Hence, since at the same load level, we have the same system state, it follows that
the variation of state from one time instant to the other is the same.

Figure 5.22 reports as well the variation for the different time discretizations
of the error computed at the time instants \( t = 1.5, t = 6.0 \) and \( t = 21.0 \). These
diagrams, likewise the previous one, allow one to appreciate better the asymptotic
behaviour of the error with respect to \( \Delta t \). At \( t = 1.5 \) a constant value is obtained
for all the error measures. This is due to the fact that the behaviour of the discrete
model is elastic, hence no time discretization error is introduced since the elastic
constitutive equation is integrated exactly. The plots relative to \( t = 6.0 \) and \( t = 21.0 \),
on the other hand, show that reducing the time step size beyond a certain limit value
has no effect on the error, hence no benefit can be expected on the improvement of
Figure 5.22: Extended dissipation error and classical measures of the exact error. Effect of time discretization for given finite element mesh. Mesh with 56 elements.

the accuracy of the solution, which relates to the error due to the discretization in space.

Remark 5.3. In Orlando & Peric (2000) by generalizing the error in the constitutive equations for linear elasticity, the extended dissipation error has been interpreted as an estimate of the error of the kinematically admissible variables. For linear elastic behaviour of the discrete model, we have seen that the extended dissipation error reduces to the energy norm of the error in the displacement which shows a linear convergence rate with the mesh size. When plastic loading occurs, several rate of convergence for the state variables are noted, (see, e.g., Johnson, 1977; Han & Reddy, 1999). Since the extended dissipation error account for the error in all
the variables, its rate of convergence should be influenced by the lower one.

5.3 Dissipation Error

The dissipation error has been defined in Section 3.5.1.1 as the error in the constitutive equations produced by a field $s_{ad}(x, t) = (\sigma_{ad}(x, t), X_{ad}(x, t), R_{ad}(x, t); u_{ad}(x, t), \epsilon_{ad}(x, t), \epsilon^p_{ad}(x, t), \alpha_{ad}(x, t), p_{ad}(x, t))$ which meets the compatibility equations, the equilibrium equations, the initial conditions and the state law. In order to apply this theory for the assessment of the quality of a finite element solution, the general considerations given in Section 5.2 for the use of the extended dissipation error apply. The solution of a fully implicit conforming finite element displacement formulation of the elasto-plastic problem, to which we refer in the following, is in general not admissible, for the finite element stresses do not satisfy the equilibrium equations in a pointwise manner. As a result, to evaluate the dissipation error as indication of the error of the finite element solution, one needs first to build an admissible solution which is as close as possible to the computed finite element solution. This will be the object of the first part of this section where such criteria are given for the Prandtl-Reuss model with linear elasticity and linear hardening following the works of Moës (1996) and Ladevèze & Pelle (2001). In the second part, after recalling the expressions of the dissipation error and of the error in solution, numerical comparisons of this error with the extended dissipation error will be illustrated on a 1D model problem.

5.3.1 Construction of the admissible solution

The Prandtl Reuss plasticity model with Linear Hardening

The equations for this model have been recalled in Section 5.2.1. Likewise the extended dissipation error, equation (3.45) expresses the dissipation error at the time $t_{n+1}$ in terms of its value at $t_n$ and of the admissible solution over $[t_n, t_{n+1}]$. As a result, the admissible solution can be obtained in an incremental manner.

Consistently with an assumed linear variation of the external load over each time interval and a normal formulation of the model as introduced in Ladevèze (1989), the admissible solution can be taken to vary linearly over $[t_n, t_{n+1}]$ so that for its complete definition one needs to compute only the value at $t_{n+1}$.

The criteria to build a statically admissible stress field in terms of the computed finite element stresses are the same as the one presented for the extended dissipation error. These criteria, in fact, are not dependent on the constitutive model.

The definition of an admissible displacement field, on the other hand, requires some further consideration because of the constraint imposed by the Hooke’s law,

$$\sigma_{ad}(x, t_{n+1}) = Ce_{ad}(x, t_{n+1}).$$

In fact, for a plasticity model which does not conserve volume, one can simply assume $u_{ad}(x, t_{n+1}) = u_h(x, t_{n+1})$. The associated admissible plastic strain field
would then be given by

$$\epsilon_{ad}^p(x, t_{n+1}) = \nabla_s u_{n+1}^h(x) - C^{-1}\sigma_{ad}(x, t_{n+1}).$$  \hfill (5.15)

In the case of a $J_2$-flow theory, such as for the Prandtl-Reuss model under consideration, on the contrary, the choice $u_{ad}(x, t_{n+1}) = u_{n+1}^h(x)$ is not always possible. The plastic strain given by (5.15) may not meet the incompressibility condition

$$\text{Tr}\left[\nabla_s u_{n+1}^h(x) - C^{-1}\sigma_{ad}(x, t_{n+1}) - \epsilon_{ad}^p(x, t_n)\right] = 0$$  \hfill (5.16)

and, consequently, the dissipation error would not assume a finite value. In such a case, then, one needs to define $u_{ad}(x, t_{n+1})$ so that condition (5.16) is met.

A procedure to build such an admissible displacement field is given in Moës (1996) who adapts the method proposed in Gastine et al. (1992) for incompressible elasticity. However, for 1D problems and plane stress state problems, one can assume without restriction $u_{ad}(x, t_{n+1}) = u_{n+1}^h(x)$ and adjust the transversal component of the plastic strain to realize the incompressibility condition.

As for the definition of the admissible accumulated plastic strain $p_{ad}(x, t_{n+1})$, we can follow the general procedure indicated in Section 5.2.1. This determines $p_{ad}(x, t_{n+1})$ as minimizer of the pointwise contribution to the dissipation error within the time step $[t_n, t_{n+1}]$ and under the further constraint $R_{ad}(x, t_{n+1}) = H_{p_{ad}}(x, t_{n+1})$, which imposes the respect of the hardening law.

The general problem given in Box 5.1 would then specialize as in Box 5.3.

It is trivial to check that the exact solution of this problem is given by

$$p_{ad}(t_{n+1}) = \max \left\{ \|\sigma_{ad}^p(t_{n+1})\| - R_0; \ p_{ad}(t_n) + \|\epsilon_{ad}^p(t_{n+1}) - \epsilon_{ad}^p(t_n)\| \right\}.$$  

Once all the admissible state variables have been computed at $t_{n+1}$, the use of a time linear interpolation over $[t_n, t_{n+1}]$ guarantees the admissibility of the solution for the convexity of the equilibrium and compatibility conditions and for the linearity of the state law as a result of having expressed the model into normal form. Yet, the above procedure delivers an admissible solution which produces a finite value of the error for the convexity of the domains $E$ and $C$ introduced in Section 3.2.4.5.

### 5.3.2 Error Expressions

**Error in the constitutive equations**

In the previous section we have seen that the admissible solution for the computation of the dissipation error, unlike the one used for the definition of the extended dissipation error, is required to meet the state laws. As a result of this constraint between the admissible static variables and their conjugate kinematic one, the residual in the state law vanishes and the accuracy of the admissible solution is defined only in terms of the residual produced in the evolution equations, which are the only equations of the model not to be satisfied. The error in the constitutive equations
is, therefore, given by
\[
e^{-\text{dis}(t_{n+1})} = e^{-\text{dis}(t_n)} +
\]
\[
+ 2 \int_{t_n}^{t_{n+1}} \left[ R_0 \| \dot{\epsilon}_p^p(x, t) \| - \sigma_{ad}(x, t) : \dot{\epsilon}_p^p(x, t) + R_{ad}(x, t) \dot{p}_{ad}(x, t) \right] dt d\Omega
\]

where

**Box 5.3.** Definition of the admissible accumulated plastic strain as minimization of the dissipation error

For each \( x \in \Omega \)

Given:
\( \sigma_{ad}(x, t_n), \sigma_{ad}(x, t_{n+1}), \)
\( \epsilon_{ad}(x, t_n), \epsilon_{ad}(x, t_{n+1}), \)
\( p_{ad}(x, t_n), R_{ad}(x, t_n) = H_{p_{ad}}(x, t_n) \)

Find:
\( p_{ad}(x, t_{n+1}) \)

such that by assuming a time linear variation over \([t_n, t_{n+1}]\) of the variables
\( \sigma_{ad}(x, t), R_{ad}(x, t); \)
\( \epsilon_{ad}(x, t), p_{ad}(x, t), \)

we realize the minimum of the following function
\[
F(p_{ad}(x, t_{n+1})) =
\]
\[
= \int_{t_n}^{t_{n+1}} \left[ R_0 \| \dot{\epsilon}_p^p(x, t) \| - \sigma_{ad}(x, t) : \dot{\epsilon}_p^p(x, t) + R_{ad}(x, t) \dot{p}_{ad}(x, t) \right] dt
\]

under the following constraints

\( \forall t \in [t_n, t_{n+1}] \)
\( \| \sigma_{ad}^D(x, t) \| - (R_{ad}(x, t) + R_0) \leq 0, \)
\( \dot{p}_{ad}(x, t) \geq \| \dot{\epsilon}_p^p(x, t) \|, \)
\( \text{Tr}[\dot{\epsilon}_p^p(x, t)] = 0 \)
\( R_{ad}(x, t) = H_{p_{ad}}(x, t) \)

\[
e^{-\text{dis}(t_n)} =
\]
\[
= \sum_{i=1}^{n-1} 2 \int_{t_i}^{t_{i+1}} \left[ R_0 \| \dot{\epsilon}_p^p(x, t) \| - \sigma_{ad}(x, t) : \dot{\epsilon}_p^p(x, t) + R_{ad}(x, t) \dot{p}_{ad}(x, t) \right] dt d\Omega.
\]
The expression (5.17) fits well the incremental origin of the admissible solution as described in the previous section, and because $s_{ad}(x, t)$ is continuous piecewise linear over each time interval $[t_i, t_{i+1}]$, it follows

$$
\int_\Omega \int_{t_i}^{t_{i+1}} \left[ R_0 \| \dot{e}_{ad}^p(x, t) \| - \sigma_{ad}(x, t) : \dot{\epsilon}_{ad}^p(x, t) + R_{ad}(x, t) \dot{p}_{ad}(x, t) \right] \, dt \, d\Omega =
$$

$$
= \int_\Omega \left\{ R_0 \| \Delta e_{ad,i}^p(x) \| - \frac{\sigma_{ad}(x, t_{i+1}) + \sigma_{ad}(x, t_i)}{2} : \Delta \dot{e}_{ad,i}^p(x) +
$$

$$+ H(p_{ad}^2(x, t_{i+1}) - p_{ad}^2(x, t_i)) \right\} \, dx
$$

where we have let

$$\Delta e_{ad,i}^p(x) = e_{ad,i}^p(x, t_{i+1}) - e_{ad,i}^p(x, t_i)$$

The value of the dissipation error at $t_1 = 0$, $e_{dis}^2(t_1)$, is assumed equal to zero.

**Error in solution**

In Section 3.5.3.1, as a result of the inequality (3.77) which represents an extension of the Prager-Synge theorem for the dissipation error, it was shown that $e_{dis}^2(T)$ provides an upper bound for the error in solution, $e_{ex}^2(T)$, defined as the error in the constitutive equations produced by $s_{ex, ad} = (\sigma_{ex}, R_{ex}; u_{ad}, e_{ad}^p, p_{ad})$ and given by equation (3.73).

### 5.3.3 Comparison between the two errors

The numerical performance of the dissipation error has been object of study in Moës (1996) and Ladevèze & Moës (1997). Therein, numerical applications are given for 2D models with constitutive equations obeying the Prandtl–Reuss law and the corresponding viscoplastic law. In the following, we are mainly interested in looking at how the dissipation error compares with the extended dissipation error in the assessment of the quality of the same finite element solution. With this regard, the same 1D model problem described in Figure 5.3 and discretized in Figure 5.9 has been considered.

The plots in Figure 5.23 and 5.24 recall the asymptotic behaviour of the dissipation error with respect to the discretization parameters $h$ and $\Delta t$, respectively. These diagrams show the ability of the dissipation error to detect effects of time and space discretization, respectively.

Furthermore, it is worth noting that the time evolution of the dissipation error of an elastic finite element solution is not linear, thus the dissipation error does not reduce to the error in the constitutive equations obtained in the case of linear elasticity, unlike the extended dissipation error. The admissible solution corresponding to an elastic finite element solution must be necessarily a plastic solution: if this was not the case, the admissible solution would be the exact solution, for its associated dissipation would be zero.
Nevertheless, the variation of the error $e_{\text{dis}}$ at $t = 1.5$ with the time step size and for the discrete models which present $t = 1.5$ in the definition of the load levels, show that the time step has no influence on the value of the error. The finite element solution is, in fact, elastic, thus it does not depend on the time discretization and so also the corresponding admissible solution.

Figure 5.25 compares more specifically the time evolution of the dissipation error and of the extended dissipation error for two different discrete schemes. These schemes have been chosen as examples of two extreme situations. In fact, according to the analysis with the extended dissipation error, the error in the evolution law, $\theta_d$, for one scheme and the error in the state law, $\theta_{st}$, for the other, represent the main error components, respectively. We recall that both the dissipation error, $e_{\text{dis}}$, and the extended dissipation error, $e_{\text{ext}}$, are measures of the error in the constitutive equations, but they assess the quality of different admissible solutions corresponding to the same finite element solution.

In both the discrete models, the dissipation error presents values of the error higher than the extended dissipation error and it is closer to the latter for the discrete model which has $\theta_d$ as main error component of $e_{\text{ext}}$. This behaviour is to
be related finally to the major dissipation associated with the admissible solution defined for the computation of the dissipation error. Thus, the previous plots point to the higher effectiveness of the extended dissipation error for the assessment of the quality of the finite element solution.

In conclusion, it is interesting to note the strictly increasing character of the dissipation error during the whole time evolution due to the $L^1$ accumulation in time of the residual of the admissible solution in the evolution law, which is always positive, as it is shown in Figure 5.26. In fact, by adapting the observations of Section 3.5.2, one concludes that there is variation of the state variables if and only if the dissipation error is different from zero.

### 5.4 Concluding Remarks

In this Chapter we have shown how to use the extended dissipation error introduced by Ladevèze et al. (1999) to assess the quality of finite element solutions of elastoplastic problems with the mesh constant throughout the loading process. The main
problem was, therefore, the definition of a corresponding admissible solution, which reflects the approximations associated with the finite element solution. After giving general guidelines, actual criteria to construct an admissible solution in the case of the Prandtl–Reuss model have been given. The general theory has then been applied to assess the quality of the finite element solution of a one dimensional elastoplastic bar under axial load. The extended dissipation error allows one to appreciate the importance of the effects of time discretization and also to show that the regions at the initial stage of plastic deformation contribute significantly to the error component associated with the evolution law, whereas already plastified sub-domains add only low values to the error. This circumstance has been justified on the basis of the specific material model under consideration.

Analysis of the relative importance of the error components associated with the residual in the state law, which depends on the current approximation of the finite element mesh, and the error component associated with the residual in the evolution law, which has nondecreasing character and accounts for the error in the history of the variables, has been presented.

Notable has also been the comparison with classical measures of the exact error in solution. This has showed that the extended dissipation error reflects quite well the evolution of the admissible solution with respect to the exact one as described by more classical measures of the error.

Comparison with the dissipation error defined by Ladevèze (1989) has also been given. Extended dissipation error and dissipation error assess the accuracy of different admissible solutions associated with the same finite element solution. The dissipation error delivered values of the error higher than the extended dissipation error and it was closer to the latter for the discrete model which had $\theta_d$ as the main error component of $e_{ext}$.

In this Chapter, the finite element mesh was constant in time. As a result, the time linear interpolation of the discrete values was a continuous function over the time interval of interest. Consequently, also the corresponding admissible solutions were time continuous. Objective of the next Chapter is to prescribe a change of finite element mesh at a given time instant $t_n$. In this case, the time linear interpolation of the computed finite element solutions and the associated admissible solution will have a discontinuity jump at the time instant $t_n$. The global accuracy in time of this solution will be assessed by means of the augmented extended dissipation error developed in Section 3.5.2.2.
Chapter 6

Numerical studies of transfer operations for adaptive finite element solutions

6.1 Introduction

Use of adaptive strategies in the finite element solution of history-dependent elastoplastic problems with incremental procedures is of paramount importance. An adaptive strategy can be defined as a computational procedure which delivers the finite element solution for the problem at hand to the prescribed accuracy. Key ingredients are, therefore, among others, the availability of an error estimator, which accounts for the sources of error associated with the approximation, and of a transfer procedure, which defines the data of the one step fully discrete problem in the case the current finite element mesh is different from the one of the previous time step.

In the previous chapter, it has been shown that the extended dissipation error applied to the assessment of the accuracy of the finite element solution obtained by a fully implicit displacement formulation of the elastoplastic problem is able to account for the effects of time and space discretization. Therein, the analysis has been carried out by assuming finite element mesh constant during the whole evolution. A property of this error is its non-decreasing character in time due to the accumulation of the discretization errors. As a result, during the computation with incremental procedures, one may need to modify the parameters which define the fully discrete scheme, namely time step size and finite element mesh, in order to obtain the corresponding solution to the prescribed global accuracy.

When only variation of the time step is sufficient to improve the accuracy of the solution, the extended dissipation error presented in the previous chapter can be used to assess the global quality of the finite element approximation because of the time continuity of the associated admissible solution. On contrary, when the finite element mesh is changed at time $t_n$, two finite element solutions are considered for the same load level: the one at $t_n^-$, which is associated with the mesh $T_{h_n}$, (henceforth, called old mesh), and the other at $t_n^+$, which is associated with the
mesh $\mathcal{T}_{n+1}$ (henceforth, referred to as a new mesh). The solution at $t_n^+$ is employed to define a time linear interpolation function which we require to satisfy the following property
\[
\lim_{\Delta t \downarrow 0} f_d(t_n + \Delta t) = \lim_{\Delta t \downarrow 0} f_i(t_n + \Delta t)
\]
where $f_i = f_i(t_n + \Delta t)$ denotes the time linear interpolation over the time interval $[t_n^+, t_{n+1}^-]$ of the discrete values $f_n^+$ and $f_{n+1}^-$ whereas $f_d = f_d(t_n + \Delta t)$ is the function which associates with any given $\Delta t$ the solution of the discrete scheme corresponding to the given $\Delta t$ and data $f_n^+$. Consequently, a discontinuity jump appears in the time linear interpolation of the discrete values across the time node $t_n$ as a result of the change of mesh and transfer procedure. The global accuracy in time of the solution, therefore, will have to depend not only on the time step and finite element mesh size but also on the value of the jump.

The extended dissipation error, augmented in Section 3.5.2.2 by the term which accounts for time discontinuity in the admissible solution, lends itself to be used for this objective. Its applicability will be illustrated on a 1D model problem where several type of change of meshes and transfer procedures (Ortiz & Quigley, 1991; Perić et al., 1996; Rashid, 2002) have been analysed.

6.2 Numerical studies of transfer operations for adaptive finite element solutions

6.2.1 Augmented Extended Dissipation Error

For the implementation of the augmented extended dissipation error, the general concepts given in Section 5.1 remain still valid. An admissible solution as close as possible to the given finite element solution needs first to be defined so that it can mirror all the approximations affecting the finite element solution.

In the following, for the Prandtl Reuss model, we first present, how, given the admissible solution at $t_n^-$ and the finite solution at $t_n^+$, we build the corresponding admissible solution at $t_n^+$. Successively, numerical applications of the augmented extended dissipation error aimed to compare the quality of different transfer procedures will be illustrated. Analysis of the reliability of the new error estimator in reflecting the quality of the finite element solution in the presence of change of mesh will be also performed.

6.2.1.1 Construction of the admissible solution

The Prandtl Reuss plasticity model with Linear Hardening

The essential equations of this model are given in Section 3.2.4.5 and are next recalled.

Yield Condition: $\|\sigma_{ad}^D\| - (R_{ad} + R_0) \leq 0$, $R_{ad} \geq 0$
FINITE ELEMENT SOLUTION AT TIME T

OBTAINED FROM THE FINITE ELEMENT SOLUTION AT T
1

REMARKS

MUST BE MADE. THIS ASSUMPTION IS IMPLICITLY REQUIRED BY THE TRANSFER PROCEDURES

FIND:

Box 6.1. Procedure to build an admissible solution at \( t^+_n \) in presence of change of mesh

**DATA:**  
Admissible solution at \( t^+_n \)  
| \( \sigma_{ad}(x, t^n_n) \), \( R_{ad}(x, t^n_n) \), \( \epsilon_{ad}(x, t^n_n) \), \( e^{ad}_{0}(x, t^n_n) \), \( p_{ad}(x, t^n_n) \). |

Finite element solution at \( t^+_n \)  
| \( u^{h+1}_n(x) \), \( e^{ad}_{0}(x, t^n_n) \), \( e^{ad}_{n+1}(x, t^n_n) \), \( p_{ad}(x, t^n_n) \). |

**FIND:**  
Admissible solution at \( t^+_n \)  
| \( \sigma_{ad}(x, t^+_n) \), \( R_{ad}(x, t^+_n) \), \( \epsilon_{ad}(x, t^+_n) \), \( e^{ad}_{0}(x, t^+_n) \), \( p_{ad}(x, t^+_n) \). |

**WHERE**  
Admissible generalised stress field at \( t^+_n \):  
| \( \sigma_{ad}(x, t^+_n) \), \( R_{ad}(x, t^+_n) \) |

Admissible kinematic solution at \( t^+_n \):  
| \( \epsilon_{ad}(x, t^+_n) \), \( e^{ad}_{0}(x, t^+_n) \), \( p_{ad}(x, t^+_n) \) |

**REMARKS**

1. As already mentioned in Section 4.5, in order to define an admissible solution, a hypothesis on the distribution over each element of the state variables, which are obtained from the finite element solution at \( t^+_n \) at the Gauss points of the new mesh, must be made. This assumption is implicitly required by the transfer procedures.
described in Ortiz & Quigley (1991) and Rashid (2002), for example, and we require it to apply also for the variables defined with the smoothing transfer by Perić et al. (1996). Hereafter, we refer to the distributions depicted in Figure 5.1. As a result, the error in the constitutive equations must be considered as the error associated with this given postulation for the variables distribution which will also allow one to quantify the discontinuity of the fields across the time node \( t_n \).

2. The admissible solution at \( t_n^- \) is known at the Gauss points of the old mesh, which are employed to compute numerically the space integrals that define the error at \( t_n^- \). The element based quadrature of the space integrals that define the error at \( t_n^+ \), on the other hand, requires the knowledge of the admissible solution at \( t_n^+ \) at the quadrature points of the new mesh. In order to implement the procedure shown in Box 6.1, the values of the fields \( \epsilon_{ad}^p(x, t_n^-) \) and \( p_{ad}(x, t_n^-) \) also at the quadrature points of the new mesh are necessary. These are obtained simply by suitable interpolation of their values at the Gauss points of the old mesh.

3. Finally, a special remark deserve the statically admissible stress fields \( \sigma_{ad}(x, t_n^-) \) and \( \sigma_{ad}(x, t_n^+) \) which correspond to the same load level but they are defined as prolongation of different finite element stresses.

![Figure 6.1](image.png)

Figure 6.1: Finite element solutions and admissible solutions for change of finite element mesh \( T_{h_n} \rightarrow T_{h_{n+1}} \) at the time instant \( t_n \).

Figure 6.1 reports schematically the notation relative to the finite element solutions and corresponding admissible solutions in the case of change of finite element mesh at the time instant \( t_n \).

### 6.2.1.2 Error Expressions

We recall hereafter the error expressions for the Prandtl Reuss model under consideration by highlighting the terms due to the change of mesh both in the classical measures of the exact error and in the augmented extended dissipation error.
Classical measures of the exact error in solution

For the same reasons expressed in Section 5.2.2, here we also assume a global control of the exact error, which is of $L^\infty$ type in time and $L^2$ in space of the exact error of the total admissible strain,

$$
\|e_{\text{ad}}\|_{L^\infty([0,T];(L^2(\Omega))^{d\times d})} = \sup_{t \leq T} \|e_{\text{ad}}(x, t)\|_{(L^2(\Omega))^{d\times d}} = \max \left\{ \sup_{t \leq t_n^-} \|e_{\text{ad}}(x, t)\|_{(L^2(\Omega))^{d\times d}}, \sup_{t_n^- \leq t \leq T} \|e_{\text{ad}}(x, t)\|_{(L^2(\Omega))^{d\times d}} \right\}
$$

along with the $L^\infty$ norm in time of the free complementary energy norm of the exact error of the generalised stress field conjugate of the kinematically admissible solution, that is,

$$
\|e_{\text{GSF}}\|_{L^\infty([0,T];V\times M)} = \sup_{t \leq T} \{ \|\varepsilon_\sigma(x, t)\|_V + \|\varepsilon_\sigma(x, t)\|_M \}^{\frac{1}{2}} = \max \left\{ \|e_{\text{GSF}}\|_{L^\infty([0,t_n^-];V\times M)}, \|e_{\text{GSF}}\|_{L^\infty([t_n^+, T];V\times M)} \right\}.
$$

In both the above expressions, the exact error has been split into two terms corresponding to the two different meshes at the time $t_n$. The term relative to the time interval $[0, t_n^-]$ refers to the old mesh, that is, to the initial mesh, whereas the term relative to the time interval $[t_n^+, T]$ refers to the new mesh, that is, to the mesh with improved approximation. The splitting of the error shows that as a result of the $L^\infty$ control in time of the error, which is expressed as a suitable norm in space of the current exact error, there is reduction in the value of the error, that is, the error will not increase, if the current error at $t_n^+$ is not greater than the global error at $t_n^-$. These observations will appear clearer in the following.

Error in the constitutive equations

This error measure is given by equation (3.69) which we recall for reader’s convenience,

$$
e_{\text{ext}}^{n,c^2}(T) = \max \left\{ \sup_{t \leq t_n^-} \left[ \frac{e_{\text{ext}}^{2}(t_n^-)}{\theta_n^{c^2}(t)} + \frac{2 \int_{\Omega} \eta_x^2(t_n^-) d\Omega + 2 \int_{\Omega} \int_0^t d_\tau \eta_x^2 d\tau d\Omega}{\theta_n^{c^2}(t)} \right], \sup_{t_n^- \leq t \leq T} \left[ \frac{\Delta e_{\text{ext}}^{2}(t_n^-)}{\Delta \theta_n^{c^2}(t_n^-)} + \frac{\int_{\Omega} \Delta d_\sigma(x, t_n^-) d\Omega + 2 \int_{\Omega} \int_{t_n^-}^t \Delta d_\sigma(x, t_n^-) d\tau d\Omega}{\Delta \theta_n^{c^2}(t_n^-)} \right] \right\}
$$

(6.1)
In equation (6.1), the notation $e_{ext}^{n,c}(T)$ has been adopted in place of $\Delta e_{ext}^{2}(T)$ used in equation (3.69). Here, the superscripts "o" and "n,c" stand for old and new mesh (after change), respectively. The expressions of $\dot{\eta}_{x,t}^{2}$ and $\eta_{x,t}^{2}$ are reported at the beginning of section 6.2.1.1, whereas for the Prandtl–Reuss model, the term $\Delta \zeta^{2}(x, t_{n})$ is given by equation (3.51), i.e.,

$$\forall x \in \Omega$$

$$\Delta \zeta^{2}(x, t_{n}) = \lim_{\Delta t \to 0+} \int_{x_{n}}^{x_{n}+\Delta t} \{ R_{0} \| e_{ad,\Delta t}^{p}(x, \tau) \| +$$

$$-\sigma_{ad,\Delta t}(x, \tau): \dot{e}_{ad,\Delta t}^{p}(x, \tau) + R_{ad,\Delta t}(x, \tau)p_{ad,\Delta t}(x, \tau) \} d\tau =$$

$$= R_{0} \| e_{ad}(x, t_{n}^{+}) - e_{ad}(x, t_{n}^{-}) \| +$$

$$-\frac{\sigma_{ad}(x, t_{n}^{+}) + \sigma_{ad}(x, t_{n}^{-})}{2} : (e_{ad}(x, t_{n}^{+}) - e_{ad}(x, t_{n}^{-})) +$$

$$+ \frac{R_{ad}(x, t_{n}^{+}) + R_{ad}(x, t_{n}^{-})}{2} (p_{ad}(x, t_{n}^{+}) - p_{ad}(x, t_{n}^{-}))$$

At this point, some comments are deemed useful on the structure of equation (6.1) which will help to gain some insight on the error evolution in presence of change of mesh. These observations recall the one exposed previously for the exact error. Similar remarks are also reported in Ladéveze et al. (1986) and Coffignal (1987) with regard to the error in the constitutive equations according to Drucker inequality. However, in this case, no additional term has been assumed to account for the discontinuity of the admissible solution.

In (6.1) we can distinguish primarily two terms. One, $\theta_{d}^{2}$, is related to the history of the variables by means of an $L^1$ accumulation in time of the error in the evolution law, whereas the other term, $\theta_{ad}^{2}$, depends on the current value of the error in the state law. As a result, further to change of mesh, only the term $\theta_{ad}^{2}$ can be reduced whereas the term $\theta_{d}^{2}$ increases by the quantity $\Delta \theta_{d}^{2}$. Therefore, there is an advantage to change mesh for given definition of the initial data if at least the following inequality is satisfied

$$\theta_{ad}^{2}(t_{n}^{-}) \geq \theta_{ad}^{n,c}(t_{n}^{+}) + \Delta \theta_{d}^{2}(t_{n}).$$

(6.2)

The occurrence of (6.2) guarantees that $\theta_{ad}^{n,c}(t_{n}^{+}) \leq e_{ext}^{o}(t_{n}^{-})$. Figure 6.2 visualizes the meaning of the several terms which appear in (6.1). In the picture, the jump $\Delta$ is given by $\sqrt{\theta_{d}^{2}(t_{n}^{-}) + \Delta \theta_{d}^{2}(t_{n}) - \theta_{d}^{2}(t_{n}^{-})}.$

The augmented extended dissipation error gives the same qualitative information of the exact error in solution which, finally, is to be related to the type of error control in time. However, it also shows that there will be no convenience to change mesh if the error associated with the evolution law, which is the error component
Figure 6.2: Components of the augmented extended dissipation error for change of mesh at \( t_n \). Terms relative to the old mesh (o) and to the new mesh (n,c) after change of mesh.

that cannot be reduced for being associated with the quality of the solution up to the current time \( t_n \), assumes values close to the prescribed global tolerance, that is, if the error associated with the past history of the solution has been relevant. This circumstance would indicate that if a global control of the solution is sought for, the incremental finite element analysis should be repeated from the beginning by starting with a finer initial mesh (see Ladéveze et al., 1986).

**Error in solution.**
The expression of the error in solution is given by equation (5.4). The jump term is not included because of the time continuity of the exact static solution \((\sigma_{ex}(x,t), R_{ex}(x,t))\) as discussed in Section (3.5.3).

### 6.2.1.3 Numerical examples

The performance of the error (6.1) to assess the quality of the finite element solution obtained with an incremental procedure and in presence of change of the finite element mesh at the time instant \( t_n \) is here illustrated on the same 1D model problem as introduced in Section 5.2.3.

The initial fully discretization of the model problem is realized with uniform time step \( \Delta t = 1.5 \) and the non uniform mesh \( me1 \) of 14 linear elements depicted in Figure 6.3. A prescribed type of change of mesh along with a certain definition of the initial data is then assumed to occur at the time \( t_n = 25.5 \). At this time instant plastic loading starts to localize once the load has been reversed in sign. This is shown in Figure 5.5 which illustrates the evolution of the exact solution.

In the following, we will consider first the case of change between embedded meshes, and then the case of not embedded meshes. In both cases, the augmented extended dissipation error (6.1) will then be used to assess the quality of the resulting
finite element solution.

**Analysis of the error: Change between embedded meshes**

The type of change of finite element meshes which is considered in this section is given in the same Figure 6.3. For this example of change of meshes the condition $V_h^n \subset V_h^{n+1}$ is realized between the interpolating spaces of the displacement field by means of a refinement of the old mesh $me_1$. In particular, the new mesh $me_2$ has been obtained by halving the corresponding elements of the mesh $me_1$.

Three types of definition of initial state $\tilde{\epsilon}_p^n(x), \tilde{\rho}_p^n(x)$ on the new mesh $me_2$ to restart the finite element analysis at the time $t_n = 25.5$ have been taken into account. These definitions exemplify the three groups of transfer procedures introduced in Section 4.5: variationally consistent transfer, weak enforcement of continuity and smoothing transfer.

The variationally consistent transfer is obtained by sampling at the Gauss points of the new mesh the fields $h^n\epsilon_p^n(x)$ and $h^n\rho_p^n(x)$, whose distribution assumption must comply with the requirements dictated by equation (4.13). Here, these fields have been obtained over each element as prolongation into a constant function of the value at the respective unique Gauss point used for the quadrature of the elemental contribution to the internal virtual power. Since the elements of the new mesh are obtained by refinement of the corresponding old element, it follows that the mapping of $h^n(\bullet)_n(x)$ into $(\bullet)_n(x)$ reduces to the identity operator. This transfer particularizes to linear elements the transfer adopted by Ortiz & Quigley (1991) for quadratic triangular elements and it is consistent with the constant total strain formulation of the element. This definition is, however, different from the transfer proposed by Radovitzky & Ortiz (1999). These authors, indeed, consider the Voronoi tessellation of the whole domain $\Omega$ defined by all the Gauss points, whereas here we have assumed the Voronoi tessellation of the element to which the Gauss points belong. Consequently, in the former case, the resulting partition of
Figure 6.4: Different plastic strain distribution assumption referred to the finite element mesh me1 and to the Voronoi tessellation of Ω with respect to Gauss points of me1.

Ω will not coincide, in general, with the finite element triangulation and over each linear element a piecewise constant distribution of the variables could be defined. This circumstance is shown, for example, for the element 7 in Figure 6.4 in the case we assume the plastic strain distribution referred to the Voronoi tessellation of Ω.

In the transfer procedure obtained by imposing the weak enforcement of the continuity as in Rashid (2002), the field $(\bullet)_n(x) \in C^{h_{n+1}}$ is obtained as $L^2$ projection onto $C^{h_n}$ of the respective field $h_n(\bullet)_n(x) \in C^{h_n}$. The sets $C^{h_n}$ and $C^{h_{n+1}}$ denote the spaces of the piecewise constant functions over each element of the old mesh $T_{h_n}$ and of the new mesh $T_{h_{n+1}}$, respectively. Since $V^{h_n} \subset V^{h_{n+1}}$, it follows $C^{h_n} \subset C^{h_{n+1}}$, therefore, this transfer, which in the following we refer to as $L^2$ transfer, coincides with the variational consistent transfer defined beforehand, that is, with the identity operator.

Finally, the transfer introduced in Perić et al. (1996) has been used as an example of smoothing transfer. The value of the state variable $h_n(\bullet)_n$ at the Gauss point of each element of the old mesh is first transferred unaltered to the two nodes of the element. An averaging is then carried out at each node and a continuous piecewise linear field is successively built by interpolation of the nodal values by means of the basis functions of the finite element space, $V^{h_n}$, associated with the old mesh. The sampling of this field at the Gauss points of the new mesh provides therein the value of the initial state $(\bullet)_n$. Note that step (d) in Figure 4.5 is not required, since $V^{h_n} \subset V^{h_{n+1}}$ so that the nodal interpolant of a function $v_{h_n} \in V^{h_n}$ with respect to $V^{h_{n+1}}$ reduces to the identity operator. The transfer of the state variable $h_n \epsilon_{p_n}$ is shown, for example, in Figure 6.5.

Once the data $\tilde{\epsilon}_{p_n}, \tilde{p}_n$ at the Gauss points of the new mesh have been assigned, we consider the finite element solution at $t_n^+$ corresponding to load increment equal to zero, i.e., load level equal to $q(x, t_n) = \mu(t_n)x$. In Section 4.4.1 this solution was said to be obtained from the equilibration of the initial state and the time instant
$t^+_n$ was denoting the time instant $t_n + \Delta t$ with $\Delta t \rightarrow 0$. Consequently, the resulting finite element solution will deliver a system state which is in equilibrium with respect to the new mesh.

Likewise for the case of finite element mesh constant in time, plastic strain $h_{n+1} \epsilon^p_n$ and accumulated plastic strain $h_{n+1} p_n$ obtained at the single Gauss points of each element of the new mesh are prolongated into a uniform field over the respective element. The effects of the data equilibration for each transfer are visualized in Figure 6.6. Here, a variation of the initial state defined by the given transfer procedure is noted. In particular, then, a saw–teeth distribution has been obtained in the case of $L^2$ transfer. In this same picture we have also plotted the distributions $h_n \epsilon^p_n(x)$, $h_n p_n(x)$ so that one can appreciate the discontinuity of these fields as a result of the change of mesh.

For the definition of the corresponding admissible solution necessary to compute the augmented extended dissipation error we implement the criteria given in Box 6.1. In particular, for the 1D model problem under consideration, the equilibrated stress field is given by equation (5.5) which can be used also in this context, for its definition depends only upon the current finite element stresses. The admissible plastic strain, on the other hand, is obtained by letting

$$\epsilon^p_{ad}(x, t^+_n) = h_{n+1} \epsilon^p_n(x)$$  \hspace{1cm} (6.3)

if

$$\sigma_{ad}(x, t^+_n) [h_{n+1} \epsilon^p_n(x) - \epsilon^p_{ad}(x, t^-_n)] \geq 0,$$  \hspace{1cm} (6.4)

otherwise we assume

$$\epsilon^p_{ad}(x, t^+_n) = \epsilon^p_{ad}(x, t^-_n),$$  \hspace{1cm} (6.5)

where for the definition of $\epsilon^p_{ad}(x, t^-_n)$ the remarks expressed in Section 6.2.1 have been taken into account.

Figure 6.5: Smoothing transfer of the variable $h_n \epsilon^p_n$
Figure 6.6: Plastic strain and accumulated plastic strain distributions at $t_n = 25.5$ resulting from different transfer assumptions. Finite element solution at $t_n^-$, $h_n(\bullet)_n(x)$; Definition of the data following the transfer operation, $(\bullet)_n(x)$; Equilibration of the data with respect to the new mesh, $h_{n+1}^{-1}(\bullet)_{n+1}(x)$.

Figure 6.7 shows the admissible plastic strain and the admissible accumulated plastic strain distribution at $t_n$ and $t_n^+$. These pictures allow one to appreciate the time discontinuity in these fields as a result of the time discontinuity of the corresponding finite element solutions. Furthermore, in the case of $L^2$ transfer the admissible plastic strain is equal to the corresponding finite element solution almost everywhere, apart from a neighbourhood of the node 15 of the mesh me2. When we use the smoothing transfer, the admissible plastic strain is different also in the elements 17 and 18 of the mesh me2. Therein, in order to guarantee (6.4), definition (6.5) has been used. The accumulated plastic strain $p_{ad}(x, t_n^+)$, conversely, differs from the corresponding finite element solution in almost all elements for both transfers. This happens because $p_{ad}(x, t_n^+)$ is defined in terms of $\Delta e_{ad}^p(x)$ and $p_{ad}(x, t_n^-)$ with the latter accounting for the history of the solution up to the current time $t_n$. 
Figure 6.7: Admissible plastic strain and admissible accumulated plastic strain distributions at \( t_n = 25.5^- \), \( t_n = 25.5^+ \) and plots of \( h_{n+1}(x) \) obtained from equilibration of the data with respect to the new mesh.

Figure 6.8: Evolution in time of the effectivity index for different transfers
Figure 6.9: Time Evolution of the Augmented Extended Dissipation Error with its components for different type of transfer at \( t_n = 25.5 \). \( L^2 \) and Smoothing transfer. Change between embedded meshes. For the meaning of the symbols we refer to Figure 6.2.
Figure 6.10: The error in the elastic law at $t = 25.5^+$ after change of mesh with different transfer assumptions (a) $L^2$ transfer: Admissible stress $\sigma_{ad}$ versus stress conjugate of the admissible elastic strain $C\epsilon_{ad}^e$ (b) $L^2$ transfer: Pointwise contribution to the error in the elastic law, $\frac{1}{C}(\sigma_{ad} - C\epsilon_{ad}^e)^2$ (c) Smoothing transfer: Admissible stress $\sigma_{ad}$ versus stress conjugate of the admissible elastic strain $C\epsilon_{ad}^e$ (d) Smoothing transfer: Pointwise contribution to the error in the elastic law, $\frac{1}{C}(\sigma_{ad} - C\epsilon_{ad}^e)^2$
The finite element solution $h_{n+1} p_n(x)$, on contrary, is computed in terms of $\tilde{p}_n(x)$ which is given by the specific transfer procedure. Consequently, the definition of $p_{ad}$ allows one to account for the approximations associated with the variable up to the current time $t_n$. This is a piece of information essential for the assessment of the global quality in time of the solution.

Figure 6.8 shows that the time variation of the effectivity index for the two schemes resulting from the different transfer assumptions is identical. Both the augmented extended dissipation error and the exact error which enter equation (5.8) involve $L^\infty$ control in time. Consequently, following change of mesh, reduction of the error with value equal to the one related to the same initial mesh me1 is obtained.

In Figure 6.9 we plot the time evolution of the augmented extended dissipation error along with its components. We observe that for both the finite element solutions resulting from the two different transfers, we have similar qualitative behaviour: reduction in the value of the error due to the reduction of the error in the state law and a slight increase of the error in the evolution law. This behaviour of the error shows an improvement of the quality of the solution by considering

<table>
<thead>
<tr>
<th>Scheme</th>
<th>$t_n = 25.5$</th>
<th>$t_{n+1} = 27.0$</th>
</tr>
</thead>
<tbody>
<tr>
<td></td>
<td>$\varepsilon_{ext}$</td>
<td>$\theta_{sl}$</td>
</tr>
<tr>
<td>me1</td>
<td>0.355</td>
<td>0.282</td>
</tr>
<tr>
<td>me2</td>
<td>0.245</td>
<td>0.140</td>
</tr>
<tr>
<td>$me1 \rightarrow me2$</td>
<td>$L^2$ transfer</td>
<td>$t_n^+$</td>
</tr>
<tr>
<td></td>
<td>$me1 \rightarrow me2$</td>
<td>Sm. transfer</td>
</tr>
</tbody>
</table>

Table 6.1: Comparison of the error components at time $t_n = 25.5$ and $t_{n+1} = 27.0$
Figure 6.12: The error in the hardening law at $t = 25.5$ after change of mesh with different transfer assumptions (a) $L^2$ transfer: Admissible thermodynamic force $R_{ad}$ versus force conjugate of the admissible accumulated plastic strain $H_{p_{ad}}$ (b) $L^2$ transfer: Pointwise contribution to the error in the hardening law, $\frac{1}{H}(R_{ad} - H_{p_{ad}})^2$ (c) Smoothing transfer: Admissible thermodynamic force $R_{ad}$ versus force conjugate of the admissible accumulated plastic strain $H_{p_{ad}}$ (b) Smoothing transfer: Pointwise contribution to the error in the hardening law, $\frac{1}{H}(R_{ad} - H_{p_{ad}})^2$

both types of transfers. In particular, the finite element solution resulting from $L^2$ transfer appears to behave slightly better. This is shown by the time variation of the current error $\theta^n_{n,c}$ which is closer to the time variation of the error $e^n_{ext}$ which is obtained with the finite element mesh $me2$ constant in time. This can be better appreciated also in Table 6.1 which reports the values at $t^n$, $t^n_+$ and $t^n_{n+1}$ of the several components of the augmented extended dissipation error defined by equation (6.1). In the same table, for completeness, we have also given the values which are obtained by assuming the finite element meshes $me1$ and $me2$ constant in time during the whole evolution. The values at the time $t^n_{n+1}$, conversely, are reported to illustrate the influence of the transfer procedure also at later time. In the case at
hand, however, the error behaviour at $t_n^+$ is similar to the one at $t_{n+1}$.

The Figures 6.10–6.14 permit one to appreciate the source of the difference of values of the error in the case of the two transfers. This is accomplished by showing the pointwise contribution to the error components at the time $t_n^+$. In particular, Figure 6.10 displays the pointwise contribution to the error in the elastic law which shows a major contribution coming from the elements 12, 15, 17 and 18 as a result of the different distribution of the admissible elastic strain therein. This, in turn, reflects for the problem at hand the difference of admissible plastic strain, given that the distribution of the total strain is similar, as shown in Figure 6.11.

The pointwise contribution to the error in the hardening law is given in Figure 6.12 which shows a slightly higher contribution in the case of smoothing transfer whereas the Figures 6.13 and 6.14 allow comparison of different contributions to the jump term $\Delta \theta_d(t_n)$ for the $L^2$ transfer and Smoothing transfer, respectively.
Figure 6.14: Smoothing transfer (a) Variation of admissible plastic strain at $t_n = 25.5$ (b) Space distribution of $f(\sigma_{ad}, R_{ad})$ at $t_n^-$ and $t_n^+$ (c) Pointwise contribution to the jump term $\Delta \theta_d(t_n)$

For the latter case, the major contribution comes from the elements 14 and 15. In these elements, the smoothing transfer assumption produces a variation of admissible plastic strain, as shown in Figure 6.14(a), whereas the variation of $f(\sigma_{ad}, R_{ad})$, given in Figure 6.14(b) indicates that the behaviour associated with $(\sigma_{ad}, R_{ad})$ should be elastic, since $f \leq 0$ therein.

Finally, Figure 6.15 reports the classical measures of the error introduced in Section 6.2.1.2. These error measures exhibit the same qualitative behaviour as the augmented extended dissipation error with the reduction in the value of the error. They also show an improved behaviour of the finite element solution corresponding to the $L^2$ transfer. This is made clear from the time evolution of the current error of the generalised stress field $\|e^{n,c}_{GSF}\|$, which is closer to the evolution of the global error $\|e^0_{GSF}\|$. The latter is obtained by assuming the constant finite element mesh $me2$ throughout the whole loading process. Therefore, it can be said that the augmented extended dissipation error is capable to mirror the approximation associated with finite element solutions in the presence of change of mesh.
Figure 6.15: Time Evolution of the Exact Error for different type of transfers at \( t_n = 25.5 \).

The meaning of the symbols used is the following: \( \forall t \in I, (\bullet)(t) = \sup_{\tau \leq t} \| (\bullet)(x, \tau) \|_{L^2(\Omega)} \) and \( (\bullet)(t) = \| (\bullet)(x, t) \|_{L^2(\Omega)} \), whereas the superscripts "o", "n", "c" and "n" retain the usual meaning.

Analysis of the error: Change between non–embedded meshes

In this second example, a change between non–embedded meshes is assumed to occur at the time \( t_n = 25.5 \). As a result, the condition \( \mathcal{V}^h_n \subset \mathcal{V}^h_{n+1} \) is no more realized. Nevertheless, the mesh associated with \( \mathcal{V}^h_{n+1} \) is chosen to contain a bigger number of elements. In particular, the new mesh \( m2es \) has been obtained by considering 28 linear equally spaced finite elements. Figure 6.16 shows the time discretization and the time instant when the change from the old mesh \( me1 \) to the new mesh \( m2es \) occurs.

Likewise the previous example, the three types of transfers introduced in Section 4.5 are next particularized for the change of mesh considered here.

When one adopts the variationally consistent transfer, the initial state \( \tilde{\epsilon}^n_p(x), \tilde{\rho}_n(x) \) is obtained by sampling the fields \( h^n \epsilon^p_n(x) \) and \( h^n \rho_n(x) \) at the Gauss points of the new mesh, respectively. Consistently with the choice of one Gauss point per element for the integration of the constitutive equations, the fields \( h^n \epsilon^p_n(x) \) and \( h^n \rho_n(x) \) are in turn assumed constant over each element of the old mesh. The piecewise value is equal to the computed finite element solution at the respective Gauss point of the element. Therefore, in order to determine the value of the data \( \tilde{\epsilon}^n_p(x), \tilde{\rho}_n(x) \) at the element Gauss point of the new mesh, \( \tilde{x}^{h_{n+1}}_{GP} \in \mathcal{V}_q^{h_{n+1}} \), one needs...
to identify first the element $\Omega^{h_n} \in \mathcal{T}_h$ of the old mesh such that $x_{e,GP}^{h_{n+1}} \in \Omega^{h_n}$. Then, by denoting with $x_{e,GP}^{h_n}$ the Gauss point of the element $\Omega^{h_n}$, one assumes

$$\left(\tilde{\mathbf{\varepsilon}}_{n}^{h}(x)\right)_{h_n}(x_{e,GP}) = h_n \left(\mathbf{\varepsilon}_{n}^{h}(x_{e,GP})\right) \quad (6.6)$$

If $x_{e,GP}^{h_{n+1}}$ lies on the boundary of the element $\Omega^{h_n}$, the average value of the variables between the two neighbouring elements is assumed. The definition of the variationally consistent transfer of the variable $h_n \mathbf{\varepsilon}_{n}^{h}(x)$ is visualized in Figure 6.17.

The initial state $\mathbf{\varepsilon}_{n}^{h}(x)$, $\bar{p}_{n}(x)$ obtained from the weak enforcement of the con-
Figure 6.18: Weak enforcement of the continuity between the fields \( h_n \epsilon^p_n(x) \) and \( \tilde{\epsilon}^p_n(x) \)

continuity with the fields \( h_n \epsilon^p_n(x) \), \( h_n p_n(x) \) as introduced in Rashid (2002) is given by

\[
\forall l = 1, 2, \ldots, N_{h_{n+1}}
\]

\[
(\tilde{\epsilon}^p_n)_n(x) = \int_{x_{l+1}^{h_{n+1}}}^{x_l^{h_{n+1}}} h_n(\bullet)_n(x) \, dx
\]

\( \forall x \in [x_l^{h_{n+1}}, x_{l+1}^{h_{n+1}}] \quad (6.7) \)

where \( N_{h_{n+1}} \) is the number of elements in the triangulation \( T_{h_{n+1}} \) and the superscript "l" stands for element. The transfer defined by equation (6.7) assumes a constant value for \( (\tilde{\epsilon}^p_n)_n(x) \) over each element \( \Omega^{h_{n+1}}_l \). This value is equal to the weighted average of the field \( h_n(\bullet)_n(x) \), with the weight given by the area of the so-called tributary regions. For the problem at hand, these regions are defined as the parts of the element, \( \Omega^{h_{n+1}}_l = [x_l^{h_{n+1}}, x_{l+1}^{h_{n+1}}] \), of the new mesh where the field \( h_n(\bullet)_n(x) \) is constant. The definition of \( \tilde{\epsilon}^p_n(x) \) is visualized in Figure 6.18.

**Remark 6.1.** Unlike the case \( \mathcal{V}^{h_n} \subset \mathcal{V}^{h_{n+1}} \), since now \( \mathcal{V}^{h_n} \not\subset \mathcal{V}^{h_{n+1}} \), the \( L^2 \) transfer and the variationally consistent transfer describe two different procedures which are not coincident with the identity operator.

Finally, the smoothing transfer has been described in general in Figure 4.5. For the problem at hand, the transfer of the variable \( h_n \epsilon^p_n(x) \) is given in Figure 6.19.

The stresses obtained by solving the incremental constitutive equations with data \( \tilde{\epsilon}^p_n, \tilde{p}_n \) and the displacement field \( u_{h_n} = u_{h_n}(x, t_n^-) \) are not in equilibrium with respect to the test functions associated with the new mesh. Therefore, the resulting residual must be equilibrated with consequent redistribution of the stresses and of the state variables. To achieve this, a finite element analysis with the new mesh and load increment equal to zero, i.e., load level equal to \( q(x, t_n) = \mu(t_n)x \), is performed. In this manner, the ensuing finite element solution will deliver at \( t_n^+ \) a system state which is in equilibrium with respect to the new mesh.
The computed plastic strain $h_{n+1} \epsilon^p_n$ and accumulated plastic strain $h_{n+1} p_n$ obtained at the single Gauss points of each element of the new mesh are prolongated into a uniform field over the respective element. The effects of the data equilibration for each transfer are visualized in Figure 6.20. In general, a variation of the initial state defined by the given transfer procedure is obtained. In particular, when the variationally consistent transfer is used, the initial state $\hat{\epsilon}^p_n(x), \hat{p}_n(x)$, by definition, presents very little variation with respect to the finite element solution $h_n \epsilon^p_n(x), h_n p_n(x)$ obtained on the old mesh $me1$. Consequently, the equilibration of the data with respect to the new mesh $me2es$ produces a non uniform redistribution of plastic strain and accumulated plastic strain with concentration of plastic strain especially in the elements 23 and 27. This delivers a picture of the plastic strain distribution at $t_n^+$ which appears to be substantially different from $h_n \epsilon^p_n(x)$, where $h_n \epsilon^p_n(x)$ denotes the plastic strain distribution at $t_n$ obtained from the finite element solution with mesh $me2es$ constant throughout the whole evolution.

Also with the $L^2$ and smoothing transfer we obtain non uniform redistribution of the variables following the equilibration of the data. However, this does not produce plastic distributions at $t_n^+$ substantially different from $h_n \epsilon^p_n(x)$.

Once the finite element solution has been computed at $t_n^+$, the criteria given in Box 6.1 are implemented to build the corresponding admissible solution necessary to evaluate the augmented extended dissipation error. The distribution of the admissible plastic strain and admissible accumulated plastic strain at $t_n^-$ and $t_n^+$ are given in Figure 6.21. A time discontinuity for these fields is introduced at $t_n^+$ as a result of the time discontinuity of the corresponding finite element solutions.

The admissible plastic strain at time $t_n^+$ is equal to the corresponding finite element solution wherever condition (6.4) is satisfied, otherwise it is assumed equal to the value at $t_n^-$. The accumulated plastic strain $p_{ad}(x, t_n^+)$, conversely, differs from the corresponding finite element solution in all the elements of the fully discrete schemes resulting from the three different transfer procedures.
Figure 6.20: Plastic strain and accumulated plastic strain distribution at $t_n = 25.5$ resulting from different transfer operations.
Variationally consistent transfer

$L^2$ transfer

Smoothing transfer

Figure 6.21: Admissible plastic strain and admissible accumulated plastic strain distribution at $t_n = 25.5^-$, $t_n = 25.5^+$ and plots of $h_{n+1}(\bullet)_n(x)$ obtained from equilibration of the data with respect to the new mesh.
This difference is due to the definition of \( p_{\text{ad}}(x, t_n^+) \) in terms of \( p_{\text{ad}}(x, t_n^-) \) and represents an essential feature in the assessment of the global quality in time of the solution. In fact, \( p_{\text{ad}}(x, t_n^-) \) accounts for the history of the solution up to the current time \( t_n \) whereas the finite element solution \( h_{n+1} p_n(x) \) is computed in terms of \( \hat{p}_n(x) \). The latter field is given by the specific transfer procedure, thus, information on the accuracy associated with the past values of the solution could be lost. The definition of \( p_{\text{ad}} \), on contrary, allows one to account for the approximations associated with the variable up to the current time \( t_n \).

Figure 6.22 shows the time variation of the effectivity index which is identical for the three schemes. Both the augmented extended dissipation error and the error in solution, which enter equation (5.8), involve \( L^\infty \) control in time. Consequently, following the change of mesh, reduction in the value of the error with value equal to the one relative to the same initial mesh \( me1 \) is obtained for both the augmented extended dissipation error and the error in solution.

The time evolutions of the augmented extended dissipation error of the admissible solutions corresponding to the finite element solutions resulting from the three different transfers are given in Figure 6.23. All the diagrams present similar qualitative behaviour: reduction in the value of the error due to the reduction of the error in the state law and a slight increase of the error in the evolution law is noted. Therefore, there is globally an improvement of the quality of the solution by considering the proposed transfers for the given change of mesh. In particular, the solution resulting from \( L^2 \) transfer appears to behave better out of the proposed transfer procedures. This is shown by the time variation of the current error \( \theta^{n,c}(t) \). In the case of the \( L^2 \) transfer, the evolution of \( \theta^{n,c}(t) \) is the closest to \( e_{\text{ext}}^n(t) \), where \( e_{\text{ext}}^n(t) \) is the extended dissipation error which is obtained with the constant finite element mesh \( me2es \) throughout the loading process.

Table 6.2 contains the values at \( t_n^- \), \( t_n^+ \) and \( t_{n+1} \) of the several components of the augmented extended dissipation error defined by equation (6.1). This table allows one to appreciate the differences between different transfer procedures. For
completeness, we have also given the values which are obtained by assuming the finite element meshes \(me1\) and \(me2es\) constant in time during the whole loading process, whereas the values at the time \(t_{n+1}\) are reported to illustrate the influence of the transfer procedure at a later time.

<table>
<thead>
<tr>
<th>(e_{ext})</th>
<th>(\theta_{sl})</th>
<th>(\frac{\theta_{sl}^+, \theta_{sl}^-}{\theta_{sl}^+})</th>
<th>(\delta\theta_d)</th>
<th>(\Delta \theta_d)</th>
<th>(e_{ext})</th>
<th>(\theta_{sl})</th>
<th>(\frac{\theta_{sl}^+, \theta_{sl}^-}{\theta_{sl}^+})</th>
<th>(\theta_d)</th>
</tr>
</thead>
<tbody>
<tr>
<td>(me1)</td>
<td>0.355</td>
<td>0.282</td>
<td>0.141</td>
<td>0.215</td>
<td>-</td>
<td>0.396</td>
<td>0.319</td>
<td>0.160</td>
</tr>
<tr>
<td>(me2es)</td>
<td>0.236</td>
<td>0.131</td>
<td>0.057</td>
<td>0.196</td>
<td>-</td>
<td>0.245</td>
<td>0.146</td>
<td>0.063</td>
</tr>
<tr>
<td>(me1 \rightarrow me2es)</td>
<td>0.355</td>
<td>0.173</td>
<td>0.076</td>
<td>0.218</td>
<td>0.033</td>
<td>0.355</td>
<td>0.190</td>
<td>0.075</td>
</tr>
<tr>
<td>Var. transfer (t_n^+)</td>
<td>0.156</td>
<td>(t_{n+1}^+)</td>
<td>(t_n^+)</td>
<td>(t_{n+1}^+)</td>
<td>(t_n^+)</td>
<td>(t_{n+1}^+)</td>
<td>(t_n^+)</td>
<td>(t_{n+1}^+)</td>
</tr>
<tr>
<td>(me1 \rightarrow me2es)</td>
<td>0.355</td>
<td>0.184</td>
<td>0.110</td>
<td>0.216</td>
<td>0.014</td>
<td>0.355</td>
<td>0.159</td>
<td>0.074</td>
</tr>
<tr>
<td>(L^2) transfer (t_n^+)</td>
<td>0.148</td>
<td>(t_{n+1}^+)</td>
<td>(t_n^+)</td>
<td>(t_{n+1}^+)</td>
<td>(t_n^+)</td>
<td>(t_{n+1}^+)</td>
<td>(t_n^+)</td>
<td>(t_{n+1}^+)</td>
</tr>
<tr>
<td>(me1 \rightarrow me2es)</td>
<td>0.355</td>
<td>0.222</td>
<td>0.144</td>
<td>0.225</td>
<td>0.064</td>
<td>0.355</td>
<td>0.179</td>
<td>0.081</td>
</tr>
<tr>
<td>Sm. transfer (t_n^+)</td>
<td>0.168</td>
<td>(t_{n+1}^+)</td>
<td>(t_n^+)</td>
<td>(t_{n+1}^+)</td>
<td>(t_n^+)</td>
<td>(t_{n+1}^+)</td>
<td>(t_n^+)</td>
<td>(t_{n+1}^+)</td>
</tr>
</tbody>
</table>

Table 6.2: Comparison of the error components at time \(t_n = 25.5\) and \(t_{n+1} = 27.0\)

The examination of this table shows that when the variationally consistent transfer is used, the least free energy norm of the error, \(\theta_{sl}^+\), at \(t_n^+\) is attained. This is essentially the result of the best fit between the admissible stress and the stress conjugate to the admissible elastic strain as shown in Figure 6.24. The fit between the admissible thermodynamic forces and the forces conjugate to the admissible accumulated plastic strain, depicted in Figure 6.25, conversely, appears to be best in the case of the \(L^2\) transfer. As for the effects of the transfer, we have already mentioned that with the adoption of the variationally consistent transfer, the non uniform redistribution of the initial state \(\tilde{\epsilon}_p\), \(\tilde{p}_n\) following the equilibration of the data produces concentration of plastic strain in the elements 23 and 27. This, in turn, gives rise to an admissible accumulated plastic distribution \(p_{ad}(x, t_n^+\) which is substantially different from the distribution of the admissible hardening forces at \(t_n^+\). This difference is kept also at \(t_{n+1}\) and is the cause of the increase of the error associated with the residual in the hardening law at the time \(t_{n+1}\).

With the \(L^2\) and smoothing transfer, on contrary, the error in the state law at \(t_{n+1}\) decreases with respect to \(t_n^+\). This decrease can be considered due mainly to the enhanced approximation properties of the new interpolation space whose effects are soon evident on the variation of admissible plastic strain. Therefore, for these two transfers, unlike the variationally consistent transfer, the values of the error at \(t_n^+\) can be assumed to reflect the effects more pertinent to the transfer procedure. For the variationally consistent transfer, on contrary, also the value of the error at \(t_{n+1}\) must be considered.

Likewise the case of change between embedded meshes, Figures 6.26–6.28 allow one to compare the different contributions to the jump term \(\Delta \theta_d(t_n)\) for the transfer procedures under consideration. This appears to be highest in the case of
the smoothing transfer assumption. The major contribution arises also here from
the elements 15, 16 and 18 and must be related to the plastic strain which is therein
introduced with the transfer and to the evolution of the admissible generalised stress
field.

Finally, Figure 6.29 reports the classical measures of the error introduced in
Section 6.2.1.2. These diagrams present the same qualitative behaviour as the aug-
mented extended dissipation error. Therefore, it is possible to assess the advantage
of change of mesh which is to be related to the reduction in the value of the error.
They also show a better behaviour of the admissible solution corresponding to the
$L^2$ transfer. This does appear from the time evolution of the current error of the
generalised stress field $\|\epsilon_{GSF}^{n,c}(t)\|$. In fact, the variation of $\|\epsilon_{GSF}^{n,c}(t)\|$ is closer to
the evolution of the global error $\|\epsilon_{GSF}^n(t)\|$ that is obtained by assuming the constant
finite element mesh $me2es$ throughout the loading process. Therefore, likewise the
previous example, it can be said that the augmented extended dissipation error is
capable to mirror the approximations associated with finite element solutions also
in presence of change between non–embedded meshes.
Variationally consistent transfer

\textit{Augmented Extended Dissipation Error and its components}

\textit{L}^2 \text{ transfer}

\textit{Smoothing transfer}

Figure 6.23: Time Evolution of the Augmented Extended Dissipation Error with its components for different type of transfer at \( t_n = 25.5 \). Variationally consistent, \( L^2 \) and Smoothing transfer.
Variationally consistent transfer

(a) Variationally consistent transfer: Admissible stress $\sigma_{ad}$ versus stress conjugate of the admissible elastic strain $C e_{ad}^e$

(b) Variationally consistent transfer: Pointwise contribution to the error in the elastic law, $1 \left( \sigma_{ad} - C e_{ad}^e \right)^2$

$L^2$ transfer

(c) $L^2$ transfer: Admissible stress $\sigma_{ad}$ versus stress conjugate of the admissible elastic strain $C e_{ad}^e$

(d) $L^2$ transfer: Pointwise contribution to the error in the elastic law, $1 \left( \sigma_{ad} - C e_{ad}^e \right)^2$

Smoothing transfer

(e) Smoothing transfer: Admissible stress $\sigma_{ad}$ versus stress conjugate of the admissible elastic strain $C e_{ad}^e$

(f) Smoothing transfer: Pointwise contribution to the error in the elastic law, $1 \left( \sigma_{ad} - C e_{ad}^e \right)^2$

Figure 6.24: The error in the elastic law at $t = 25.5^+$ after change of mesh with different transfer operations (a) Variationally consistent transfer: Admissible stress $\sigma_{ad}$ versus stress conjugate of the admissible elastic strain $C e_{ad}^e$ (b) Variationally consistent transfer: Pointwise contribution to the error in the elastic law, $1 \left( \sigma_{ad} - C e_{ad}^e \right)^2$ (c) $L^2$ transfer: Admissible stress $\sigma_{ad}$ versus stress conjugate of the admissible elastic strain $C e_{ad}^e$ (d) $L^2$ transfer: Pointwise contribution to the error in the elastic law, $1 \left( \sigma_{ad} - C e_{ad}^e \right)^2$ (e) Smoothing transfer: Admissible stress $\sigma_{ad}$ versus stress conjugate of the admissible elastic strain $C e_{ad}^e$ (f) Smoothing transfer: Pointwise contribution to the error in the elastic law, $1 \left( \sigma_{ad} - C e_{ad}^e \right)^2$
Figure 6.25: The error in the hardening law at $t = 25.5^+$ after change of mesh with different transfer operations (a) Variationally consistent transfer: Admissible thermodynamic force $R_{ad}$ versus force conjugate of the admissible accumulated plastic strain $H_{p_{ad}}$ (b) Variationally consistent transfer: Pointwise contribution to the error in the hardening law, $\frac{1}{\mathcal{H}}(R_{ad} - H_{p_{ad}})^2$ (c) $L^2$ transfer: Admissible thermodynamic force $R_{ad}$ versus force conjugate of the admissible accumulated plastic strain $H_{p_{ad}}$ (d) $L^2$ transfer: Pointwise contribution to the error in the hardening law, $\frac{1}{\mathcal{H}}(R_{ad} - H_{p_{ad}})^2$ (e) Smoothing transfer: Admissible thermodynamic force $R_{ad}$ versus force conjugate of the admissible accumulated plastic strain $H_{p_{ad}}$ (f) Smoothing transfer: Pointwise contribution to the error in the hardening law, $\frac{1}{\mathcal{H}}(R_{ad} - H_{p_{ad}})^2$
Figure 6.26: Variationally consistent transfer (a) Variation of admissible plastic strain at $t_n = 25.5$ (b) Space distribution of $f(\sigma_{ad}, R_{ad})$ at $t_n^- and t_n^+$ (c) Pointwise contribution to the jump term $\Delta \theta_d(t_n)$
Figure 6.27: $L^2$ transfer (a) Variation of admissible plastic strain at $t_n = 25.5$ (b) Space distribution of $f(\sigma_{ad}, R_{ad})$ at $t_n^-$ and $t_n^+$ (c) Pointwise contribution to the jump term $\Delta \theta_{il}(t_n)$
Smoothing transfer

Figure 6.28: Smoothing transfer (a) Variation of admissible plastic strain at $t_n = 25.5$ (b) Space distribution of $f(\sigma_{ad}, R_{ad})$ at $t^-_n$ and $t^+_n$ (c) Pointwise contribution to the jump term $\Delta \theta_d(t_n)$
Figure 6.29: Time Evolution of the Exact Error for different type of transfers at $t_n = 25.5$. The meaning of the symbols used is the following: $\forall t \in \mathcal{T}, (\bullet)(t) = \sup_{\tau \leq t} \| (\bullet)(x, \tau) \|_{L^2(\Omega)}$ and $\tau(\bullet)(t) = \| (\bullet)(x, t) \|_{L^2(\Omega)}$, whereas the superscripts "o", "c" and "n" retain the usual meaning.
6.3 Concluding remarks

When the finite element mesh is changed at time instant $t_n$, a discontinuity jump is introduced in the solution at the time instant $t_n$. The jump produces a deterioration of the global accuracy of the approximation which is due both to the low order regularity of the approximation across the time node $t_n$ and to the eventual diffusion of inelastic deformations as a result of the transfer procedure. However, change of mesh along with suitable definition of the data can enhance the quality of the solution because of the improved approximation properties of the new finite element subspace. Therefore, the advantage of changing mesh with a transfer procedure depends on the interplay between the above opposing features.

In this chapter we have presented a general methodology for the assessment of the global quality of displacement finite element solutions of elastoplastic problems discretized in time with the backward Euler method on dynamically changing mesh.

This methodology employs the extended dissipation error, augmented by the term which accounts for the time discontinuity in the admissible solutions. The applicability of this new error estimator has been illustrated on a one dimensional model problem. Here, the behaviour of the finite element solution with respect to several definitions of transfer procedures and type of change of meshes has been considered. We have shown that the proposed error estimator reflects quite well the several sources of approximations, which result from the change of finite element mesh with the time step. Furthermore, the new error estimator accounts also for the improved approximation property, which arises from the enrichment of the finite element space and from suitable definition of the data on the new mesh. Therefore, the augmented extended dissipation error represents an effective tool for the critical assessment of the effects of transfer procedures for evolving meshes in small strain elastoplasticity.
Chapter 7
Conclusions

A general methodology for the assessment of the global quality of displacement finite element solutions of elastoplastic problems discretized in time with the backward Euler method on dynamically changing mesh has been presented.

The motivating idea has been the observation that change of data and/or finite element mesh from one time interval to the other can be both related to a discontinuity jump of the approximate solution across the time instant $t_n$. Consequently, in the development of reliable a posteriori error estimates one needs to account not only for the time step and finite element mesh size but also for the value of the jump. Two simple error analysis of a first order ordinary differential equation, chosen as elementary prototype of the evolution law of the internal variables, show the influence on the error of the discontinuity jump, thus, the need for including such term in an a posteriori error estimate.

As a result, only measures of error that account for time discretization effects can reflect the low order regularity of the approximation across the time instant $t_n$ when the change of mesh occurs. Thus, the extended dissipation error introduced by Ladevèze et al. (1999) can be used for this aim. This is a measure of the error in the constitutive equations, that is, the state law and the evolution law, produced by an admissible solution which is time continuous, and satisfies the equilibrium and the kinematic conditions at any time instant.

The extended dissipation error capability to capture the effects of time and space discretization has been shown in the assessment of the quality of finite element solutions of elastoplastic problems with the mesh constant throughout the loading process. Criteria to construct an admissible solution, which mirrors the approximations associated with the finite element solution, have been given for the Prandtl–Reuss plasticity model and illustrated with a numerical example. This has shown that all trends on the state law and dissipation contribution to the error were meaningful. Notable was also the comparison with classical measures of the exact error in solution showing that the extended dissipation error reflects quite well the evolution of the admissible solution with respect to the exact one as described by more classical measures of the error.

However, the extended dissipation error given in Ladevèze et al. (1999) assumes
time continuity of the admissible solution. Thus, in the second part of Chapter 3, we
have defined a new measure of the error in the constitutive equations which accounts
for the discontinuity jump in the admissible solution. The theory has been developed
for rate–independent plasticity material models and exploits the observation that
the solution of the initial boundary value problem, which governs the evolution of
this class of material models, depends only on the sequence of load levels while
time has just the function of ordering this sequence. Thus, fictitious continuous
admissible processes have been defined over the time interval \([t_n, t_n + \Delta t]\), with \(\Delta t\)
not influential in the solution, along which the discontinuity was assumed to be
taking place.

The analysis of the error in the constitutive equations along the aforementioned
fictitious processes has lead to the definition of an additional nonnegative term which
was depending on the jump, in agreement with the simple a posteriori error analysis
of the first order differential equation. This term and the behaviour of the error
component in the state law characterize completely the discontinuity jump. This is
the content of Theorem 3.8 of Chapter 3 which motivates the use of the augmented
extended dissipation error as basis of a methodology for the assessment of the global
accuracy in time of finite element solutions on evolving meshes.

Applications of the theory have been presented for the displacement finite ele-
ment solution of the Prandtl-Reuss plasticity model solved with incremental proce-
dure. For this material model we have given the criteria to build admissible solutions
as close as possible to the computed finite element solution.

The applicability of the methodology has been finally illustrated on a one di-
imensional model problem where a detailed study of transfer operators (Ortiz &
Quigley, 1991; Perić et al., 1996; Rashid, 2002) has been carried out, with the nu-
merical experiments providing confirmation of the theoretical developments. The
augmented extended dissipation error was able to mirror the several sources of ap-
proximations which are incurred by the change of finite element mesh with the time
step. Also, it was able to account for the improved approximation property, which
were arising from the enrichment of the finite element space and from suitable defi-
nition of the data on the new mesh. Therefore, the augmented extended dissipation
error has proven to be an effective tool for the critical assessment of the effects of
transfer procedures for evolving meshes in small strain elastoplasticity.

7.1 Suggestions for further research

Although notable are the advances in the numerical simulation of more and more
complex physical models, very little has been done on the corresponding side of
assessment of the accuracy of the produced approximation.

This work has an aim to indicate a new way of designing adaptive strategies
for problems solved with incremental procedure. In this sense, still further research
is necessary. The following points bring up some issues that deserve special attention.
Definition of a new transfer.

It has since been recognized that the issues of how to change mesh and the definition of data is a very delicate matter in the adaptive solution of history dependent material models solved with incremental procedures.

To date, the available transfer operations have been introduced upon the request to meet several vaguely defined properties which are invoked to prevent corrupting the quality of the resulting finite element solution (Ortiz & Quigley, 1991; Perić et al., 1996; Rashid, 2002). With the methodology set in this work, a more rational treatment of the transfer operation seems possible to be devised in the context of the ensuing error. This should therefore lead to the definition of a transfer operation such as the one that minimizes the error produced. The definition of time step size, mesh size and indication on how to change mesh and to give data are not separate steps arising from heuristic arguments but should result from a unified analysis of the error contribution of each component.

Extension to higher dimensional problems.

The theory presented in this work has been formulated in tensorial notations. Thus, its application to higher dimensional problems should not give further complications apart from the implementation aspects. Criteria to build statically admissible stress fields are already in place (Ladevèze & Leguillon, 1983; Ladevèze et al., 1991; Ladevèze & Rougeot, 1997) whereas the definition of the other internal variables to build an admissible solution has been indicated in this work.

Extension to viscoplasticity.

The considerations of Section 3.5.2 cannot be carried over as they stand to a viscoplastic model. For example, in the viscoplastic model corresponding to the Prandtl–Reuss plasticity model (Ladevèze & Pelle, 2001), the dissipation potential \( \varphi(\dot{\varepsilon}^p, -\dot{p}) \) is given by

\[
\varphi(\dot{\varepsilon}^p, -\dot{p}) = R_0 \|\dot{\varepsilon}^p\| + k \frac{n}{n+1} \left( \frac{\dot{p}}{k} \right)^{\frac{n+1}{n}} + I_C
\]

where \( C \) is the same domain defined as in the Prandtl–Reuss plasticity model and \( k, n \) are material constants, with \( n > 0 \). Thus, it follows that \( \gamma \overset{\text{def}}{=} \frac{n+1}{n} > 1 \). As a result, in presence of a discontinuity jump in the internal variable \( p, \dot{p} \) is a \( \delta \)-Dirac type distribution. Hence, \( (\delta)^\gamma \) with \( \gamma > 1 \) is no more a distribution (Schwartz, 1966) and the limit (3.48), which had been assumed as error associated with the discontinuity jump, is not finite.

In this case, it can be worth exploiting the following idea: Starting from the expression of the dissipation error for a time continuous fictitious admissible solution defined over \([t_n, t_n + T_c] \), where \( T_c \) stands for a critical time to be computed, we assume that, for instance, the continuous admissible plastic strain has the following expression

\[
\varepsilon^p(x, t) = \varepsilon^p(x, t_n) + [\varepsilon^p(x, t_n^+) - \varepsilon^p(x, t_n^-)] \varphi(t)
\]
with the jump $[\mathbf{e}_p]_{t_n}$ very small so that $[\mathbf{e}_p]_{t_n} \varphi(t)$ may be assumed as a variation.

Applying the methods of the calculus of the variations, we can state the following problem: For given $T_c$ find the function $\varphi(t)$ which minimizes the error.

The function $\varphi(t)$ is required to meet the boundary conditions $\varphi(t = 0) = 0$, $\varphi(t = T_c) = 1$ and the minimization should be carried out in a-dimensional format so that a certain value for $T_c$ can be computed.

The extension to viscoplasticity should also be such that when the viscoplastic model reduces to the corresponding rate independent model, the expression of the error reduces to the one obtained for the corresponding rate independent model.
Bibliography


214


221


